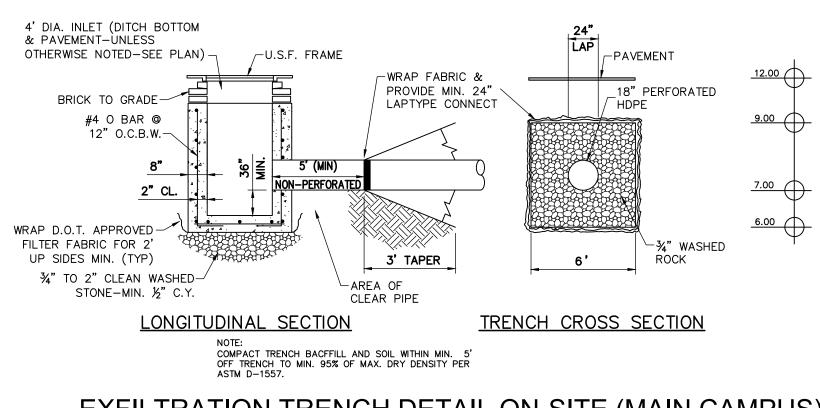
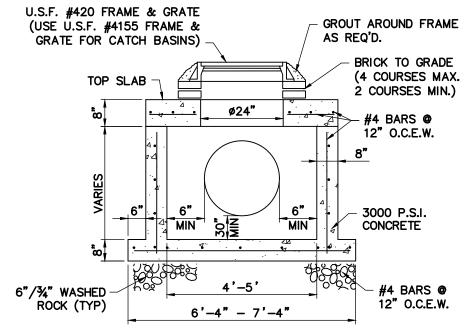


POLLUTION RETARDANT BASIN

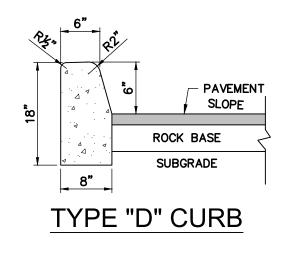
(ALL STRUCTURES SEE PLAN FOR BAFFLE LOCATION)



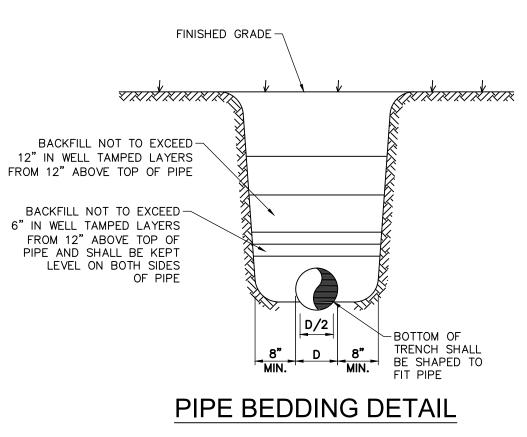
N.T.S

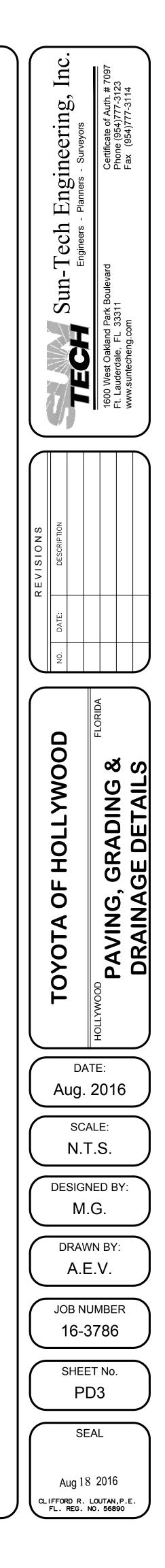


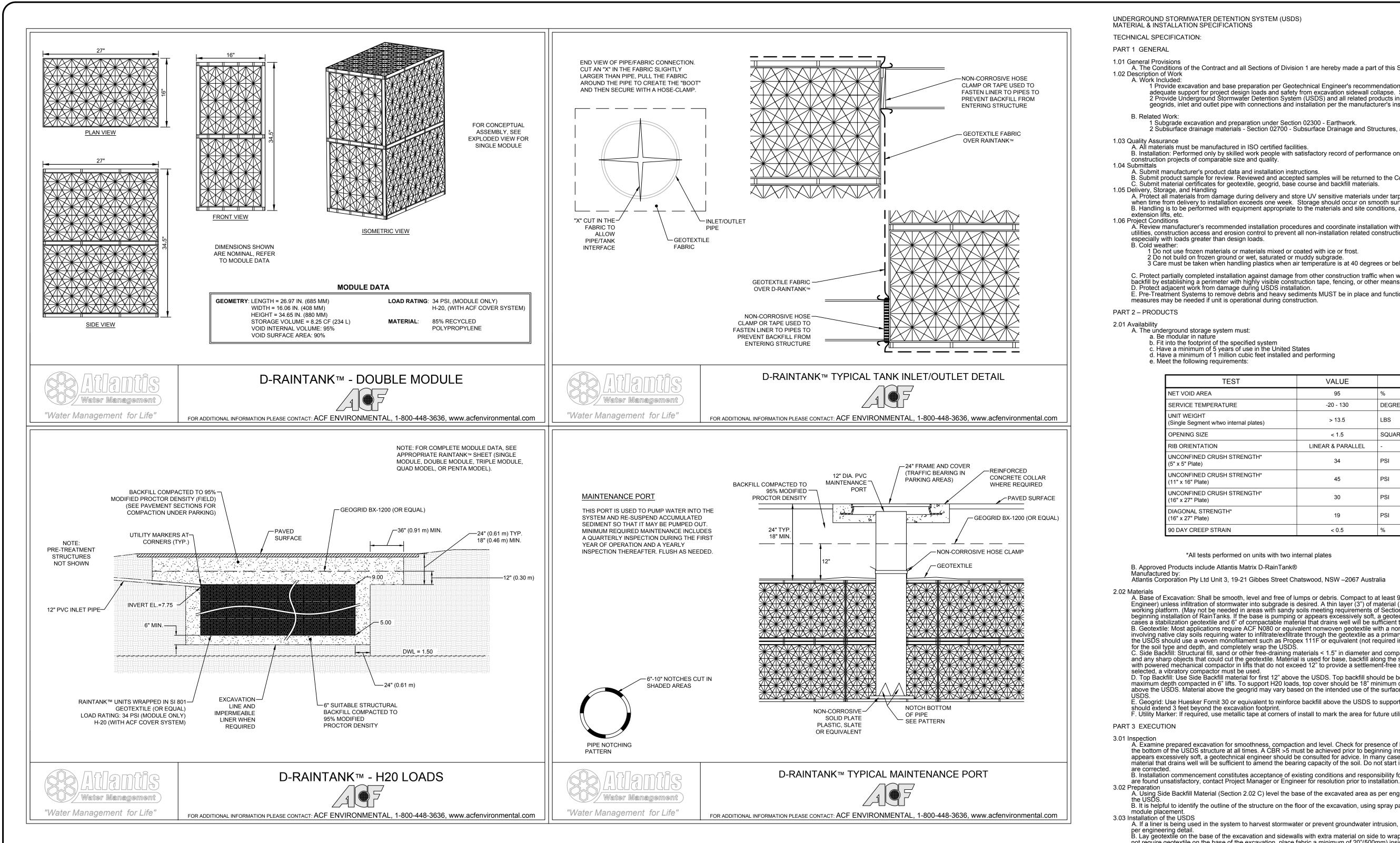




EXFILTRATION TRENCH DETAIL ON-SITE (MAIN CAMPUS)







4.01 Maintenance Requirements

are in place.

UNDERGROUND STORMWATER DETENTION SYSTEM (USDS)

A. The Conditions of the Contract and all Sections of Division 1 are hereby made a part of this Section

1 Provide excavation and base preparation per Geotechnical Engineer's recommendations and/or as shown on drawings, to provide adequate support for project design loads and safety from excavation sidewall collapse. See 2.02 Materials. 2 Provide Underground Stormwater Detention System (USDS) and all related products including storage media, geotextiles, geogrids, inlet and outlet pipe with connections and installation per the manufacturer's instructions furnished under this section.

1 Subgrade excavation and preparation under Section 02300 - Earthwork. 2 Subsurface drainage materials - Section 02700 - Subsurface Drainage and Structures, as needed.

A. All materials must be manufactured in ISO certified facilities B. Installation: Performed only by skilled work people with satisfactory record of performance on bulk earthworks, pipe, chamber, or pond/landfill construction projects of comparable size and quality.

A. Submit manufacturer's product data and installation instructions.
 B. Submit product sample for review. Reviewed and accepted samples will be returned to the Contractor.
 C. Submit material certificates for geotextile, geogrid, base course and backfill materials.

 A. Protect all materials from damage during delivery and store UV sensitive materials under tarp to protect from sunlight – including all plastics -when time from delivery to installation exceeds one week. Storage should occur on smooth surfaces, free from dirt, mud and debris. B. Handling is to be performed with equipment appropriate to the materials and site conditions, and may include hand, handcart, forklifts,

A Review manufacturer's recommended installation procedures and coordinate installation with other work affected, such as grading, excavation, utilities, construction access and erosion control to prevent all non-installation related construction traffic over the completed USDS installation,

1 Do not use frozen materials or materials mixed or coated with ice or frost. Do not build on frozen ground or wet, saturated or muddy subgrade.

3 Care must be taken when handling plastics when air temperature is at 40 degrees or below as plastic becomes brittle.

C. Protect partially completed installation against damage from other construction traffic when work is in progress and following completion of backfill by establishing a perimeter with highly visible construction tape, fencing, or other means until construction is complete. D. Protect adjacent work from damage during USDS installation. E. Pre-Treatment Systems to remove debris and heavy sediments MUST be in place and functional PRIOR to operation of the USDS. Additional

c. Have a minimum of 5 years of use in the United States d. Have a minimum of 1 million cubic feet installed and performing

TEST	VALUE	UNIT		
D AREA	95	%		
TEMPERATURE	-20 - 130	DEGREES FAHRENHEIT		
IGHT egment w/two internal plates)	> 13.5	LBS		
G SIZE	< 1.5	SQUARE INCHES		
ENTATION	LINEAR & PARALLEL	-		
INED CRUSH STRENGTH* late)	34	PSI		
INED CRUSH STRENGTH* ' Plate)	45	PSI		
INED CRUSH STRENGTH* ' Plate)	30	PSI		
AL STRENGTH* ' Plate)	19	PSI		
CREEP STRAIN	< 0.5	%		

*All tests performed on units with two internal plates

Atlantis Corporation Pty Ltd Unit 3, 19-21 Gibbes Street Chatswood, NSW -2067 Australia

. Base of Excavation: Shall be smooth, level and free of lumps or debris. Compact to at least 95% Standard Proctor Density (or as required by Engineer) unless infiltration of stormwater into subgrade is desired. A thin layer (3") of material (See Section C) is recommended to establish a level working platform. (May not be needed in areas with sandy soils meeting requirements of Section C below.) A CBR >5 must be achieved prior to beginning installation of RainTanks. If the base is pumping or appears excessively soft, a geotechnical engineer should be consulted for advice. In many cases a stabilization geotextile and 6" of compactable material that drains well will be sufficient to amend the bearing capacity of the soil. B. Geotextile: Most applications require ACF N080 or equivalent nonwoven geotextile with a nominal weight of 8 oz per square yard. Applications involving native clay soils requiring water to infiltrate/exfiltrate through the geotextile as a primary mode of introducing or removing water from the USDS should use a woven monofilament such as Propex 111F or equivalent (not required in areas with sandy soils). Geotextile should be appropriate for the soil type and depth, and completely wrap the USDS. C. Side Backfill: Structural fill, sand or other free-draining materials < 1.5" in diameter and compactable to 95%. Must be free from lumps, debris and any sharp objects that could cut the geotextile. Material is used for base, backfill along the sides of the structure, and top cover. Must be compacted with powered mechanical compactor in lifts that do not exceed 12" to provide a settlement-free surface. Even when "self-compacting" backfill materials are selected, a vibratory compactor must be used. D. Top Backfill: Use Side Backfill material for first 12" above the USDS. Top backfill should be between 12"(300mm) minimum and 36" (500mm) maximum depth compacted in 6" lifts. To support H20 loads, top cover should be 18" minimum depth (24" recommended), reinforced with a geogrid 12" above the USDS. Material above the geogrid may vary based on the intended use of the surface area. In no case should clays be used to backfill the

E. Geogrid: Use Huesker Fornit 30 or equivalent to reinforce backfill above the USDS to support H20 loads (otherwise, not required). Geogrid should extend 3 feet beyond the excavation footprint.

F. Utility Marker: If required, use metallic tape at corners of install to mark the area for future utility detection.

A. Examine prepared excavation for smoothness, compaction and level. Check for presence of high water table, which must be kept at levels below the bottom of the USDS structure at all times. A CBR >5 must be achieved prior to beginning installation of RainTanks. If the base is pumping or appears excessively soft, a geotechnical engineer should be consulted for advice. In many cases a stabilization geotextile and 6" of compactable material that drains well will be sufficient to amend the bearing capacity of the soil. Do not start installation of the USDS until unsatisfactory conditions B. Installation commencement constitutes acceptance of existing conditions and responsibility for satisfactory performance. If existing conditions

A. Using Side Backfill Material (Section 2.02 C) level the base of the excavated area as per engineering detail to establish a working platform for B. It is helpful to identify the outline of the structure on the floor of the excavation, using spray paint or chalk line, to ensure squareness during

A. If a liner is being used in the system to harvest stormwater or prevent groundwater intrusion, install per manufacturer's recommendations and B. Lay geotextile on the base of the excavation and sidewalls with extra material on side to wrap the top of the USDS. If engineering drawings do not require geotextile on the base of the excavation, place fabric a minimum of 20" (500mm) inside the excavated area to secure the material. C. Install the USDS. If RainTank modules are being used, the large side plate of the tank should be placed on the perimeter of the system. This will typically require that the two ends of the tank area will have a row of tanks placed perpendicular to all other tanks. D. Wrap the USDS in geotextile fabric from the sides and the top to prevent soil entry into the system. Overlap geotextile 12" or as recommended by manufacturer. Take great care to avoid damage to (optional) liner during placement.

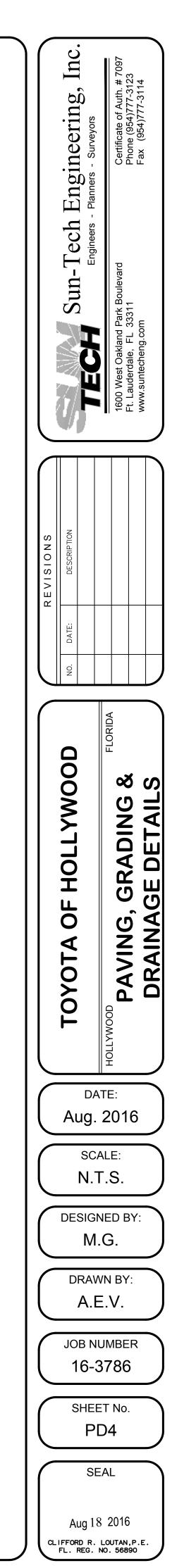
by manufacturer. Take great care to avoid damage to (optional) liner during pracement.
E. Identify locations of inlet, outlet, inspection ports, and any other penetrations of the geotextile and (optional) liner, securing pipe into boots with stainless steel pipe clamps. Support pipe in trenches during backfill operations to prevent damage to geotextile, (optional) liner or pipe.
F. Backfilling with recommended backfill, compacting in 12" (max) lifts. Place backfill CAREFULLY to avoid shoving or damaging system components. Use a powered mechanical compactor to compact backfill on structure sides with care to avoid damage to geotextile or (optional) liner.
G. Backfill above system should be compacted in 6" lifts (do not use drivable rolling compactors with 6" of cover). Alternately, a single 12" lift of backfill may be placed and compacted over the system so long as compaction goals can be obtained. When backfill reaches an elevation 12" above the USDs. USDS, place a layer of geogrid directly over the top of the backfill (required only when there will be traffic loads (H20 loads) above the system)

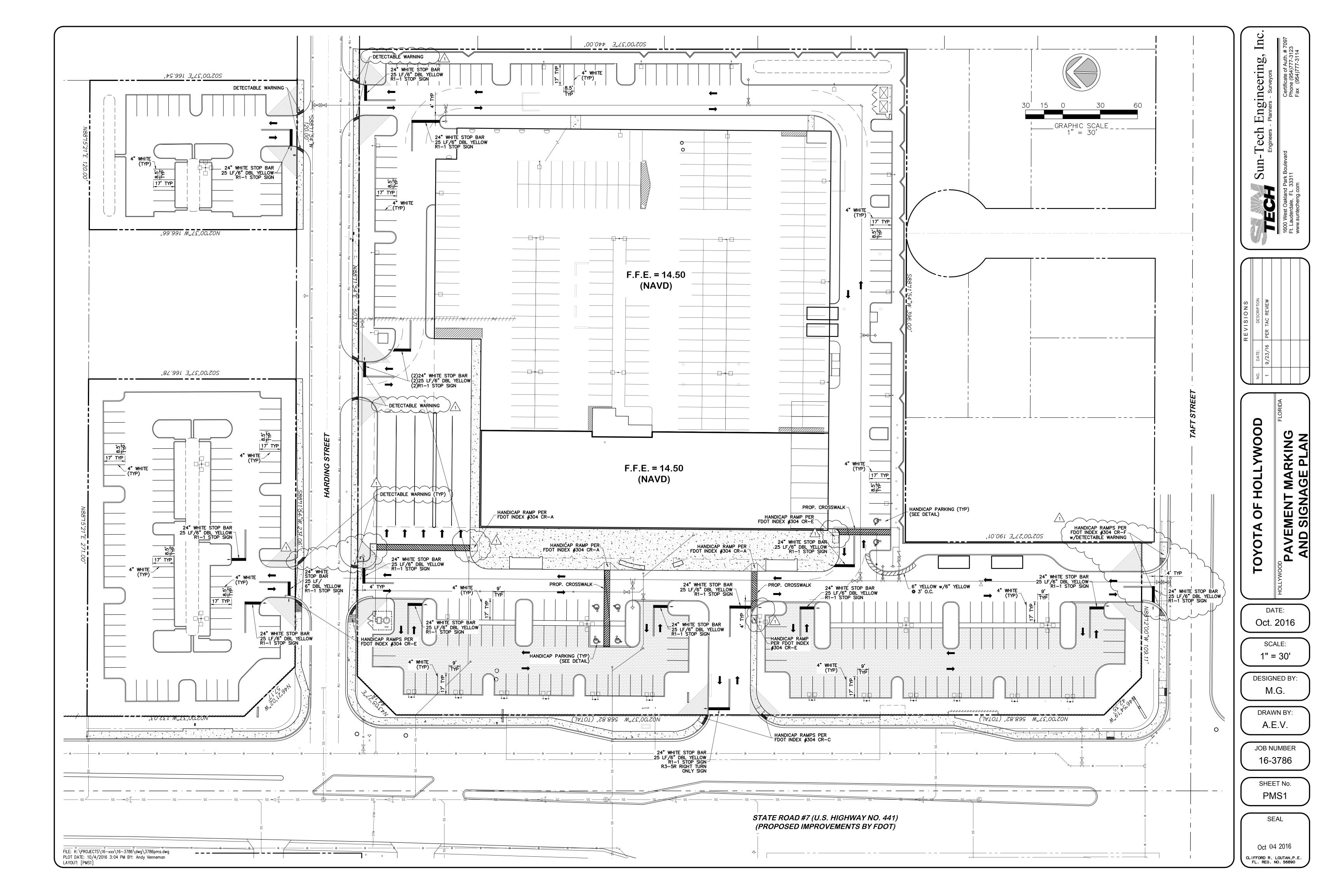
extending 3' beyond the excavation walls. H. Place sufficient sandy gravel backfill (Section 2.02 E) material over geogrid to ensure support of design loads. Place cover backfill in 6" lifts and compact with vibrating plates or walk-behind rollers (do not use drivable rolling compactors) to a minimum of 95% Standard Proctor Density, with a minimum depth of 6" (12" is recommended) and a maximum depth of 36" or as specified on engineering drawings. Take care to PLACE backfill on top of structure to avoid damage to structure, geotextile or (optional) liner, using low pressure tire or track vehicles. I. Ensure that all unrelated construction traffic be kept away from the limits of excavation until the project is complete and final surface materials J. Place surfacing materials, such as groundcovers (no large trees), or paving materials over the structure with care to avoid displacement of cover fill and damage to surrounding areas. K. Backfill depth over USDS must be a MINIMUM of 18" prior to Proof Rolling area directly above USDS. If backfill depth is less than 18" and proof

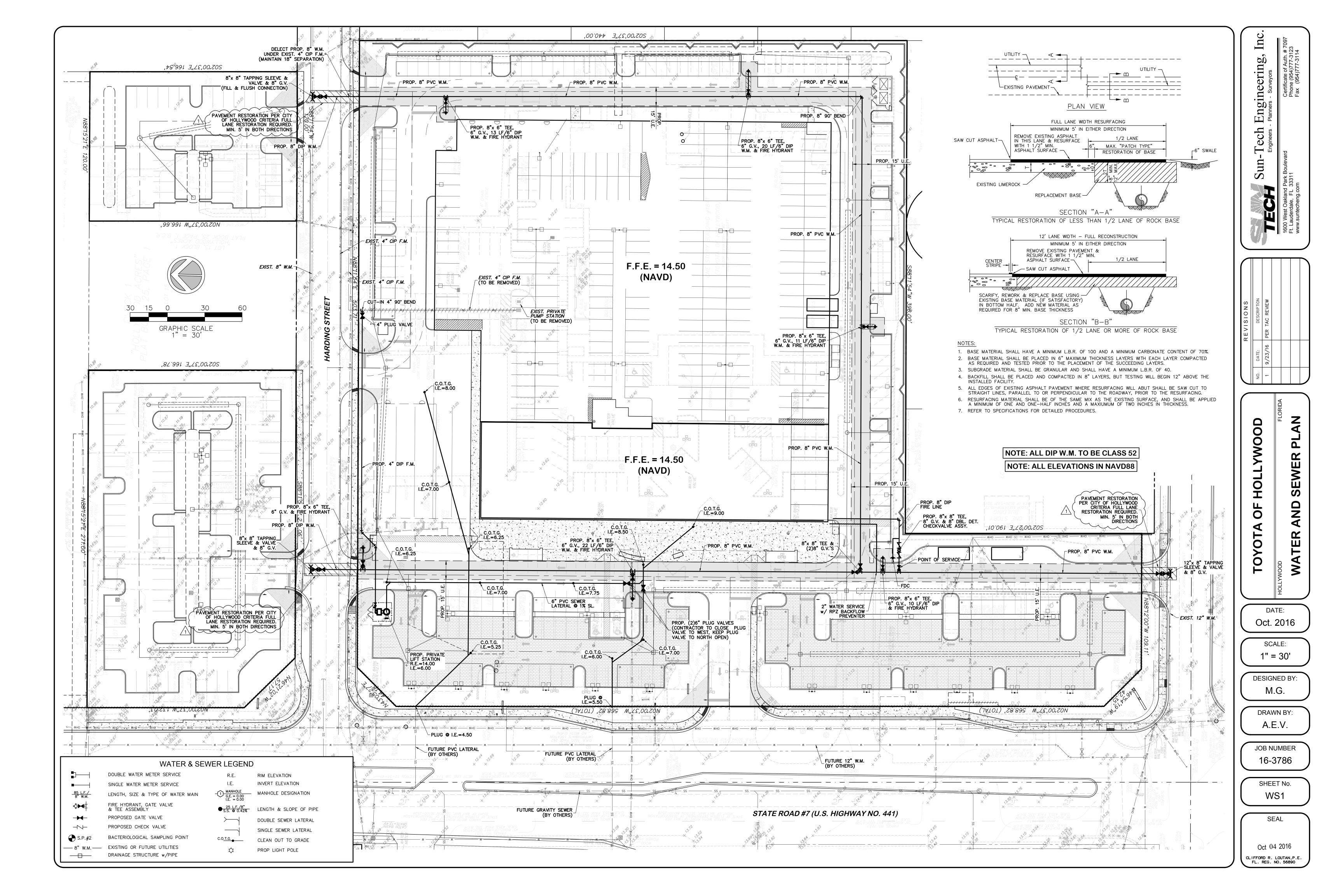
3.04 Cleaning A. Perform cleaning during the installation of work and upon completion of the work. Remove from site all excess materials, debris, and equipment. Repair any damage to adjacent materials and surfaces resulting from installation of this work. PART 4 – USING THE SYSTEM

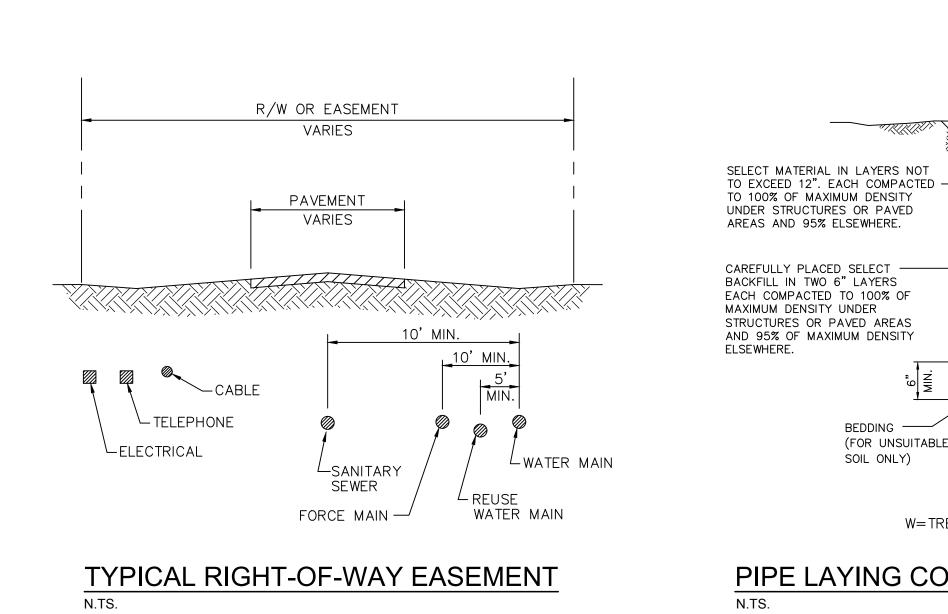
rolling is required, contact engineer or manufacturer's representative for assistance.

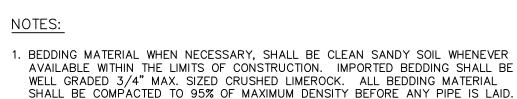
A. Maintenance efforts should be focused on pretreatment systems. Ensuring these structures are clean and functioning properly will prevent contamination of the USDS system and stormwater released from the site. Pre-treatment systems should be inspected as directed by the manufacturer (for proprietary systems) or at least quarterly for the first year of use, yearly thereafter. Maintain as needed using acceptable practices or following manufacturer's guidelines (for proprietary systems). B. If the USDS system included Inspection or Maintenance Ports, it will be necessary to inspect the system for accumulation of sediments. This is done by removing the cap of the port and using a measuring device long enough to reach the bottom of the USDS and stiff enough to push through the Lose sediments, allowing a depth measurement. C. If sediment has accumulated beyond an acceptable level, it will be necessary to flush the USDS. This can be done by pumping water into the Maintenance Port or adjacent structure, allowing the turbulent flows through the USDS to re-suspend the fine sediments. If multiple Maintenance Ports have been installed, water should be pumped into each port to maximize flushing efficiency. Sediment-laden water can be pumped out and either captured for disposal or filtered through a Dirtbag[™] if permitted by the locality.









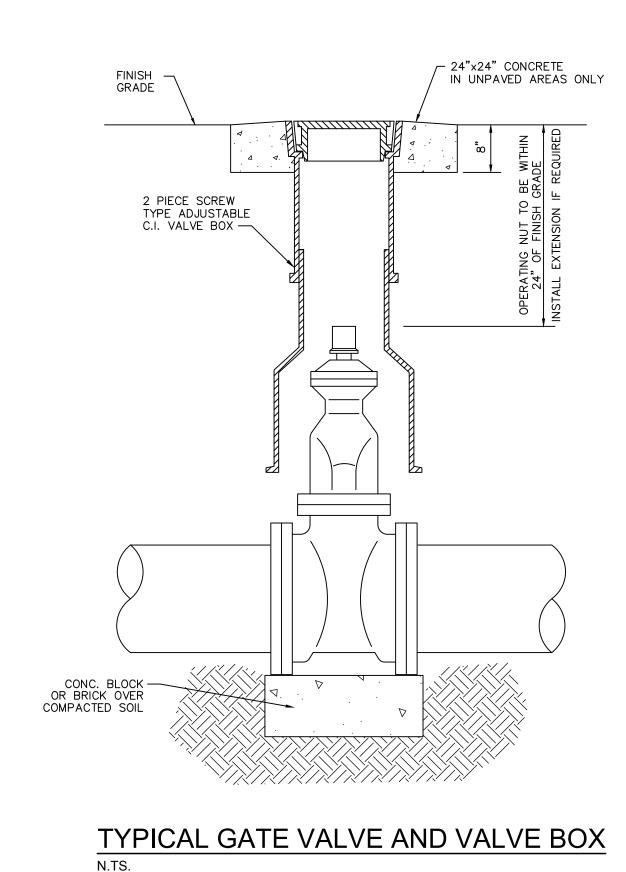


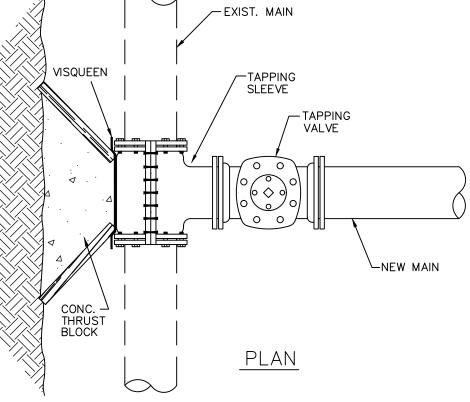
BEDDING

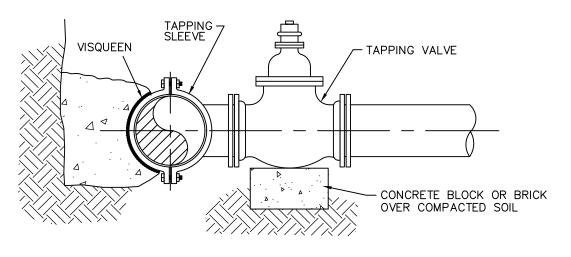
SOIL ONLY)

(FOR UNSUITABLE

2. DENSITY TESTING SHALL BE IN ACCORDANCE WITH AASHTO T-180 AND ASTM D-3017.

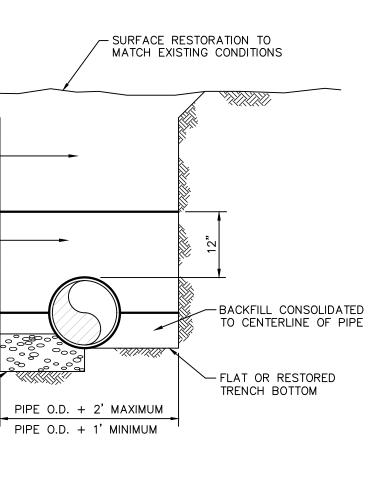






NOTES: 1. NOTIFY THE CITY OF HOLLYWOOD 48 HOURS IN ADVANCE OF PROPOSED TAP. 2. TAPPING MUST BE DONE IN THE PRESENCE OF AN AUTHORIZED CITY REPRESENTATIVE.

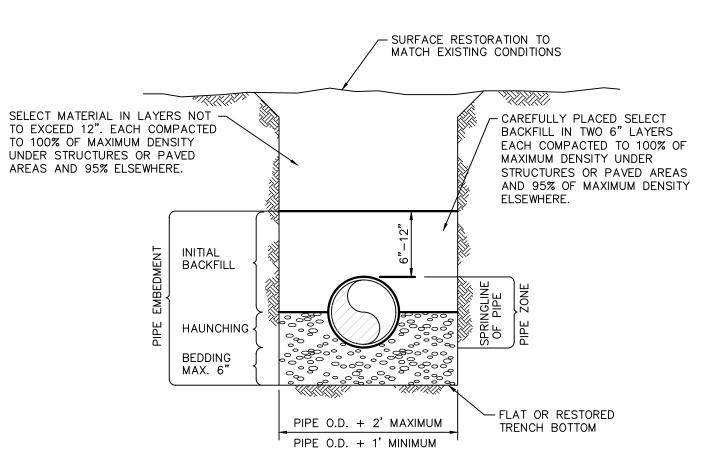




D=DIAMETER W=TRENCH WIDTH

PIPE LAYING CONDITION TYPICAL SECTION

AVAILABLE WITHIN THE LIMITS OF CONSTRUCTION. IMPORTED BEDDING SHALL BE



PVC PIPE LAYING CONDITION TYPICAL SECTION N.TS.

NOTES:

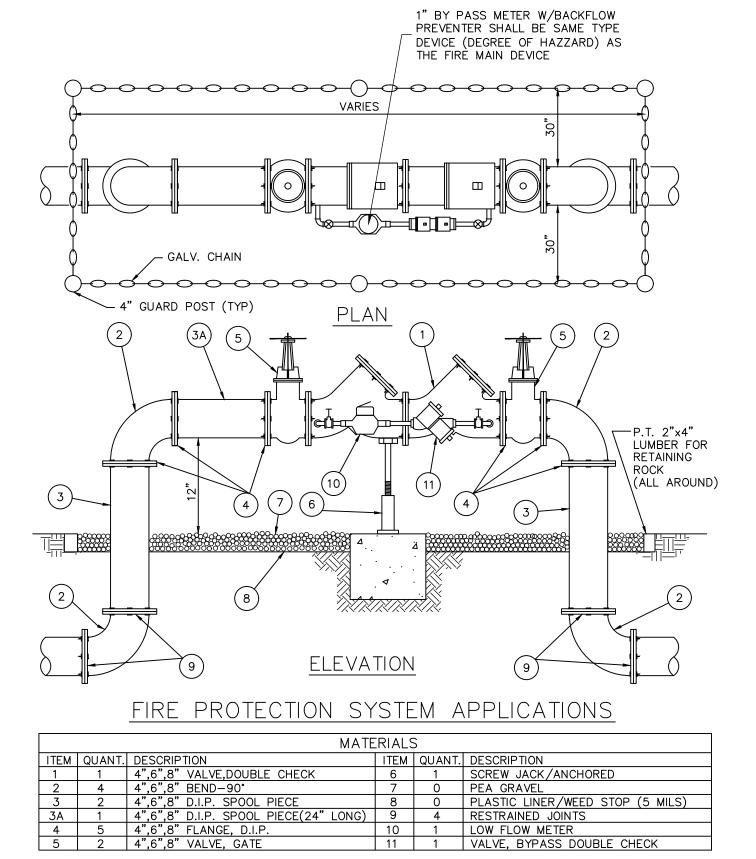
NOTES:

- 1. BEDDING AND HAUNCHING SHALL BE WELL GRADED 3/4" MAX. SIZE CRUSHED LIMEROCK
- 2. DENSITY TESTING SHALL BE IN ACCORDANCE WITH AASHTO T-180 AND ASTM D-3017.



ELEVATION

TYPICAL TAPPING SLEEVE AND VALVE



BACKFLOW PREVENTER 90° N.TS.

DOUBLE DETECTOR CHECK VALVE AND

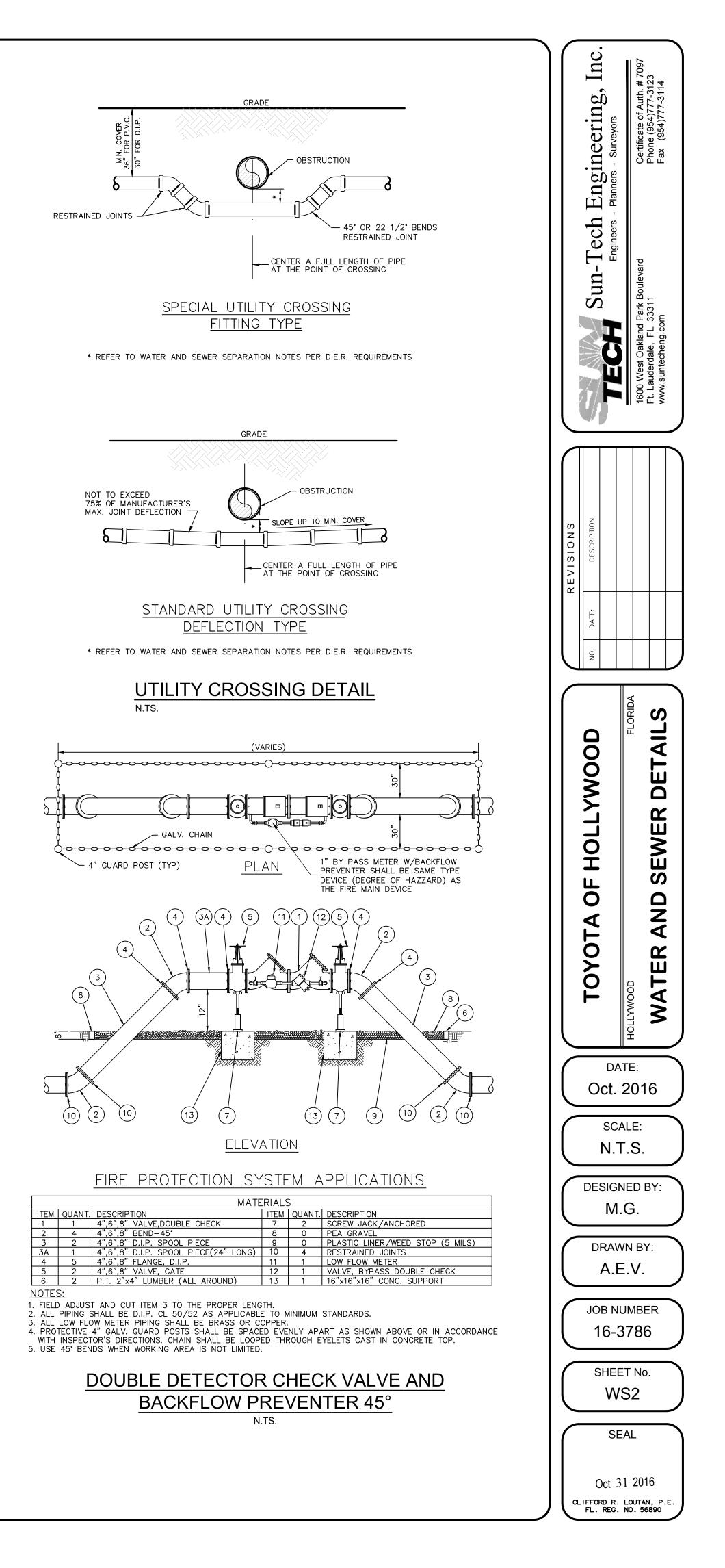
WITH INSPECTOR'S DIRECTIONS. CHAIN SHALL BE LOOPED THROUGH EYELETS CAST IN CONCRETE TOP.

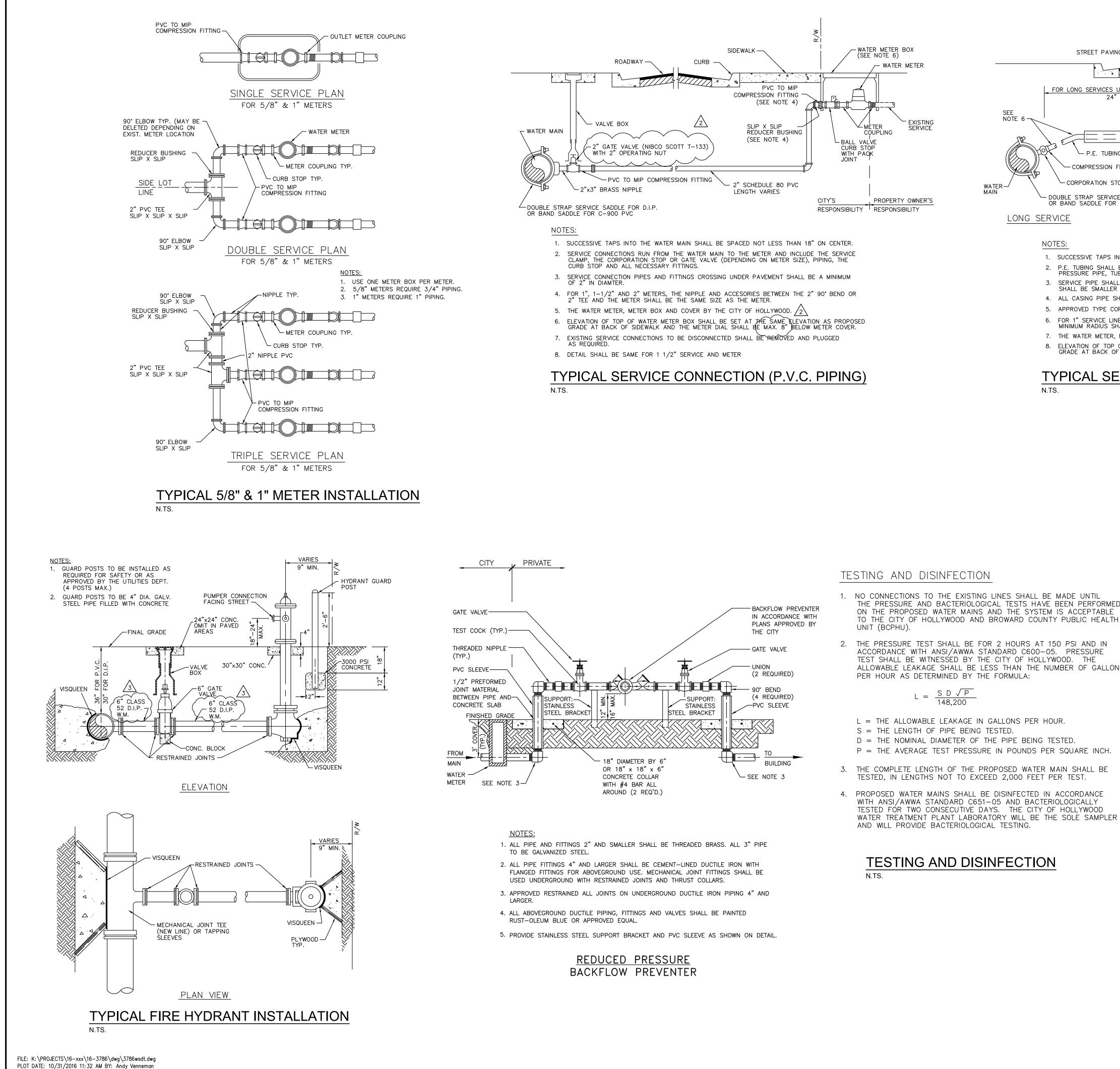
4. PROTECTIVE 4" GALV. GUARD POSTS SHALL BE SPACED EVENLY APART AS SHOWN ABOVE OR IN ACCORDANCE

1. FIELD ADJUST AND CUT ITEM 3 TO THE PROPER LENGTH. 2. ALL PIPING SHALL BE D.I.P. CL 50/52 AS APPLICABLE TO MINIMUM STANDARDS.

3. ALL LOW FLOW METER PIPING SHALL BE BRASS OR COPPER.

5. USE 45' BENDS WHEN WORKING AREA IS NOT LIMITED.

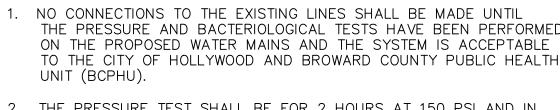




LAYOUT: [WS3]

- THE PRESSURE AND BACTERIOLOGICAL TESTS HAVE BEEN PERFORMED ON THE PROPOSED WATER MAINS AND THE SYSTEM IS ACCEPTABLE TO THE CITY OF HOLLYWOOD AND BROWARD COUNTY PUBLIC HEALTH
- ACCORDANCE WITH ANSI/AWWA STANDARD C600-05. PRESSURE TEST SHALL BE WITNESSED BY THE CITY OF HOLLYWOOD. THE ALLOWABLE LEAKAGE SHALL BE LESS THAN THE NUMBER OF GALLONS

2. THE PRESSURE TEST SHALL BE FOR 2 HOURS AT 150 PSI AND IN

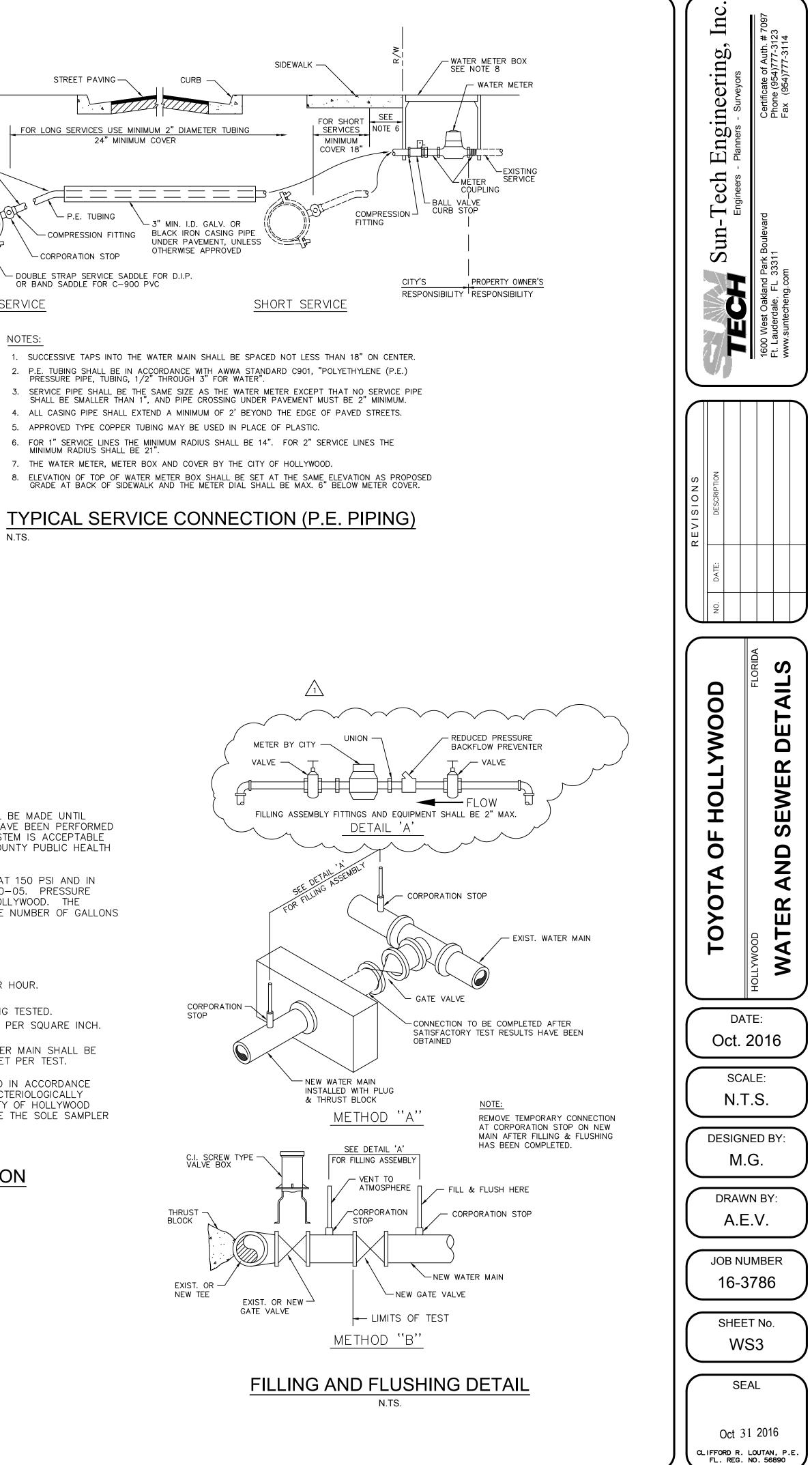




STREET PAVING FOR LONG SERVICES USE MINIMUM 2" DIAMETER TUBING 24" MINIMUM COVER SEE NOTE 6 - P.E. TUBING - COMPRESSION FITTING CORPORATION STOP WATER-MAIN DOUBLE STRAP SERVICE SADDLE FOR D.I.P.
 OR BAND SADDLE FOR C-900 PVC LONG SERVICE

NOTES:

N.TS.



<u>NOTE:</u>

ALL PIPE AND PIPE FITTINGS INSTALLED UNDER THIS PROJECT WILL BE COLOR CODED OR MARKED IN ACCORDANCE WITH SUBPARAGRAPH 62-555.320(21)(B)3, F.A.C., USING BLUE AS A PREDOMINANT COLOR. (UNDERGROUND PLASTIC PIPE WILL BE SOLID-WALL BLUE PIPE, WILL HAVE A CO-EXTRUDED BLUE EXTERNAL SKIN, OR WILL BE WHITE OR BLACK PIPE WITH BLUE STRIPES INCORPORATED INTO, OR APPLIED TO, THE PIPE WALL; AND UNDERGROUND METAL OR CONCRETE PIPE WILL HAVE BLUE STRIPES APPLIED TO THE PIPE WALL. PIPE STRIPED DURING MANUFACTURING OF THE PIPE WILL HAVE CONTINUOUS STRIPES THAT RUN PARALLEL TO THE AXIS OF THE PIPE, THAT ARE LOCATED AT NO GREATER THAN 90-DEGREE INTERVALS AROUND THE PIPE, AND THAT WILL REMAIN INTACT DURING AND AFTER INSTALLATION OF THE PIPE. IF TAPE OR PAINT IS USED TO STRIPE PIPE DURING INSTALLATION OF THE PIPE, THE TAPE OR PAINT WILL BE APPLIED IN A CONTINUOUS LINE THAT RUNS PARALLEL TO THE AXIS OF THE PIPE AND THAT IS LOCATED ALONG THE TOP OF THE PIPE; FOR PIPE WITH AN INTERNAL DIAMETER OF 24 INCHES OR GREATER, TAPE OR PAINT WILL BE APPLIED IN CONTINUOUS LINES ALONG EACH SIDE OF THE PIPE AS WELL AS ALONG THE TOP OF THE PIPE. ABOVEGROUND PIPE WILL BE PAINTED BLUE OR WILL BE COLOR CODED OR MARKED LIKE UNDERGROUND PIPE.)

WHERE IT IS NOT POSSIBLE TO MAINTAIN A VERTICAL DISTANCE OF 18" IN PARALLEL INSTALLATION, THE WATER MAINS SHALL BE CONSTRUCTED OF D.I.P. AND THE GRAVITY SEWER SHALL BE CONSTRUCTED OF PVC SDR-26 OR C-900 WITH A MINIMUM VERTICAL

CORROSION SHALL BE USED AS DETERMINED BY THE DESIGN.

CLEARANCE OF 12".

PIPE MARKINGS:

IN CASES WHERE IT IS NOT POSSIBLE TO MAINTAIN A 10 FOOT HORIZONTAL SEPARATION, THE WATER MAIN MUST BE LAID IN A SEPARATE TRENCH OR ON AN UNDISTURBED EARTH SHELF LOCATED ON ONE SIDE OF THE SEWER OR FORCE MAIN AT SUCH AN ELEVATION THAT THE BOTTOM OF THE WATER MAIN IS AT LEAST 18 INCHES ABOVE THE TOP OF THE SEWER.

A MINIMUM 10 FOOT HORIZONTAL SEPARATION SHALL BE MAINTAINED BETWEEN ANY TYPE OF SEWER AND WATER MAIN IN PARALLEL INSTALLATIONS WHENEVER POSSIBLE.

ALL DIP SHALL BE CLASS 51 OR HIGHER. ADEQUATE PROTECTIVE MEASURES AGAINST

WHERE A NEW PIPE CONFLICTS WITH AN EXISTING PIPE WITH LESS THAN 18 INCHES VERTICAL

CLEARANCE, THE NEW PIPE SHALL BE ARRANGED TO MEET THE CROSSING REQUIREMENTS

ABOVE.

SANITARY SEWERS AND FORCE MAINS SHOULD CROSS UNDER WATER MAINS WHENEVER

POSSIBLE. SANITARY SEWERS AND FORCE MAINS CROSSING WATER MAINS SHALL BE LAID TO

PROVIDE A MINIMUM VERTICAL DISTANCE OF 18 INCHES BETWEEN THE INVERT OF THE UPPER

WHERE SANITARY SEWERS & GRAVITY SEWERS MUST CROSS A WATER MAIN WITH LESS THAN

18 INCHES VERTICAL DISTANCE, THE GRAVITY SEWER SHALL BE PVC SDR 26 AND THE

WATER MAIN SHALL BE CONSTRUCTED OF DUCTILE IRON PIPE (DIP) AT THE CROSSING. SUFFICIENT LENGTHS OF DIP MUST BE USED TO PROVIDE A MINIMUM SEPARATION OF 10

ALL CROSSING SHALL BE ARRANGED SO THAT THE SEWER PIPE JOINTS AND THE WATER MAIN

PIPE JOINTS ARE EQUIDISTANT FROM THE POINT OF CROSSING (PIPES CENTERED ON THE CROSSING).

FEET BETWEEN ANY TWO JOINTS. ALL JOINTS ON THE WATER MAIN WITHIN 20 FEET OF THE CROSSING MUST BE MECHANICALLY RESTRAINED. A MINIMUM VERTICAL CLEARANCE OF 12 INCHES MUST BE MAINTAINED AT ALL CROSSINGS. MAINTAIN 18" VERTICAL SEPARATION

3. 2" TAP IN BOTTOM OF CAP ON MAIN.

NOTES:

WATER SYS

250.

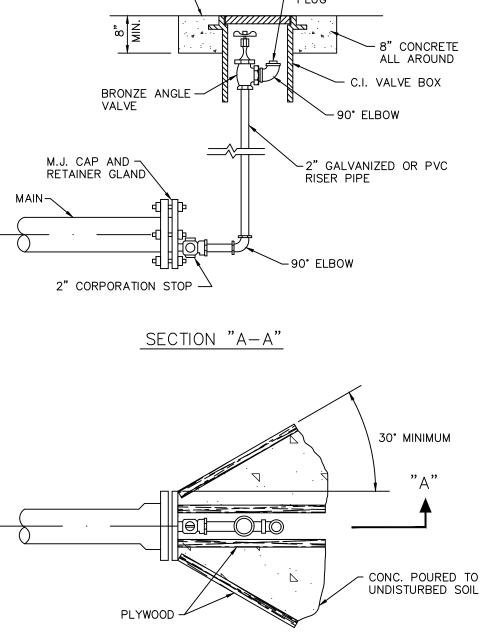
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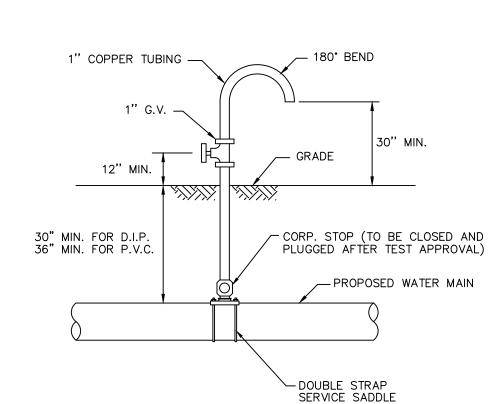
HYDRANTS.

N.TS.

2" CORPORATION STOP

FINISHED GRADE -BRONZE ANGLE VALVE





SAMPLING POINT SHALL NOT BE REMOVED UNTIL APPROVAL IS OBTAINED FROM

SAMPLE POINT DETAIL

WATER AND SEWER SEPARATION NOTES:

BETWEEN WATER AND FORCE MAIN.

PIPE AND THE CROWN OF THE LOWER PIPE WHENEVER POSSIBLE.

N.TS.

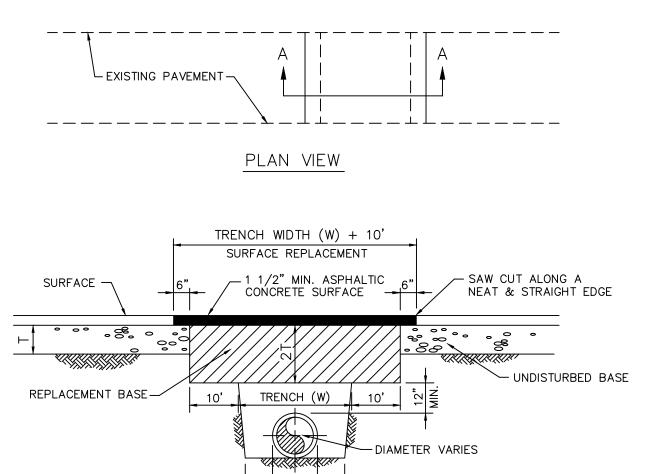
BROWARD COUNTY PUBLIC HEALTH UNIT (B.C.P.H.U.)

PLAN VIEW

1. PIPE JOINT COMPOUND SHALL BE APPLIED TO MALE THREADS ONLY. 2. COAT ALL EXPOSED THREADS WITH BITUMASTIC BEFORE BACKFILLING.

4. PLYWOOD AND CONCRETE TO HAVE A HEIGHT EQUAL TO THE DIAMETER OF THE PIPE.

2" TERMINAL BLOW-OFF DETAIL



SECTION "A-A"

2" IVARIES I

NOTES:

- 1. REPLACED BASE MATERIAL OVER TRENCH SHALL BE TWICE THE THICKNESS OF THE ORIGINAL BASE, MINIMUM 12", MAXIMUM 18". 2. BASE MATERIAL SHALL BE PLACED IN 6" MAXIMUM (LOOSE MEASUREMENT)
- LAYERS AND EACH LAYER THOROUGHLY ROLLED OR TAMPED TO 98% OF MAXIMUM DENSITY, PER AASHTO T-180.
- 3. ASPHALT CONCRETE PAVEMENT JOINTS SHALL BE MECHANICALLY SAWED. 4. SURFACE TREATED PAVEMENT JOINTS SHALL BE LAPPED AND FEATHERED
- 5. SURFACE MATERIAL SHALL BE CONSISTENT WITH THE EXISTING SURFACE
- 6. BASE MATERIAL SHALL HAVE A MINIMUM LBR OF 100 AND A MINIMUM CARBONATE CONTENT OF 70%.
- 7. IF THE TRENCH IS FILLED TEMPORARILY, IT SHALL BE COVERED WITH A 2" ASPHALTIC CONCRETE PATCH TO KEEP THE FILL MATERIAL FROM RAVELING UNTIL REPLACED WITH A PERMANENT PATCH.
- 8. BACKFILL SHALL BE IN ACCORDANCE WITH DETAIL OF PIPE LAYING CONDITION TYPICAL SECTION, EXCEPT AS SHOWN ABOVE.

FLEXIBLE PAVEMENT RESTORATION PERPENDICULAR UTILITY INSTALLATION (CITY STREETS ONLY)

N.TS.

STEM NOTES:	
$\frac{1}{2}$	

1. DUCTILE IRON WATER MAIN PIPE SHALL CONFORM TO THE REQUIREMENTS OF A.N.S.I./ A.W.W.A. C-151/A 21.51-02 AND LINED AND COATED PER A.N.S.I./A.W.W.A. C-104/A-214-03. 20" AND SMALLER PIPE SHALL BE PRESSURE CLASS 350; 24" AND LARGER, PIPE SHALL BE PRESSURE CLASS

2. ALL P.V.C. MAINS SHALL BE SERIES 1120, CLASS 150 (DR 18) PRESSURE PIPE, CONFORMING TO A.N.S.I./A.W.W.A. C-900-97, OR LATEST REVISION, AND SHALL HAVE PUSH ON JOINTS, AND IRON PIPE

3. FITTINGS SHALL BE DUCTILE IRON MEETING A.N.S.I./A.W.W.A. C153/21.00 AND SHALL BE COATED WITH 6 TO 8 MIL. THICKNESS COAL TAR EPOXY CONFORMING TO THE REQUIREMENTS OF A.N.S.I./A.W.W.A. C550-05 AND C116/A21.53-00.

4. RESTRAINED JOINT PIPE SHALL BE USED FOR ALL BENDS, TEES, CROSSES, PLUGS, AND FIRE

GENERAL PRESSURE PIPE NOTES

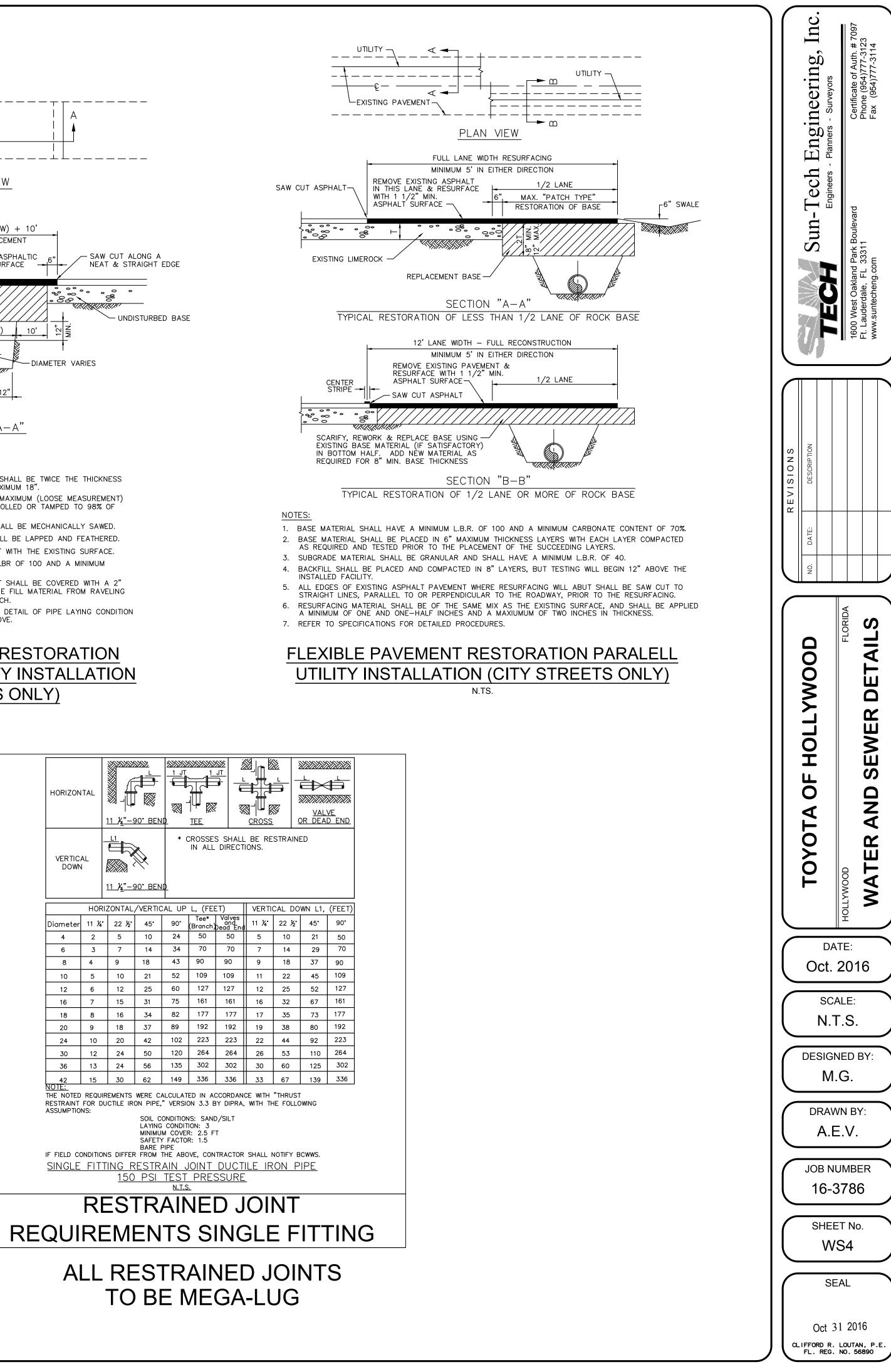
1. THERE SHALL BE 36" MINIMUM COVER FROM FINISHED GRADE TO TOP OF PIPE. 2. ALL TRENCHING, PIPE-LAYING, BACKFILL, PRESSURE TESTING MUST COMPLY WITH ALL APPLICABLE

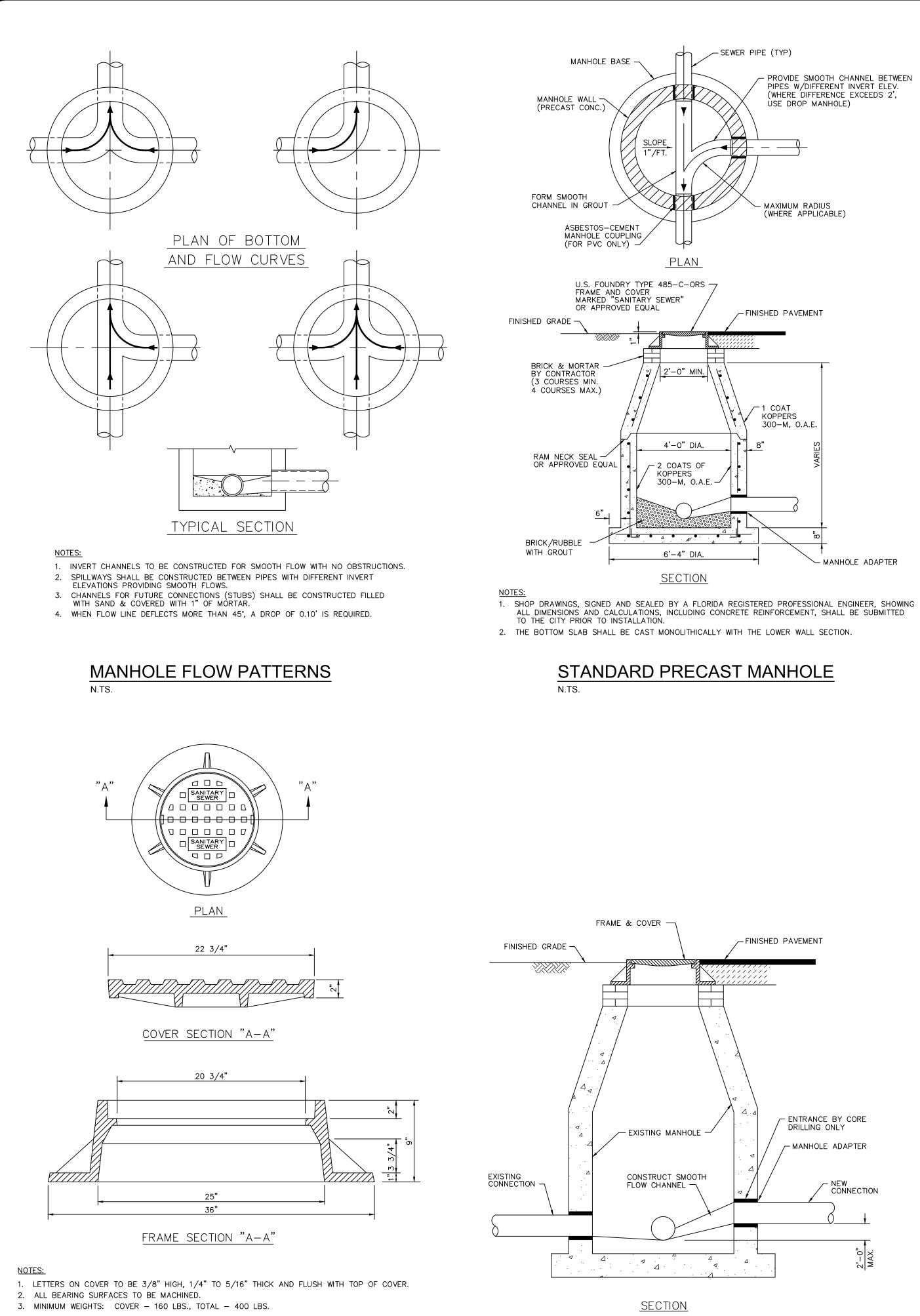
FEDERAL, STATE, COUNTY, CITY AND HEALTH DEPARTMENT STANDARDS AND REGULATIONS. 3. THESE NOTES AND THE DETAIL SHEETS THAT ACCOMPANY THESE PLANS ARE TYPICAL IN NATURE. THE MAIN PLANS AND SPECIFICATIONS PROVISIONS WILL TAKE PRECEDENCE OVER ANY NOTE CONTAINED ON THIS OR OTHER DETAIL SHEETS.

4. THE CONTRACTOR MUST POT HOLE AND VERIFY THE LOCATION, SIZE, AND ELEVATION OF EXISTING PRESSURE MAINS BEFORE MAKING A TIE-IN.

		AL			Ŧ		S SHALL DIRECT		
			<u>11 ¼"-9</u>					1	
		HOR	IZONTAL	/VERTIC	AL UP	· · ·		VERTIO	
	Diameter	11 <i>¼</i> *	22 ½ [•]	45 '	90 °	Tee* (Branch)	Valves and Dead End	11 ¼*	22
	4	2	5	10	24	50	50	5	1
	6	3	7	14	34	70	70	7	1
	8	4	9	18	43	90	90	9	1
	10	5	10	21	52	109	109	11	2
	12	6	12	25	60	127	127	12	2
	16	7	15	31	75	161	161	16	3
	18	8	16	34	82	177	177	17	3
	20	9	18	37	89	192	192	19	3
	24	10	20	42	102	223	223	22	4
	30	12	24	50	120	264	264	26	5
	36	13	24	56	135	302	302	30	6
	42	15	30	62	149	336	336	33	6
	NOTE: THE NOTED RESTRAINT ASSUMPTIO	FOR D NS:	UCTILE IR	ON PIPE, SOIL C LAYING MINIMU SAFET BARE I	VERSIC ONDITION CONDIT M COVEF FACTOI PIPE	DN 3.3 E IS: SANE ION: 3 R: 2.5 F R: 1.5	BY DIPRA, D/SILT T	WITH TH	ie f
	SINGLE		<u>TING</u> <u>150</u>	ESTR	AIN J	IOINT PRES		<u>ile ir</u>	
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ALL RESTRAINED JOINTS TO BE MEGA-LUG





MANHOLE FRAME AND COVER N.TS.

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N.TS.

NEW CONNECTION TO EXISTING MANHOLE

WYE BRANCH CONNECTION N.TS.

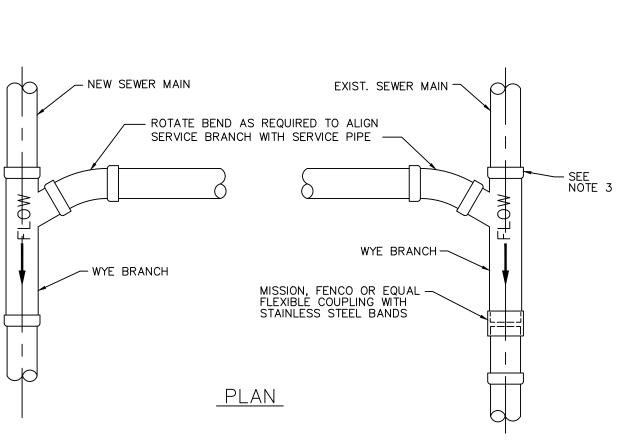
2. USE RISER CONNECTIONS WHERE INVERT OF SEWER IS GREATER THAN 7'-0" DEEP.

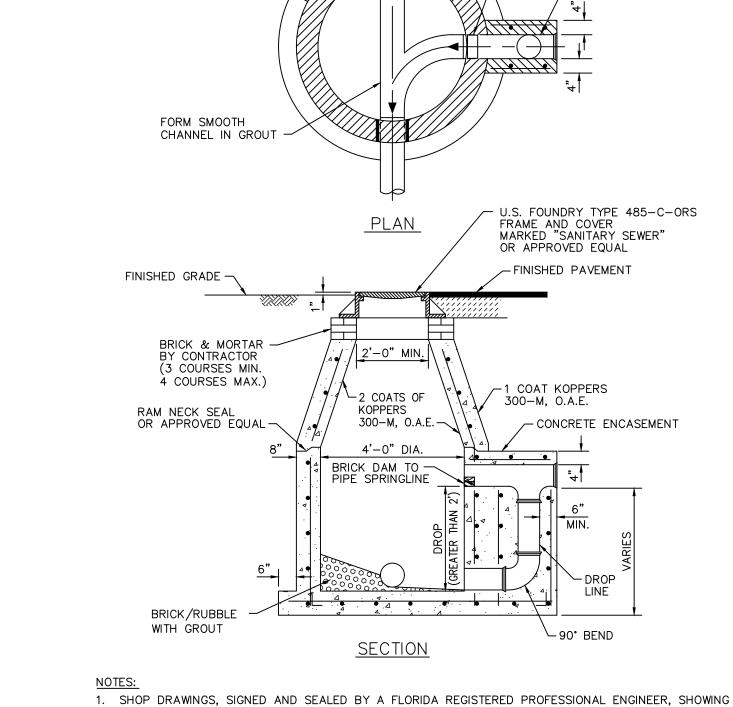
3. WHERE BELL OF WYE AND SPIGOT OF EXISTING MAIN ARE NOT COMPATIBLE, USE A SECOND FLEXIBLE COUPLING.

1. SINGLE SERVICE CONNECTIONS SHALL USE 6" PIPE AND FITTINGS.

NOTES:

ALTERNATE ADDITIONAL RISE AND BEND WHERE REQUIRED BY GREATER SEWER DEPTH SLOPE UP TO PL ELEVATION





TO THE CITY PRIOR TO INSTALLATION.

N.TS.

MANHOLE BASE -

MANHOLE WALL -

(PRECAST CONC.)

- U.S. FOUNDRY TYPE 485–C–ORS FRAME AND COVER MARKED "SANITARY SEWER" OR APPROVED EQUAL - FINISHED PAVEMENT -1 COAT KOPPERS 300-M, O.A.E. - CONCRETE ENCASEMENT MIN

ALL DIMENSIONS AND CALCULATIONS, INCLUDING CONCRETE REINFORCEMENT, SHALL BE SUBMITTED

DROP MANHOLE DETAIL

2. THE BOTTOM SLAB SHALL BE CAST MONOLITHICALLY WITH THE LOWER WALL SECTION.

3. ALL PIPE WORK SHALL BE CLASS 50 D.I.P., ENCASED FITTINGS SHALL BE CLASS 350.

- SEWER PIPE (TYP)

- BRICK DAM GROUTED IN PLACE

TFF

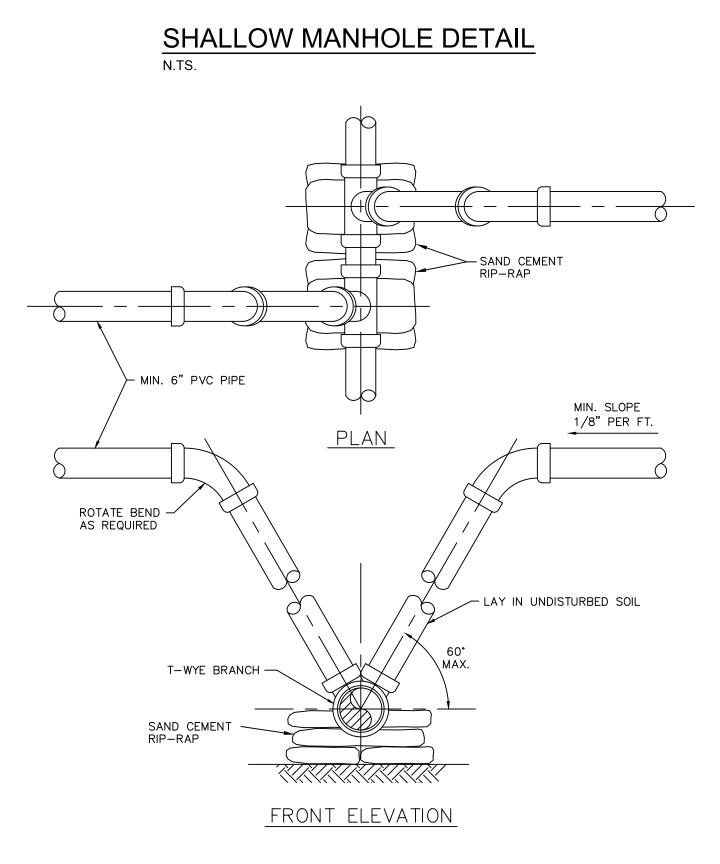
─90° BEND

U.S. FOUNDRY TYPE 485–C–ORS – FRAME AND COVER MARKED "SANITARY SEWER" OR APPROVED EQUAL - FINISHED PAVEMENT FINISHED GRADE BRICK & MORTAR BY CONTRACTOR (3 COURSES MIN. '-0" MIN. 4 COURSES MAX.) • • • RAM NECK SEAL 4'-0" DIA. OR APPROVED EQUAL -1 COAT 2 COATS OF KOPPERS KOPPERS 300-M, O.A.E. 300-M, O.A.E. - MANHOLE ADAPTER • BRICK/RUBBLE WITH GROUT

<u>SECTION</u>

NOTES:

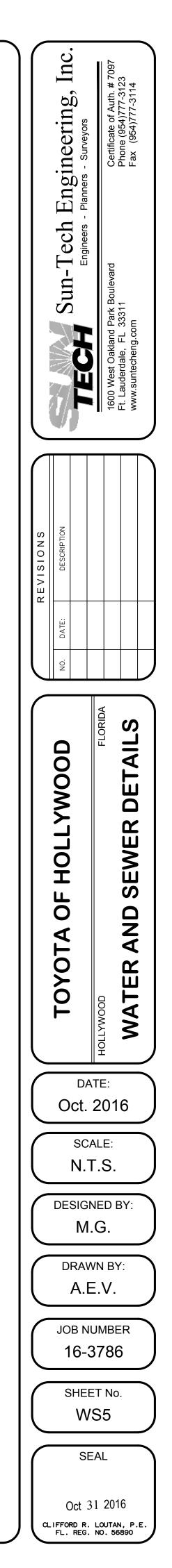
1. SHOP DRAWINGS, SIGNED AND SEALED BY A FLORIDA REGISTERED PROFESSIONAL ENGINEER, SHOWING ALL DIMENSIONS AND CALCULATIONS, INCLUDING CONCRETE REINFORCEMENT, SHALL BE SUBMITTED TO THE CITY PRIOR TO INSTALLATION. 2. THE BOTTOM SLAB SHALL BE CAST MONOLITHICALLY WITH THE LOWER WALL SECTION.

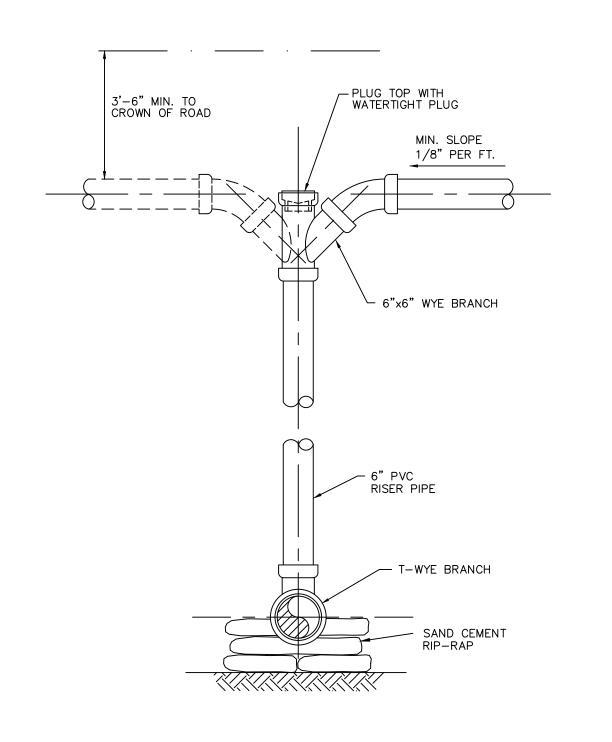


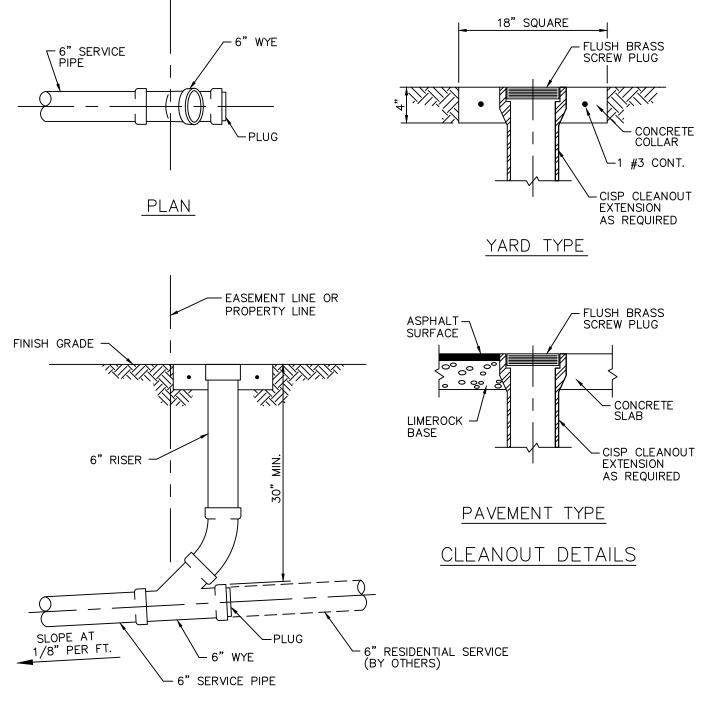
NOTES:

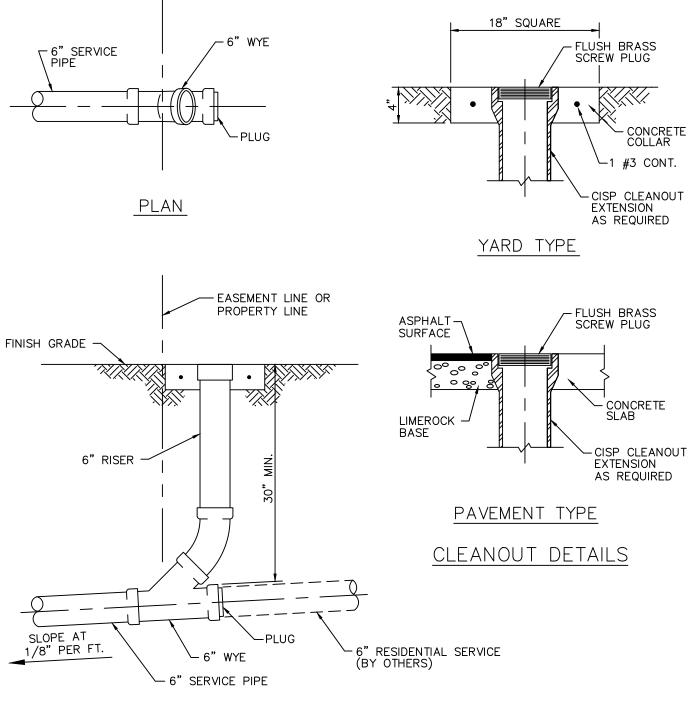
- 1. MODIFIED RISER CONNECTION TO BE USED ONLY WHEN DIRECTED BY THE CITY.
- 2. 3/4" DRAINFIELD LIMEROCK MAY BE USED AS BEDDING OVER UNDISTURBED SOIL WITH P.V.C. PIPE.
- 3. SINGLE SERVICE CONNECTIONS SHALL USE 6" PIPE AND FITTINGS.

SANITARY SEWER LATERAL MODIFIED RISER N.TS.









NOTES:

RISER CONNECTION TO BE USED ONLY WHEN INVERT OF SEWER IS GREATER THAN 7'-0" OR WHEN DIRECTED BY THE CITY.

2. SINGLE SERVICE CONNECTIONS SHALL USE 6" PIPE AND FITTINGS.

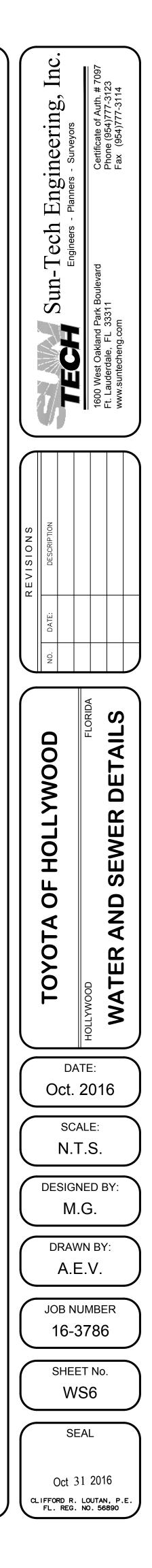
SANITARY SEWER LATERAL VERTICAL RISER N.TS.

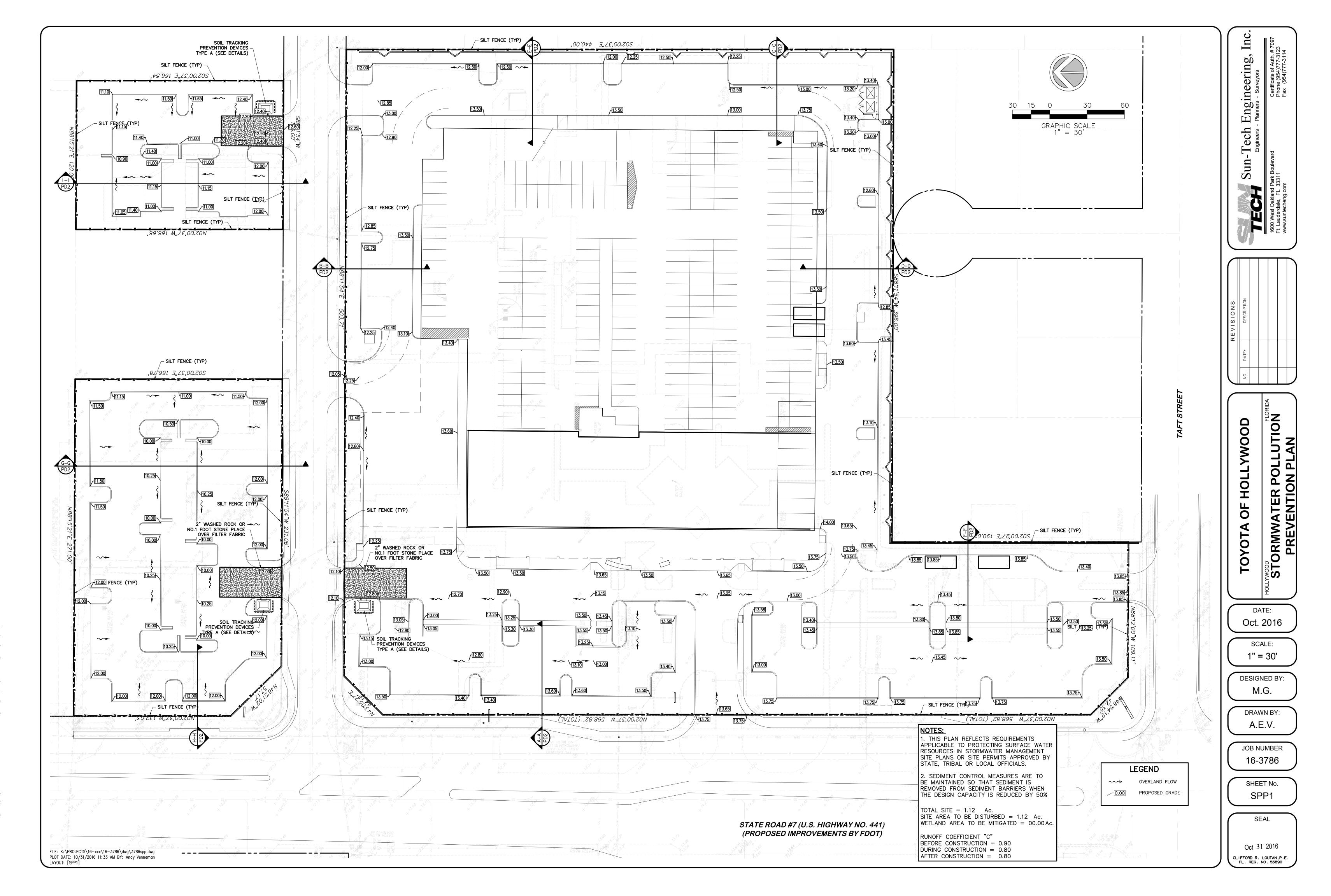
FILE: K:\PROJECTS\16-xxx\16-3786\dwg\3786wsdt.dwg PLOT DATE: 10/31/2016 11:33 AM BY: Andy Venneman LAYOUT: [WS6]

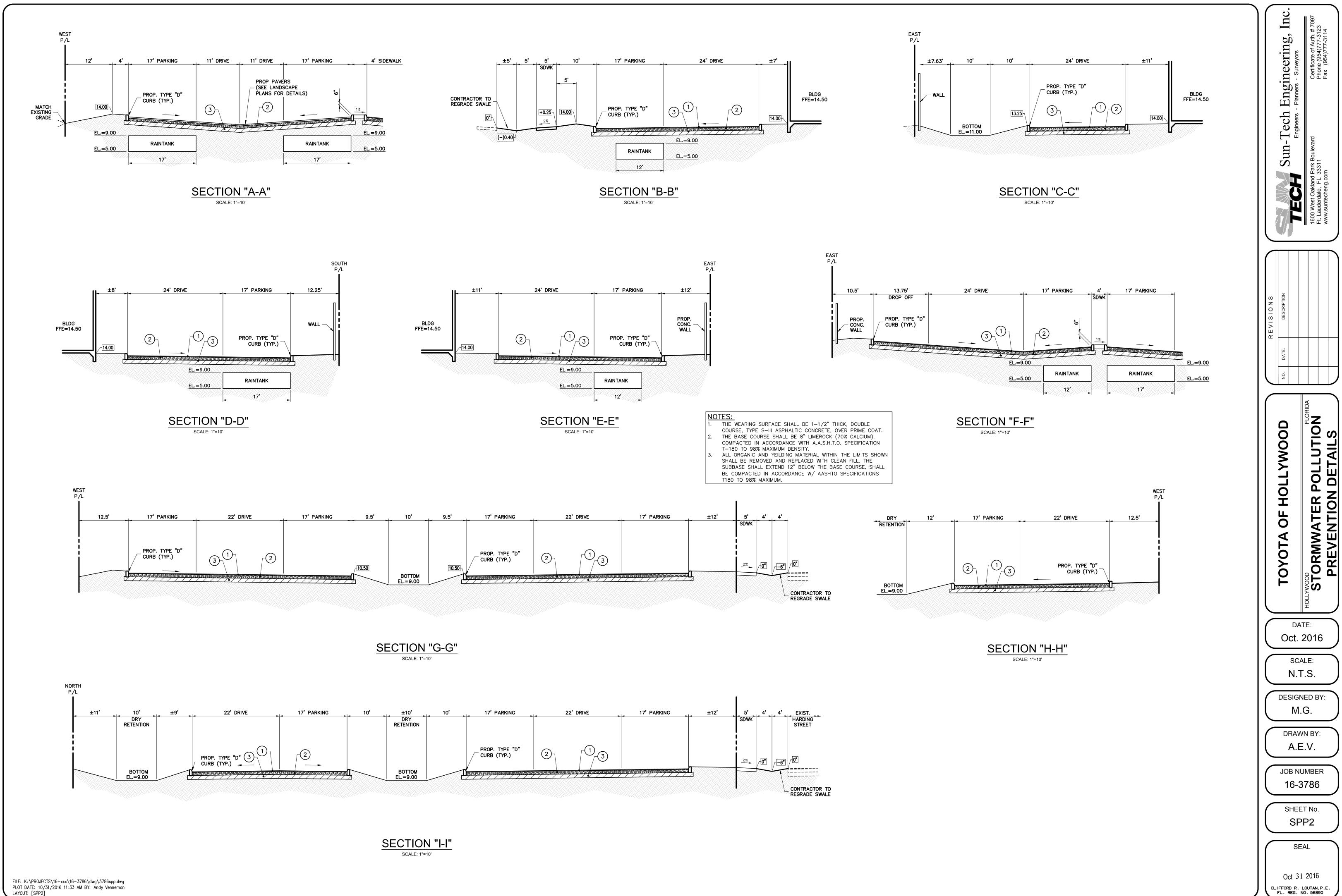
ELEVATION

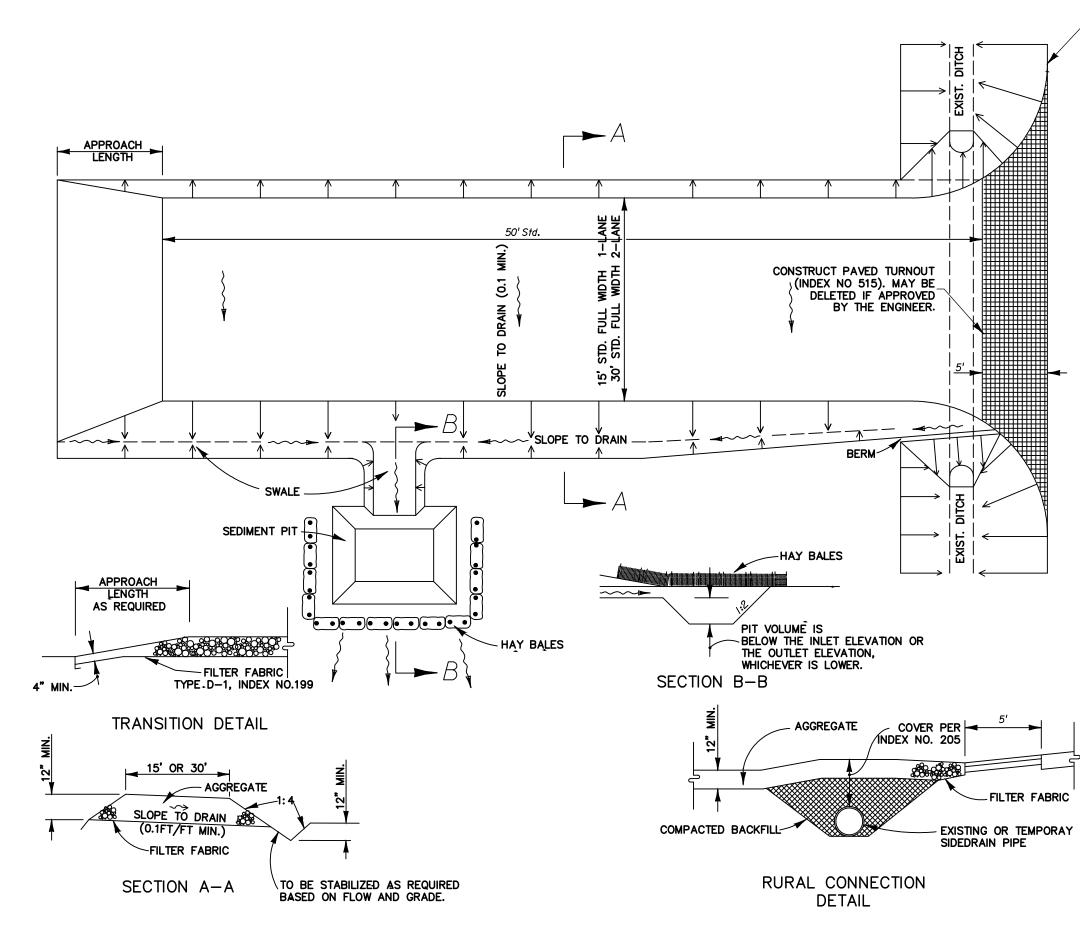
SINGLE SERVICE CONNECTION











SOIL TRACKING PREVENTION DEVICE - TYPE 'A'

EXISTING ROADWAY

GENERAL NOTES

- 1. A SOIL TRACKING PREVENTION DEVICE (S.T.P.D.) SHALL BE CONSTRUCTED AT ALL LOCATIONS DESIGNATED BY THE ENGINEER FOR POINTS OF EGRESS FROM UNSTABILIZED AREAS OF THE PROJECT TO PUBLIC ROADS WHERE OFFSITE TRACKING OF MUD COULD OCCUR. TRAFFIC FROM UNSTABILIZED AREAS OF THE CONSTRUCTION PROJECT SHALL BE DIRECTED THRU A S.T.P.D. BARRIERS, FLAGGING, OR OTHER POSITIVE MEANS SHALL BE USED AS REQUIRED TO LIMIT AND DIRECT VEHICULAR EGRESS ACROSS THE S.T.P.D.
- 2. THE CONTRACTOR MAY PROPOSE AN ALTERNATIVE TECHNIQUE TO MINIMIZE OFFSITE TRACKING OF SEDIMENT. THE ALTERNATIVE MUST BE REVIEWED AND APPROVED BY THE ENGINEER APPROVED TO ITS USE.
- 3. ALL MATERIALS SPILLED, DROPPED OR TRACKED ONTO PUBLIC ROADS (INCLUDING THE S.T.P.D. AGGREGATE AND CONSTRUCTION MUD) SHALL BE REMOVED DAILY, OR MORE FREQUENTLY IF SO DIRECTED BY THE ENGINEER.
- AGGREGATES SHALL BE AS DESCRIBED IN SECTION 901 EXCLUDING 901-2.3. AGGREGATES SHALL BE FDOT SIZE #1. IF THIS SIZE IS NOTE AVAILABLE, THE NEXT AVAILABLE SMALLER SIZE AGGREGATE MAY BE SUBSTITUTED WITH THE APPROVAL OF THE ENGINEER. SIZES CONTAINING EXCESSIVE SMALL AGGREGATE WILL TRACK OFF THE PROJECT AND ARE UNSUITABLE.
- 5. THE SEDIMENT PIT SHOULD PROVIDE A RETENTION VOLUME OF 3600 CUBIC FEET/ACRE OF SURFACE AREA DRAINING TO THE PIT. WHEN THE S.T.P.D. IS ISOLATED FROM OTHER DRAINAGE AREAS, THE FOLLOWING PIT VOLUMES WILL SATISFY THIS REQUIREMENT:
 - 15'x50'=100 FT 30'x50'=200 FT
- AS AN OPTION TO THE SEDIMENT PIT, THE WIDTH OF THE SWALE BOTTOM CAN BE INCREASED TO OBTAIN THE VOLUME. WHEN THE SEDIMENT PIT OR SWALE VOLUME HAS BEEN REDUCED TO ONE HALF, IT SHALL BE CLEANED. WHEN A SWALE IS USED, HAY BALES OR SILT FENCE SHALL BE PLACE ALONG THE ENTIRE LENGTH.
- THE SWALE DITCH DRAINING THE S.T.P.D, SHALL HAVE A 0.2% MINIMUM AND A 1.0% MAXIMUM GRADE ALONG THE S.T.P.D. AND TO THE SEDIMENT PIT.
- 6. MITERED END SECTIONS ARE NOT REQUIRED WHEN THE SIDE DRAIN PIPE SATISFIES THE CLEAR ZONE REQUIREMENTS.
- 8. THE S.T.P.D. SHALL BE MAINTAINED IN A CONDITION THAT WILL ALLOW IT TO PERFORM ITS FUNCTION. TO PREVENT OFFSITE TRACKING, THE S.T.P.D. SHALL BE RINSED (DAILY WHEN IN USE) TO MOVE ACCUMULATED MUD DOWNWARD THRU THE STONE. ADDITIONAL STABILIZATION OF THE VEHICULAR ROUTE LEADING TO THE S.T.P.D. MAY BE REQUIRED TO LIMIT THE MUD TRACKED.
- 9. A S.T.P.D. SHALL BE PAID FOR UNDER THE CONTRACT UNIT PRICE FOR SOIL TRACKING PREVENTION DEVICE, EA. THE UNIT PRICE SHALL CONSTITUTE FULL COMPENSATION FOR CONSTRUCTION, MAINTENANCE, REPLACEMENT OF MATERIALS, REMOVAL, AND RESTORATION OF THE AREA UTILIZED FOR THE S.T.P.D.; INCLUDING BUT NOT LIMITED TO EXCAVATION, GRADING, TEMPORARY PIPE (INCLUDING M.E.S. WHEN REQUIRED), FILTER FABRIC, AGGREGATE, PAVED TURNOUT (INCLUDING ASPHALT AND BASE CONSTRUCTION). DITCH STABILIZATION, APPROACH ROUTE STABILIZATION, SEDIMENT REMOVAL AND DISPOSAL, WATER, RINSING AND CLEANING OF THE S.T.P.D. AND CLEANING OF PUBLIC ROADS, GRASSING AND SOD. HAY BALES SHALL BE PAID FOR UNDER THE CONTRACT UNIT PRICE FOR HAY OR STRAW BALE, EA. SILT FENCE SHALL BE PAID FOR UNDER THE CONTRACT UNIT PRICE FOR STAKED SILT FENCE, LF.
- 10. THE NOMINAL SIZE OF A STANDARD S.T.P.D. IS 15'x50' UNLESS OTHERWISE SHOWN IN THE PLANS. IF THE VOLUME OF ENTERING AND EXITING VEHICLES WARRANT, A 30' WDTH S.T.P.D. MAY BE USED IF APPROVED BY THE ENGINEER. WHEN A DOUBLE WDTH (30') S.T.P.D. IS USED, THE PAY QUANTITY SHALL BE 2 FOR EACH LOCATION.

