

THAT PART OF LOT 15, BLOCK 12 OF SAID "HOLLYWOOD SOUTH SIDE ADDITION NO. 2" WHICH IS INCLUDED IN THE EXTERNAL AREA FORMED BY A 15 FOOT RADIUS ARC WHICH IS TANGENT TO THE SOUTH LINE OF SAID LOT 15 AND TANGENT TO A LINE WHICH IS 7 FEET WEST OF AND PARALLEL WITH THE EAST LINE OF SAID LOT 15.

C-1.0

EROSION SEDIMENTATION CONTROL NOTES

MAINTENANCE OF EROSION CONTROL MEASURES IS OF PARAMOUNT IMPORTANCE. THE CONTRACTOR IS RESPONSIBLE FOR COMPLYING WITH ALL EROSION CONTROL MEASURES SHOWN ON THE PLANS. THE EROSION CONTROL SYSTEM DESCRIBED WITHIN THE CONSTRUCTION DOCUMENTS SHOULD BE CONSIDERED TO REPRESENT THE MINIMUM ACCEPTABLE STANDARDS FOR THIS PROJECT. ADDITIONAL EROSION CONTROL MEASURES MAY BE REQUIRED DEPENDENT UPON THE STAGE OF CONSTRUCTION, THE SEVERITY OF THE RAINFALL EVENT AND/OR AS DEEMED NECESSARY AS A RESULT OF ON-SITE INSPECTIONS BY THE OWNER, THEIR REPRESENTATIVES OR THE JURISDICTIONAL AUTHORITIES. THESE ADDITIONAL MEASURES SHALL BE INSTALLED AT NO ADDITIONAL COST TO THE OWNER. IT IS THE CONTRACTOR'S ULTIMATE RESPONSIBILITY TO ASSURE THAT THE STORM WATER DISCHARGE FROM THE SITE DOES NOT EXCEED THE TOLERANCES ESTABLISHED BY ANY OF THE JURISDICTIONAL AUTHORITIES. REFERENCE THE EROSION CONTROL PLAN AND DETAILS.

THIS PLAN HAS BEEN PREPARED TO ENSURE COMPLIANCE WITH RULES OF THE FLORIDA DEPARTMENT OF ENVIRONMENTAL PROTECTION, CHAPTER 17-25-FAC.

SITE

A. SITE CONDITIONS

1. SITE OPERATOR (CONTRACTOR) SHALL PREPARE A CONSTRUCTION SCHEDULE THAT INCLUDES THE DATE GRADING WILL BEGIN AND THE EXPECTED DATE OF STABILIZATION AND SHALL INCLUDE THE CONSTRUCTION SCHEDULE AS PART OF THIS STORM WATER POLLUTION PREVENTION PLAN (SWPPP).

B. SEQUENCE OF IMPLEMENTATION OF CONTROLS

1. INSTALLATION OF CONTROL MEASURES (CONSTRUCTION ENTRANCE, SILT FENCE, FILTER SACKS, GUTTER EEL) ETC.
2. DEMOLITION AND CLEARING, GRUBBING AND EXCAVATION.
3. CONSTRUCTION ACTIVITIES ASSOCIATED WITH THE BUILDING, SITE DEVELOPMENT, AND INFRASTRUCTURE NECESSARY TO SERVE THE PROPOSED PROJECT.
4. FINAL STABILIZATION.

C. PHASING OF CONTROL MEASURES

1. PHASE 1- INITIAL PHASE
THIS PHASE INCLUDES EROSION AND SEDIMENT CONTROL MEASURES REQUIRED BEFORE AND DURING DEMOLITION STAGE OF CONSTRUCTION.

- THIS PHASE INCLUDES:
- ESTABLISH STABILIZED CONSTRUCTION ENTRANCE.
 - INSTALLATION OF SILT FENCE.
 - INSTALLATION OF SILK DIKE.
 - INSTALL FILTER SACKS IN OFF-SITE INLET GRATES AND PLACE GUTTER EEL ON TOP OF GRATES.
 - INSTALL FILTER SACKS IN EXISTING ONSITE STORM INLETS DURING DEMOLITION.

2. PHASE 2- INTERMEDIATE PHASE
THIS PHASE INCLUDES EROSION AND SEDIMENT CONTROL MEASURES REQUIRED DURING SITE GRADING FROM INITIAL GRADING THROUGH CURB AND GUTTER PHASE.

- THIS PHASE INCLUDES:
- INSTALLATION OF FILTER SACKS IN PROPOSED INLETS.
 - MAINTAIN ALL MEASURES IN PHASE 1

3. PHASE 3- FINAL PHASE
THIS PHASE INCLUDES EROSION AND SEDIMENT CONTROL MEASURES REQUIRED DURING FINAL BUILDING CONSTRUCTION, PAVING, AND FINAL LANDSCAPE.

- THIS PHASE INCLUDES:
- REMOVAL OF GUTTER EELS FROM RIGHT OF WAY INLETS.
 - MAINTAIN REMAINING MEASURES FROM PHASES 1 AND 2.

CONTROLS

EROSION AND SEDIMENT CONTROL MEASURES ARE TO BE PLACED PRIOR TO, OR AS THE FIRST STEP IN CONSTRUCTION. THE CONTRACTOR IS RESPONSIBLE FOR COMPLYING WITH ALL EROSION CONTROL MEASURES SHOWN ON THE PLANS. THE EROSION CONTROL SYSTEM DESCRIBED WITHIN THE CONSTRUCTION DOCUMENTS SHOULD BE CONSIDERED TO REPRESENT THE MINIMUM ACCEPTABLE STANDARDS FOR THIS PROJECT. ADDITIONAL EROSION CONTROL MEASURES MAY BE REQUIRED DEPENDENT UPON THE STAGE OF CONSTRUCTION, THE SEVERITY OF THE RAINFALL EVENT AND/OR AS DEEMED NECESSARY AS A RESULT OF ON-SITE INSPECTIONS BY THE OWNER, THEIR REPRESENTATIVES, OR THE APPLICABLE JURISDICTIONAL AUTHORITIES. THESE ADDITIONAL MEASURES (IF NEEDED) SHALL BE INSTALLED AT NO ADDITIONAL COST TO THE OWNER. IT IS NOTED THAT THE MEASURES IDENTIFIED ON THIS PLAN ARE ONLY SUGGESTED BEST MANAGEMENT PRACTICES (BMPs). THE CONTRACTOR SHALL PROVIDE POLLUTION PREVENTION AND EROSION CONTROL MEASURES AS SPECIFIED IN FDOT INDEXES #100 THROUGH #102 AND AS NECESSARY FOR EACH SPECIFIC APPLICATION. IT IS THE CONTRACTOR'S ULTIMATE RESPONSIBILITY TO ASSURE THAT THE STORM WATER DISCHARGE FROM THE SITE DOES NOT EXCEED THE TOLERANCES ESTABLISHED BY ANY OF THE APPLICABLE JURISDICTIONAL AUTHORITIES.

A. EROSION AND SEDIMENT CONTROLS

GENERAL EROSION CONTROL

- A. CLEARING AND GRUBBING OPERATIONS SHALL BE CONTROLLED SO AS TO MINIMIZE UNPROTECTED ERODIBLE AREAS EXPOSED TO WEATHER. GENERAL EROSION CONTROL BMPs SHALL BE EMPLOYED TO MINIMIZE SOIL EROSION AND OFF-SITE SEDIMENTATION. WHILE THE VARIOUS TECHNIQUES REQUIRED WILL BE SITE AND PLAN SPECIFIC, THEY SHOULD BE EMPLOYED PRIOR TO ANY CONSTRUCTION ACTIVITY.
- B. EXCAVATED MATERIAL WILL NOT BE DEPOSITED IN LOCATIONS WHERE IT COULD BE WASHED AWAY BY HIGH WATER OR STORM WATER RUNOFF. STOCKPILED MATERIAL SHALL BE COVERED OR ENCLOSED WITH SEDIMENT CONTAINMENT DEVICES.
- C. STABILIZATION MEASURES SHALL BE IMMEDIATELY INITIATED FOR EROSION AND SEDIMENT CONTROL ON DISTURBED AREAS. CLEARED SITE DEVELOPMENT AREAS WHICH WILL REMAIN AT ROUGH GRADE FOR 7 DAYS OR MORE SHOULD BE STABILIZED IMMEDIATELY BY COVERING WITH ADEQUATE AMOUNTS OF HAY, OVER SEEDED AND PERIODICALLY WATERED SUFFICIENT TO STABILIZE THE TEMPORARY GROUND COVER, OR BY THE USE OF AN APPROPRIATE ALTERNATIVE BMP.
- D. ALL GRASS SLOPES CONSTRUCTED STEEPER THAN 4H:1V SHALL BE SODDED IMMEDIATELY AFTER FINAL GRADE IS ESTABLISHED. PERMANENT SOIL EROSION CONTROL MEASURES FOR ALL SLOPES, CHANNELS, DITCHES OR ANY DISTURBED LAND AREAS SHALL BE COMPLETED IMMEDIATELY AFTER FINAL GRADING.
- E. EROSION AND SEDIMENT CONTROL MEASURES ARE TO BE PROVIDED AND INSTALLED PRIOR TO COMMENCEMENT OF CONSTRUCTION. SEDIMENT CONTROL CONSISTS OF SILT FENCING AND FLOATING TURBIDITY BARRIERS PER FDOT INDEX NO. 102 AND 103. EROSION CONTROL CONSISTS OF SEEDING AND MULCHING, SODDING, WETTING SURFACES, PLACEMENT OF COARSE AGGREGATE, TEMPORARY PAVING.
- F. MAINTAIN TEMPORARY EROSION CONTROL SYSTEMS AS DIRECTED BY OWNER OR GOVERNING AUTHORITIES TO CONTROL EROSION AND SILTATION DURING LIFE OF CONTRACT. OWNER HAS AUTHORITY TO LIMIT SURFACE AREA OF ERODIBLE EARTH MATERIAL EXPOSED BY CLEARING AND GRUBBING, EXCAVATION, TRENCHING, BORROW AND EMBANKMENT OPERATIONS. OWNER ALSO HAS AUTHORITY TO DIRECT CONTRACTOR TO PROVIDE IMMEDIATE PERMANENT OR TEMPORARY EROSION AND SEDIMENT CONTROL MEASURES.
- G. CONTRACTOR SHALL RESPOND TO EROSION AND SEDIMENT CONTROL MAINTENANCE REQUIREMENTS OR IMPLEMENT ADDITIONAL MEASURES TO CONTROL EROSION ORDERED BY OWNER OR GOVERNING AUTHORITIES WITHIN 48 HOURS OR SOONER IF REQUIRED AT NO ADDITIONAL COST TO THE OWNER.
- H. CONTRACTOR WILL BE REQUIRED TO INCORPORATE PERMANENT EROSION CONTROL FEATURES INTO PROJECT AT EARLIEST PRACTICAL TIME TO MINIMIZE NEED FOR TEMPORARY CONTROLS.
- I. THE EROSION AND SEDIMENT CONTROL MEASURES SHOWN ON THE PLANS REPRESENT A MINIMUM REQUIREMENT. THE CONTRACTOR IS RESPONSIBLE FOR DETERMINING ADDITIONAL EROSION AND SEDIMENT CONTROL MEASURES NEEDED IN ORDER TO PREVENT THE TRANSFER OF SEDIMENT FROM THE PROJECT AREA AND PREVENT THE EROSION OF SURFACES DURING CONSTRUCTION, AS NEEDED TO PROTECT ADJACENT PROPERTIES AND WATER BODIES.
- J. INSPECT EVERY TWO WEEKS DURING CONSTRUCTION. REMOVE ANY SEDIMENT BUILD-UP. REPAIR AND REINSTALL ANY DAMAGED OR MISSING SEDIMENT CONTROL MEASURES. INSTALL ADDITIONAL MEASURES IF INSPECTION REVEALS ADDITIONAL SEDIMENTATION CONTROL IS NECESSARY.
- K. AREAS TO BE PAVED SHALL BE TREATED WITH A BITUMINOUS PRIME COAT AND SANDED TO MINIMIZE EROSION, WHERE PAVING IS SCHEDULED TO OCCUR MORE THAN 48 HOURS AFTER INSTALLATION OF BASE COURSE. AREAS TO RECEIVE CONCRETE PAVING SHALL BE EITHER PROTECTED WITH A LAYER OF FDOT COARSE AGGREGATE MATERIAL OR SHALL BE PAVED WITHIN 48 HOURS OF INSTALLATION OF THE SUBGRADE. INSTALL FINAL SURFACE COURSES WITHIN 14 DAYS AFTER REMOVAL OF EXISTING PAVEMENT.

CONTROL OF WIND EROSION

- A. BARE EARTH AREAS SHALL BE WATERED DURING CONSTRUCTION AS NECESSARY TO MINIMIZE THE TRANSPORT OF FUGITIVE DUST. IT MAY BE NECESSARY TO LIMIT CONSTRUCTION VEHICLE SPEED IF BARE EARTH HAS NOT BEEN EFFECTIVELY WATERED. IN NO CASE SHALL FUGITIVE DUST BE ALLOWED TO LEAVE THE SITE UNDER CONSTRUCTION.
 - B. AS REQUIRED AFTER COMPLETION OF CONSTRUCTION, BARE EARTH AREAS SHALL BE VEGETATED.
 - C. AT ANY TIME BOTH DURING AND AFTER SITE CONSTRUCTION THAT WATERING AND/OR VEGETATION ARE NOT EFFECTIVE IN CONTROLLING WIND EROSION AND/OR TRANSPORT OF FUGITIVE DUST, OTHER METHODS AS ARE NECESSARY FOR SUCH CONTROL SHALL BE EMPLOYED. THESE METHODS MAY INCLUDE ERECTION OF DUST CONTROL FENCES. IF REQUIRED, DUST CONTROL FENCES SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE DETAIL FOR A SILT FENCE EXCEPT THE MINIMUM HEIGHT SHALL BE 4 FEET.
- IN ADDITION TO THOSE RESPONSIBILITIES OUTLINED WITHIN THE CONSTRUCTION PLANS AND DOCUMENTS, THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE FOLLOWING MEASURES:

- A. PROJECT SCHEDULE WITH EROSION AND SEDIMENT CONTROL INSTALLATION AND MAINTENANCE TIED TO SPECIFIC DATES OR CONSTRUCTION ACTIVITIES.
- B. ALTERATIONS TO THE DESIGN EROSION AND SEDIMENT CONTROLS DUE TO DIFFERENCES BETWEEN THE DESIGN PLANS AND ANTICIPATED CONSTRUCTION PHASING AND THE CONTRACTOR'S CONSTRUCTION METHODS.
- C. NAME AND PHONE NUMBER OF CONTRACTOR'S REPRESENTATIVE RESPONSIBLE FOR EROSION AND SEDIMENT CONTROL INSTALLATION AND MAINTENANCE ON A 24 HOUR BASIS.
- D. THE CONTRACTOR WILL FURNISH, INSTALL, MAINTAIN AND SUBSEQUENTLY REMOVE, ALL NECESSARY EROSION CONTROL. THE CONTRACTOR WILL FURNISH AND INSTALL ALL NECESSARY PERMANENT EROSION CONTROLS.
- E. THE DEVELOPMENT OF THE APPLICABLE BMPs TO ENSURE THE CONTROL OF OFF-SITE TRACKING (SPILLAGE, SANITARY WASTE, FERTILIZERS & PESTICIDES, SOLID WASTE DISPOSAL, AND NON-STORMWATER DISCHARGES & HAZARDOUS WASTE. WHEN A SPILL OF REPORTABLE QUANTITIES IS DISCOVERED ON THE SITE, THE CONTRACTOR SHALL CLEAN UP ALL SPILLED MATERIALS AND DISPOSE OF IN ACCORDANCE WITH APPLICABLE LAWS AND REGULATIONS. THE CONTRACTOR SHALL NOTIFY THE APPROPRIATE AUTHORITIES, IN ACCORDANCE WITH APPLICABLE LAWS AND REGULATIONS AND PROJECT ENGINEER. THE CONTRACTOR SHALL RETAIN CLEAN UP INFORMATION AS WELL AS DISPOSAL MANIFESTS WITH THEIR SWPPP.

THE CONTRACTOR IS ADVISED THAT THE CONTRACT DRAWINGS ONLY INDICATE EROSION, SEDIMENT, AT LOCATIONS DETERMINED IN THE DESIGN PROCESS. HOWEVER, THE CONTRACTOR IS REQUIRED TO PROVIDE ANY ADDITIONAL CONTROLS NECESSARY TO PREVENT THE POSSIBILITY OF SILTING ANY ADJACENT LOWLAND PARCEL OR RECEIVING WATER.

STORM WATER MANAGEMENT

THE STORM WATER RUNOFF FROM THE PROJECT AREA WILL BE COLLECTED IN A SYSTEM OF INLETS AND CONVEYED TO EXFILTRATION TRENCHES FOR WATER QUALITY TREATMENT AND PEAK ATTENUATION.

MAINTENANCE

MAINTENANCE OF EROSION CONTROL DEVICES IS OF PARAMOUNT IMPORTANCE TO CVS/ PHARMACY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING ALL POLLUTION PREVENTION CONTROLS. DAILY REVIEW SHALL BE MADE BY THE CONTRACTOR TO DETERMINE IF CONSTRUCTION ACTIVITIES HAVE ALTERED THE EFFECTIVENESS OF EROSION, SEDIMENTATION, AND POLLUTION CONTROL MEASURES. CORRECTIVE ACTION SHALL BE PERFORMED IMMEDIATELY AT THE END OF EACH DAY OF WORK. THE CONTRACTOR WILL COMPLETE A REPORT DETAILING MEASURES THAT ARE NOT ACHIEVING MEANT COMPLIANCE AND THE CORRECTIVE ACTION THAT IS TAKEN. UNLESS OTHERWISE SPECIFIED, ACCUMULATED SEDIMENTS SHOULD BE REMOVED BEFORE THEY REACH ONE-HALF OF THE CAPACITY OF THE CONTROL DEVICE.

INSPECTION

THE CONTRACTOR IS REQUIRED TO INSPECT AND MAINTAIN CONTROLS WEEKLY AND WITHIN 24 HOURS AFTER A RAINSTORM IN EXCESS OF 0.25 INCHES. THE CONTRACTOR SHALL REPORT ALL INSPECTION FINDINGS AND CORRECTIVE ACTIONS TAKEN AS A RESULT OF THE INSPECTION. INSPECTION REPORTS SHALL BE SIGNED BY THE INSPECTOR AND CONTRACTOR AND MAINTAINED FOR FUTURE REFERENCE AS NEEDED.

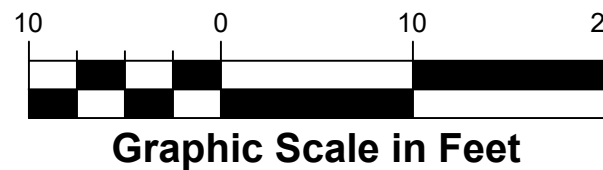
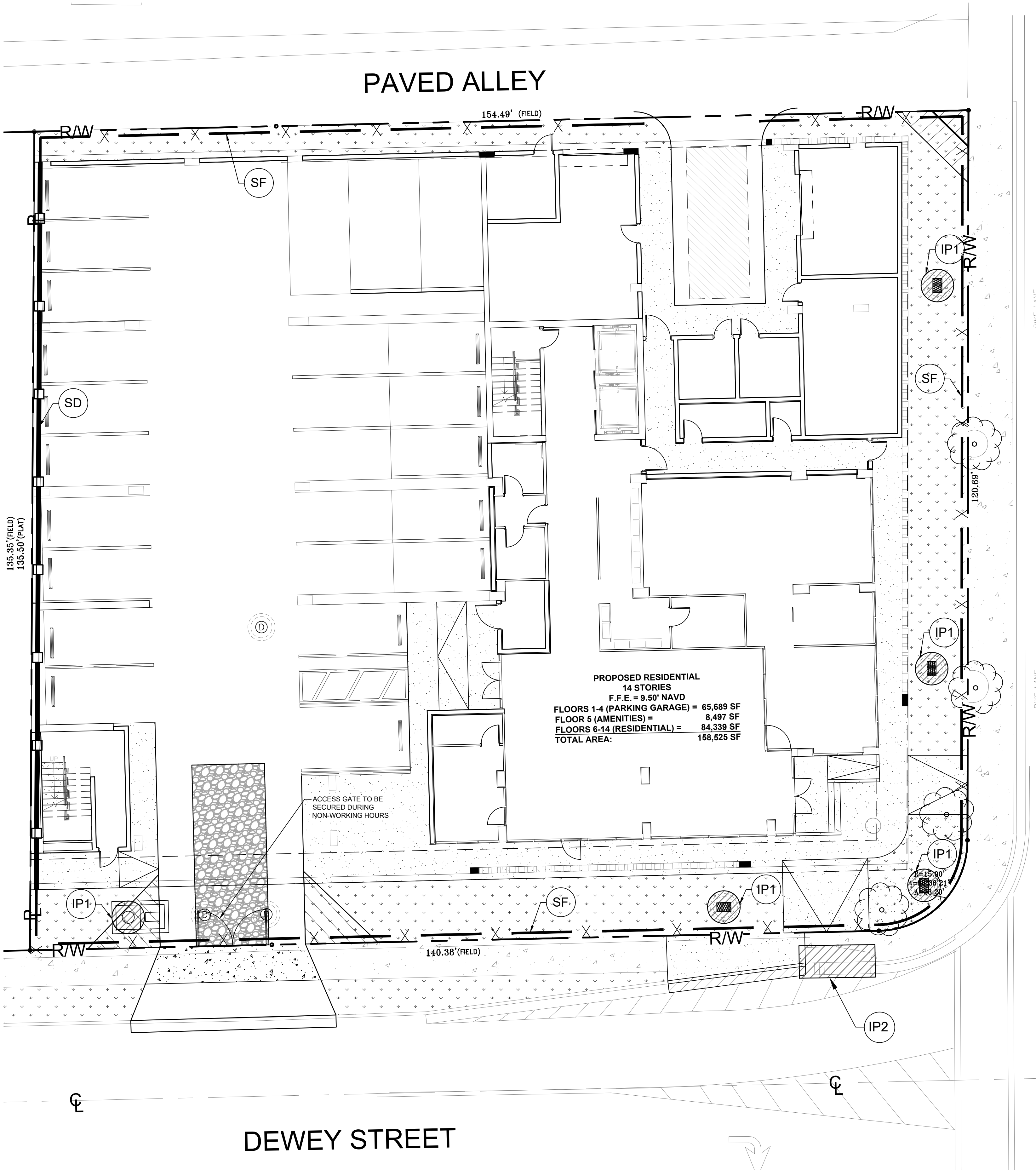
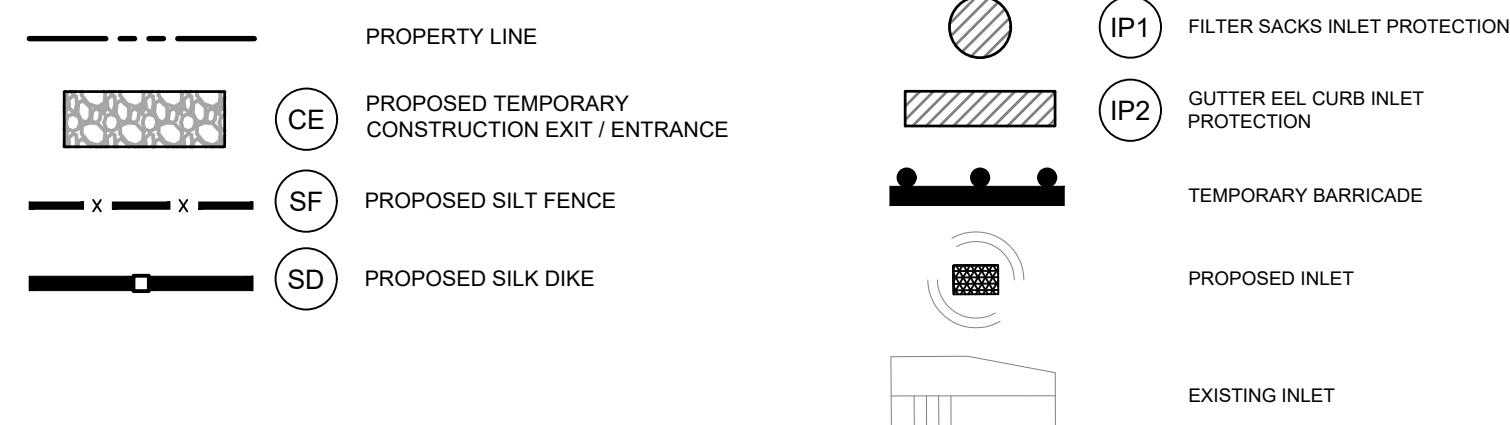
THE INSPECTOR MUST BE A QUALIFIED EROSION AND SEDIMENT CONTROL INSPECTOR AS DEFINED BY THE FLORIDA DEPARTMENT OF ENVIRONMENTAL PROTECTION.

IT IS THE CONTRACTOR'S RESPONSIBILITY (FOR ALL SITES WHICH ARE ONE (1) ACRE AND GREATER) TO FILE "NOTICE OF INTENT (NOI) FOR STORM WATER DISCHARGES ASSOCIATED WITH CONSTRUCTION ACTIVITY UNDER A NPDES GENERAL PERMIT" (EPA FORM 4510-9 OR LATEST VERSION) TO EPA AND "NOTICE OF INTENT" TO USE GENERIC PERMIT FOR STORMWATER DISCHARGE FROM LARGE AND SMALL CONSTRUCTION ACTIVITIES" (DEP FORM 62-621-300(A)) (OR LATEST VERSION) TO DEP TO THE FOLLOWING ADDRESSES: NPDES STORMWATER NOTICES CENTER, MS #2510 FLORIDA DEPARTMENT OF ENVIRONMENTAL PROTECTION 2600 BLAIR STONE ROAD TALLAHASSEE, FLORIDA 32399-2400

NOTES:

1. CONTRACTOR IS REQUIRED TO INSPECT ALL EROSION CONTROL DEVICES DAILY AND AFTER EVERY STORM EVENT AND REPAIR AND/OR REPLACE AS NEEDED.
2. ANY NEARBY INLETS WITHIN RIGHT-OF-WAY SHALL HAVE GUTTER BUDDY INSTALLED.

DRAINAGE SYMBOLS LEGEND



666 N.E. 125th STREET,
SUITE 247
NORTH MIAMI, FL 33161
Phone: 305.720.2079
C.O.A. 33221



2/26/2024

This item has been digitally signed and sealed by Christopher P. Collins, P.E. on the date adjacent to the seal.

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PERMIT SET
MUST BE ON JOBSITE AT ALL TIMES DURING CONSTRUCTION

NOTICE
INSPECTION REQUIRED
24 HRS. PRIOR TO COMMENCING ANY WORK IN THE PUBLIC ROW CONTACT THE BROWARD COUNTY HIGHWAY CONSTRUCTION AND ENGINEERING DIVISION AT 954-577-4689 FOR INSPECTION.

NOTE:
APPROVAL OF THIS PLAN DOES NOT CONSTITUTE A PERMIT FOR CONSTRUCTION.
A PERMIT FOR CONSTRUCTION MUST BE OBTAINED FROM THE BROWARD COUNTY HIGHWAY CONSTRUCTION AND ENGINEERING DIVISION PRIOR TO COMMENCING CONSTRUCTION IN THE PUBLIC RIGHT OF WAY.
ALL MATERIALS USED AND INSTALLATIONS WITHIN THE PUBLIC RIGHT OF WAY ON EARTH'S SHALL BE IN ACCORDANCE WITH BROWARD COUNTY ENGINEERING DIVISION SPECIFICATIONS.

BROWARD COUNTY HIGHWAY CONSTRUCTION AND ENGINEERING DIVISION

☐ PLAN CONSISTENT WITH PLAT REQUIREMENTS
☐ PUBLIC RIGHT OF WAY APPROVAL FOR PAVING, GRADING AND DRAINAGE

BY: _____ DATE: _____
DOES NOT INCLUDE APPROVAL OF PAVEMENT MARKING & SIGNS

Revision Date	
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Revision Date	02/26/2024
Comment	TAC Rev 1
Designed by:	C.P.C.
Drawn by:	A.T.S.
Checked by:	C.P.C.
Approved by:	C.P.C.
Scale:	1" = 10'
Date:	12/20/2023
Job No.:	C010
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Plans for

THE GEORGE
950 S. FEDERAL HWY.
HOLLYWOOD, FLORIDA 33020

**EROSION
CONTROL
PLAN**

Sheet No.

C-4.0



Call 811 or visit sunshine811.com two full business days before digging to have buried facilities located and marked.
Check positive response codes before you dig!

SITE DATA

FOLIO NUMBER: 5142-22-10-2240 & 5142-22-10-2250
ADDRESS: 950 S FEDERAL HWY, HOLLYWOOD, FLORIDA 33020
EXISTING AND PROPOSED SITE AREA: 0.48 AC (20,891 SF)

LAND USE

EXISTING AND PROPOSED: OFFICE

ZONING

EXISTING AND PROPOSED: FH-2 FEDERAL HIGHWAY MEDIUM-HIGH INTENSITY MIXED-USE DISTRICT

KEYNOTE LEGEND

- 1 PROP. 24" STOP BAR
- 2 PROP. 2-6" DOUBLE YELLOW STRIPING
- 3 PROP. WHITE DIRECTIONAL TRAFFIC ARROWS
- 4 PROP. SINGLE 4" YELLOW SOLID LINE
- 5 PROP. WHEEL STOP (TYP.)
- 6 PROP. 5' ACCESSIBLE PARKING STRIPING
- 7 PROP. ENTRANCE / EXIT DOOR (REF. TO ARCH. PLANS)
- 8 EXIST. INLET TO REMAIN AND BE PROTECTED DURING CONSTRUCTION
- 9 PROP. LANDSCAPE (REF. TO LANDSCAPE PLANS)
- 10 MATCH EXISTING CURB, CONC. SIDEWALK LINE AND GRADE
- 11 PROP. 12x12" VISIBILITY TRIANGLE
- 12 EXISTING TREES. REFER TO LANDSCAPE PLANS FOR TREES TO BE REMOVED, RELOCATED OR TO REMAIN
- 13 PROP. 6'x6" PEDESTRIAN SIGHT TRIANGLE
- 14 PROP. SERVICE AREA
- 15 PROP. 23 LF OF TYPE "F" CURB AND GUTTER TO BE RECONSTRUCTED IN ACCORDANCE WITH BROWARD COUNTY STANDARD SPECIFICATIONS
- 16 EXIST. ENTRANCE TO BE CLOSED PER LATEST BROWARD COUNTY STANDARD SPECIFICATIONS
- 17 PROP. 4" THICK CONCRETE SIDEWALK
- 18 EXIST. 4" THICK CONCRETE SIDEWALK
- 19 PROP. 6" THICK CONCRETE SIDEWALK
- 20 PROP. 6" THICK CONCRETE DRIVEWAY
- 21 PROP. BUILDING WALL (REF. TO ARCH. PLANS)
- 22 PROP. 6'x6" RIGHT OF WAY DEDICATION
- 23 PROP. FIRE AERIAL APPARATUS (47' L X 21' W)
- 24 PROP. DROP CURB
- 25 EXIST. TYPE "F" CURB AND GUTTER
- 26 EXIST. BRICK CROSSWALK
- 27 EXIST. STRIPING

HATCH LEGEND

- PROPOSED 4" THICK CONCRETE SIDEWALK
- PROPOSED 6" THICK CONCRETE SIDEWALK
- EXISTING 6" THICK CONCRETE SIDEWALK
- PROPOSED SOD (REFER TO LANDSCAPE PLANS FOR DETAILS)
- EXISTING SOD
- CONTRACTOR TO CLOSE EXISTING DRIVEWAY AND CONSTRUCT TYPE "F" CURB AND GUTTER PER BROWARD COUNTY STANDARD SPECIFICATIONS

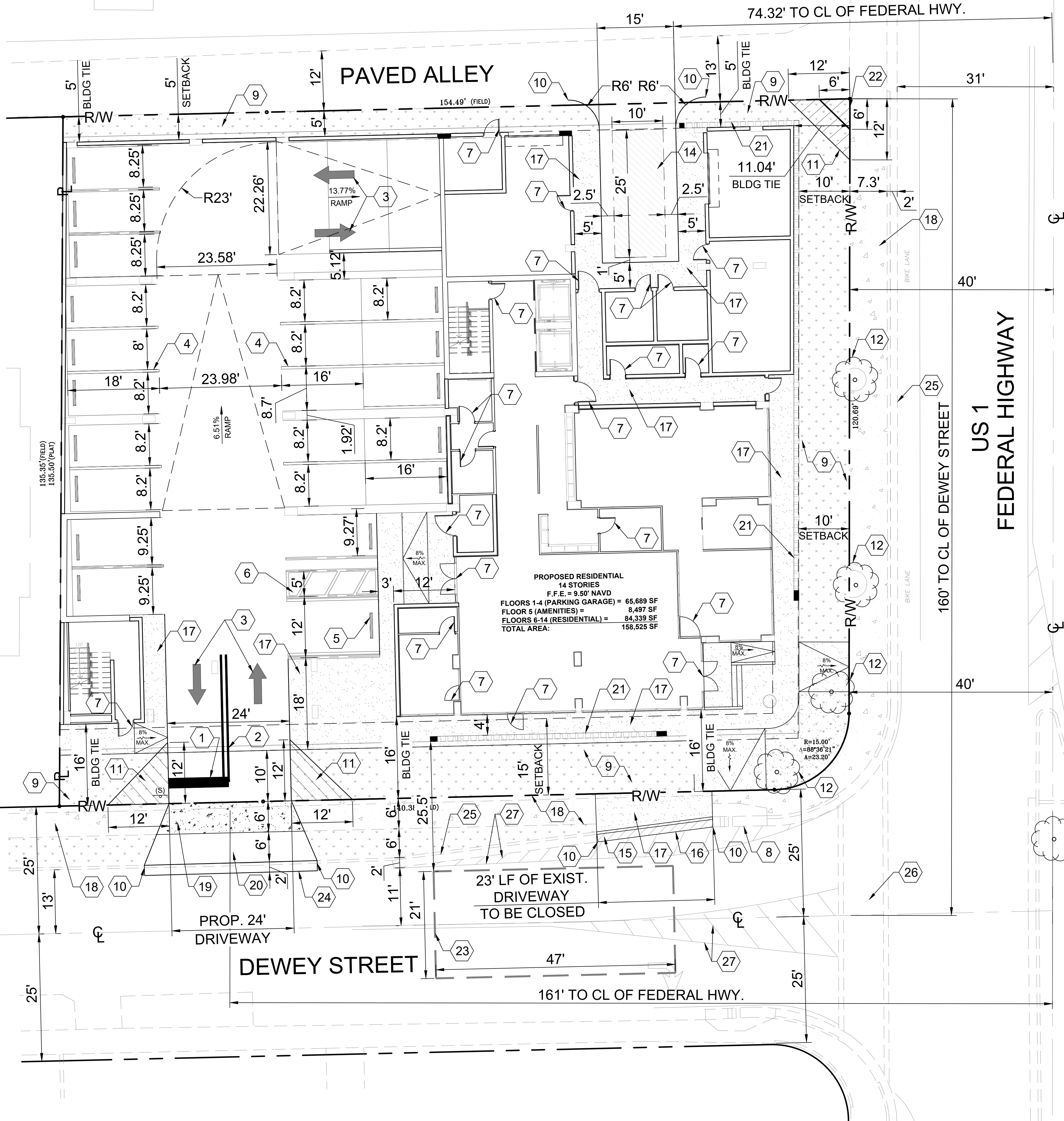
SITE SYMBOLS / LEGEND

- (S) STOP SIGN (R1-1)
- (HC) HANDICAP PARKING SIGN
- (EVP) ELECTRIC VEHICLE PARKING
- (CS) CHARGING STATION
- PROPOSED PARKING SPACE COUNT
- ACCESSIBLE PARKING SPACE
- PROPOSED SOLID DIRECTIONAL ARROW

SIGNAGE NOTE

ALL SIGNAGE SHALL BE IN COMPLIANCE WITH THE ZONING AND LAND DEVELOPMENT REGULATIONS.

EXISTING
1 STORY CBS
BUILDING



PERMIT SET
MUST BE ON JOBSITE AT ALL TIMES DURING CONSTRUCTION

NOTICE
INSPECTION REQUIRED
24 HRS. PRIOR TO COMMENCING ANY WORK IN THE PUBLIC R/W CONTACT THE BROWARD COUNTY HIGHWAY CONSTRUCTION AND ENGINEERING DIVISION AT 954-577-4669 FOR INSPECTION.

NOTE:
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A PERMIT FOR CONSTRUCTION MUST BE OBTAINED FROM THE BROWARD COUNTY HIGHWAY CONSTRUCTION AND ENGINEERING DIVISION PRIOR TO COMMENCING CONSTRUCTION IN THE PUBLIC RIGHT OF WAY.
ALL MATERIALS USED AND INSTALLATIONS WITHIN THE PUBLIC RIGHT OF WAY OR EASEMENTS SHALL BE IN ACCORDANCE WITH BROWARD COUNTY HIGHWAY CONSTRUCTION AND ENGINEERING DIVISION SPECIFICATIONS.

BROWARD COUNTY HIGHWAY CONSTRUCTION AND ENGINEERING DIVISION

☐ PLAN CONSISTENT WITH PLAT REQUIREMENTS
☐ PUBLIC RIGHT OF WAY APPROVAL FOR PAVING, GRADING AND DRAINAGE

BY: _____ DATE: _____
DOES NOT INCLUDE APPROVAL OF PAVEMENT MARKING & SIGNS

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Approved by:		C.P.C.
Scale:		1" = 10'
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Job No.:		C010
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Plans for		

THE GEORGE
950 S FEDERAL HWY.
HOLLYWOOD, FLORIDA 33020

SITE
PLAN

Sheet No.

C-5.0

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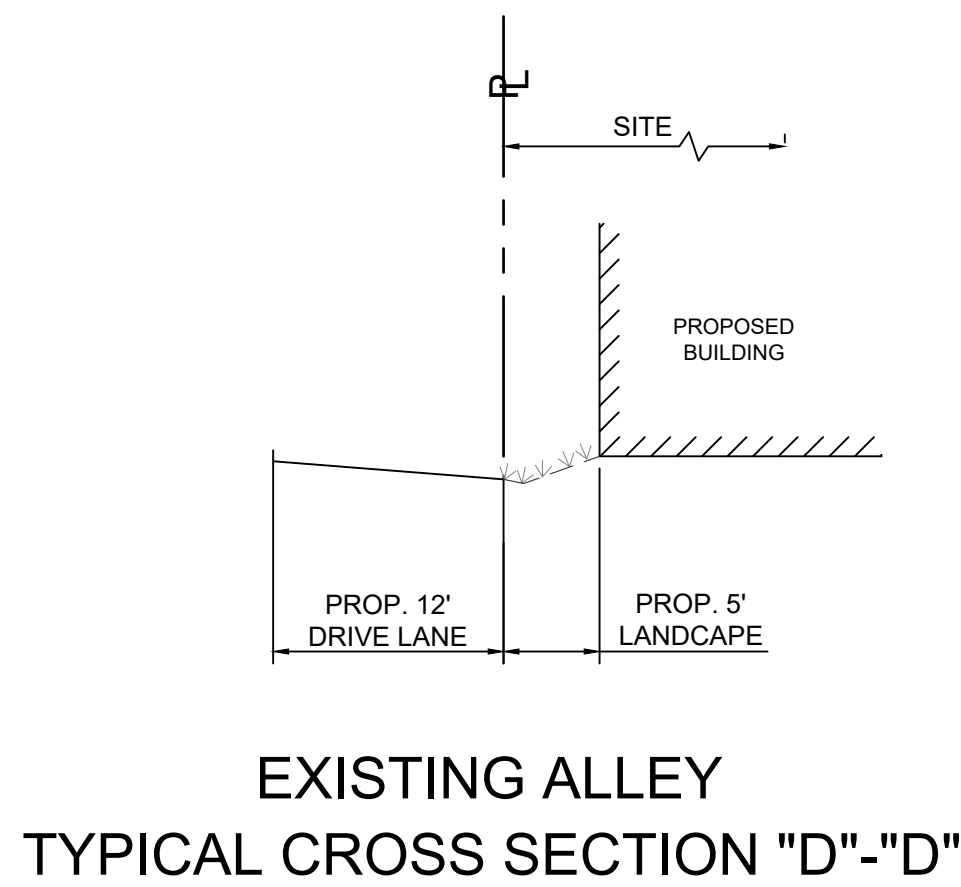
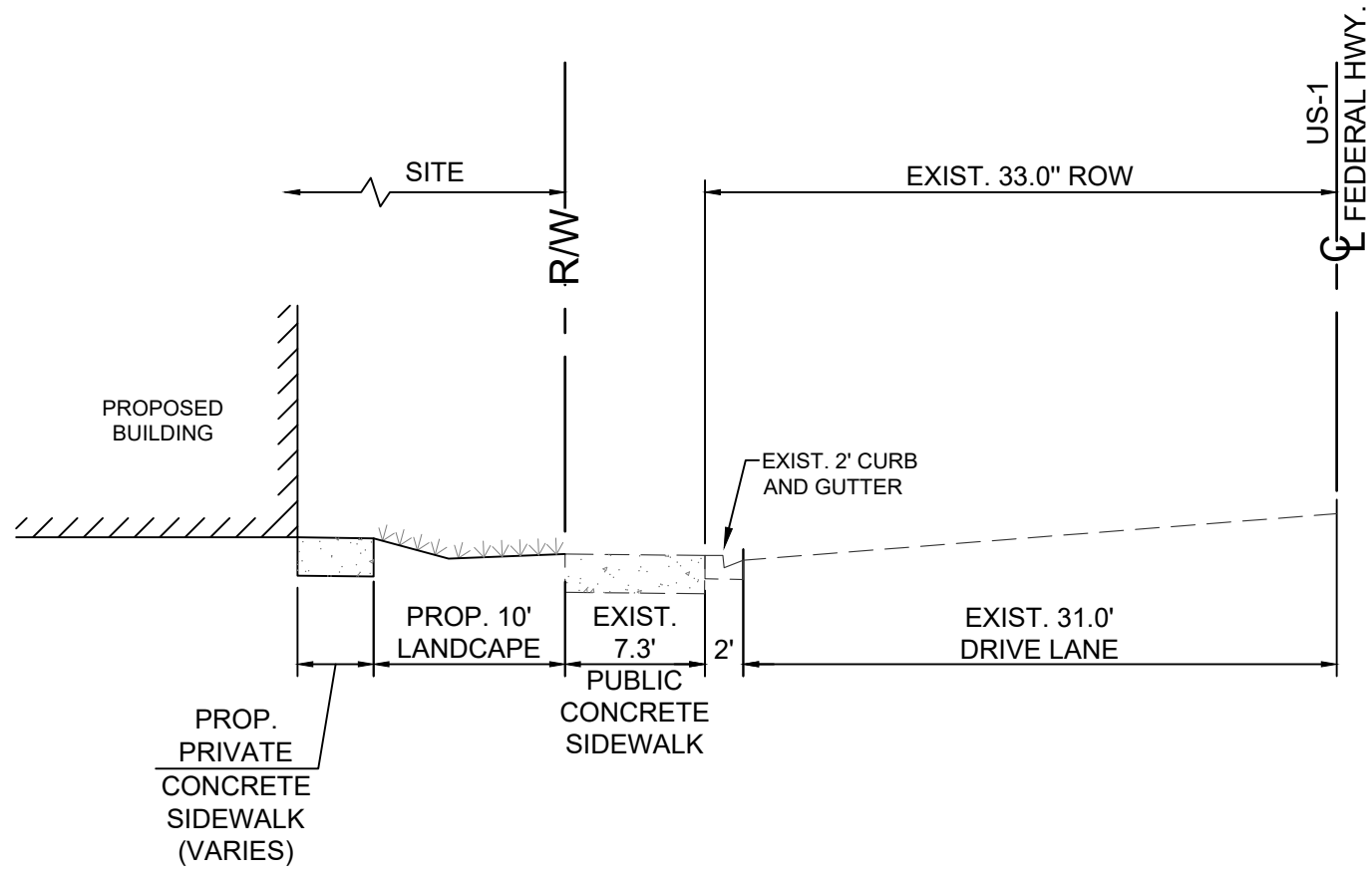
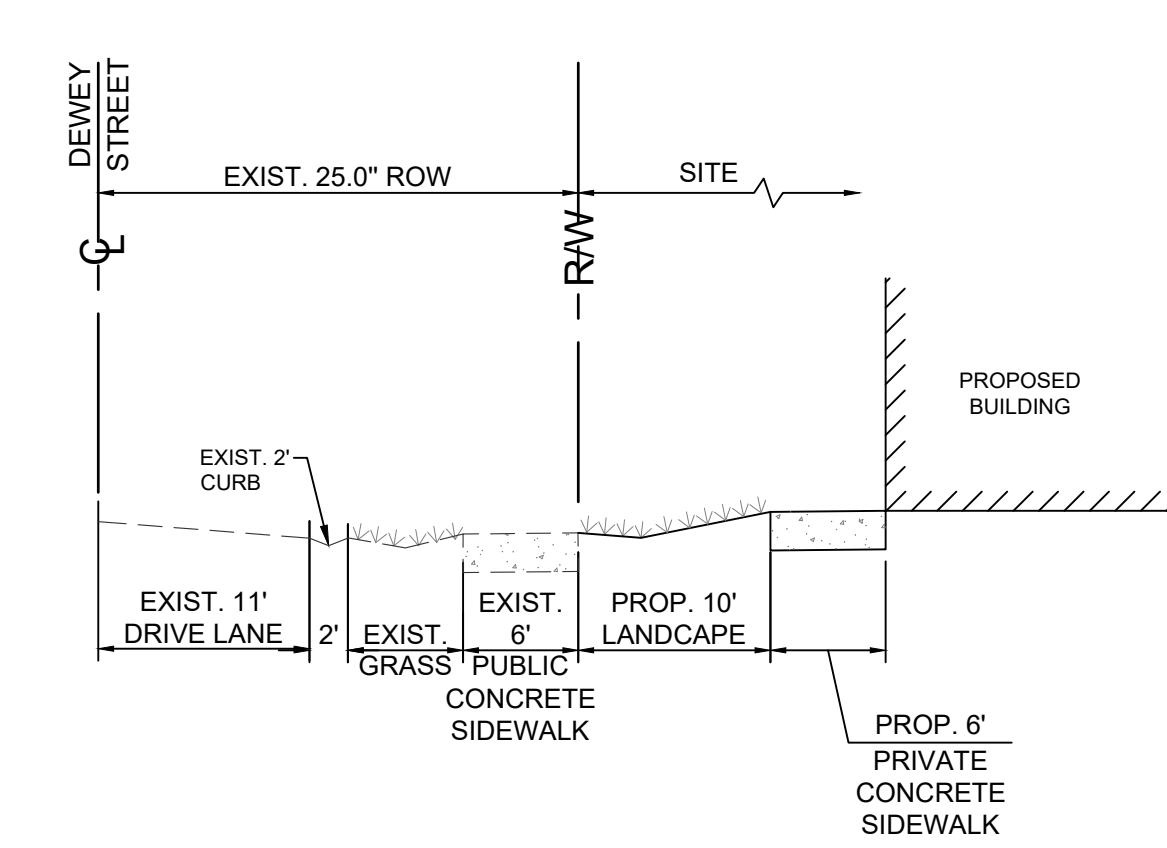


DESIGN

666 N.E. 125th STREET,
SUITE 247
NORTH MIAMI, FL 33161
Phone: 305.720.2079
C.O.A. 33221



2/26/2024
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Plans for

THE GEORGE
950 S FEDERAL HWY.
HOLLYWOOD, FLORIDA 33020

CROSS SECTIONS
Sheet No.
C-5.2

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CHRISTOPHER PATRICK COLLINS
LICENSE No. 73819
STATE OF FLORIDA
PROFESSIONAL ENGINEER

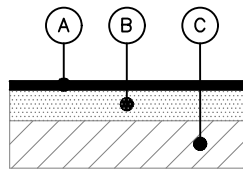
CHRISTOPHER COLLINS, P.E.
REG-# 73819



C-5.3

PAVEMENT NOTES:

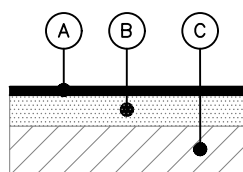
- DAMAGED PAVEMENT MARKINGS (INCLUDING THERMOPLASTIC MARKINGS AND RPMS) SHALL BE RESTORED TO MEET CITY OF MIAMI STANDARD SPECIFICATIONS AT THE PERMITTEE EXPENSE.
- DAMAGE TO SIDEWALK, RAMPS AND/OR CURB AND GUTTER SHOULD BE RESTORED PER CITY OF MIAMI STANDARD SPECIFICATIONS.
- DAMAGED SIGNAL LOOPS SHALL BE RESTORED TO MEET CITY OF MIAMI STANDARD SPECIFICATIONS AT THE PERMITTEE EXPENSE.
- CONTRACTOR TO PROVIDE INLET PROTECTION SYSTEM BEFORE ANY CONSTRUCTION BEGINS AND ENSURE TO BE USED THROUGHOUT THE DURATION OF CONSTRUCTION.
- FLOWABLE FILL SHALL NOT BE USED FOR LENGTHS OVER 50' AND DEPTH OVER 6". REFER TO CITY OF MIAMI STANDARD SPECIFICATIONS.
- ASPHALT AND SIDEWALK SHALL BE SAW CUT BEFORE REMOVING SHOULDER OR GUTTER TO PREVENT DAMAGE TO THE EXISTING ASPHALT.
- QC COMPANY/LAB INFORMATION IS TO BE PROVIDED TO THE DEPARTMENT REPRESENTATIVE AT THE PRE-COMMENCEMENT MEETING.
- FINAL INSPECTION OF WORK PRIOR TO REQUESTING FINAL INSPECTION FROM CITY OF MIAMI, CLOSE-OUT DOCUMENTATION AND CERTIFICATION OF WORK COMPLETION MUST BE SUBMITTED TO THE DEPARTMENT REPRESENTATIVE VIA EMAIL OR UPLOADED ONTO ONE STOP PERMITTING WEBSITE, PROVIDE CITY OF MIAMI WITH ALL MATERIAL CERTIFICATIONS, TEST RESULTS, CORE LOGS, SIGNED, SEALED AS-BUILTS FOR APPROVED FIELD CHANGES, AND ANY APPLICABLE DOCUMENTATION RELATED TO THE COMPLETED WORK.



- A 4" OPTIONAL BASE GROUP 9 (OBG9), ASPHALT SHALL BE PLACED IN TWO (2) LIFTS: 3" OF SP-12.5 AND 1.25" OF SP-9.5.
- B PROPOSED 8" CRUSHED LIMEROCK BASE WITH MINIMUM LBR-100 SHALL BE COMPACTED TO MINIMUM DENSITY OF 98% OF MAXIMUM DRY DENSITY AS DETERMINED BY AASHTO-T-160 UNDER ALL PAVED AREAS.
- C PROPOSED TYPE B-12.5, 12" STABILIZED SUB-BASE WITH MINIMUM LBR-40 AND SHALL BE COMPACTED TO 98% OF MAXIMUM DRY DENSITY AS PER AASHTO-T-160.

ASPHALT PAVEMENT DETAIL

N.T.S.



- A PROPOSED 1.25" FDOT SP 9.5 TL-B ASPHALT
- B EXISTING 8" CRUSHED LIMEROCK BASE
- C EXISTING TYPE B OR C, 12" STABILIZED SUB-BASE

MILLING & RESURFACE PAVEMENT DETAIL

N.T.S.

SITE SYMBOLS /LEGEND

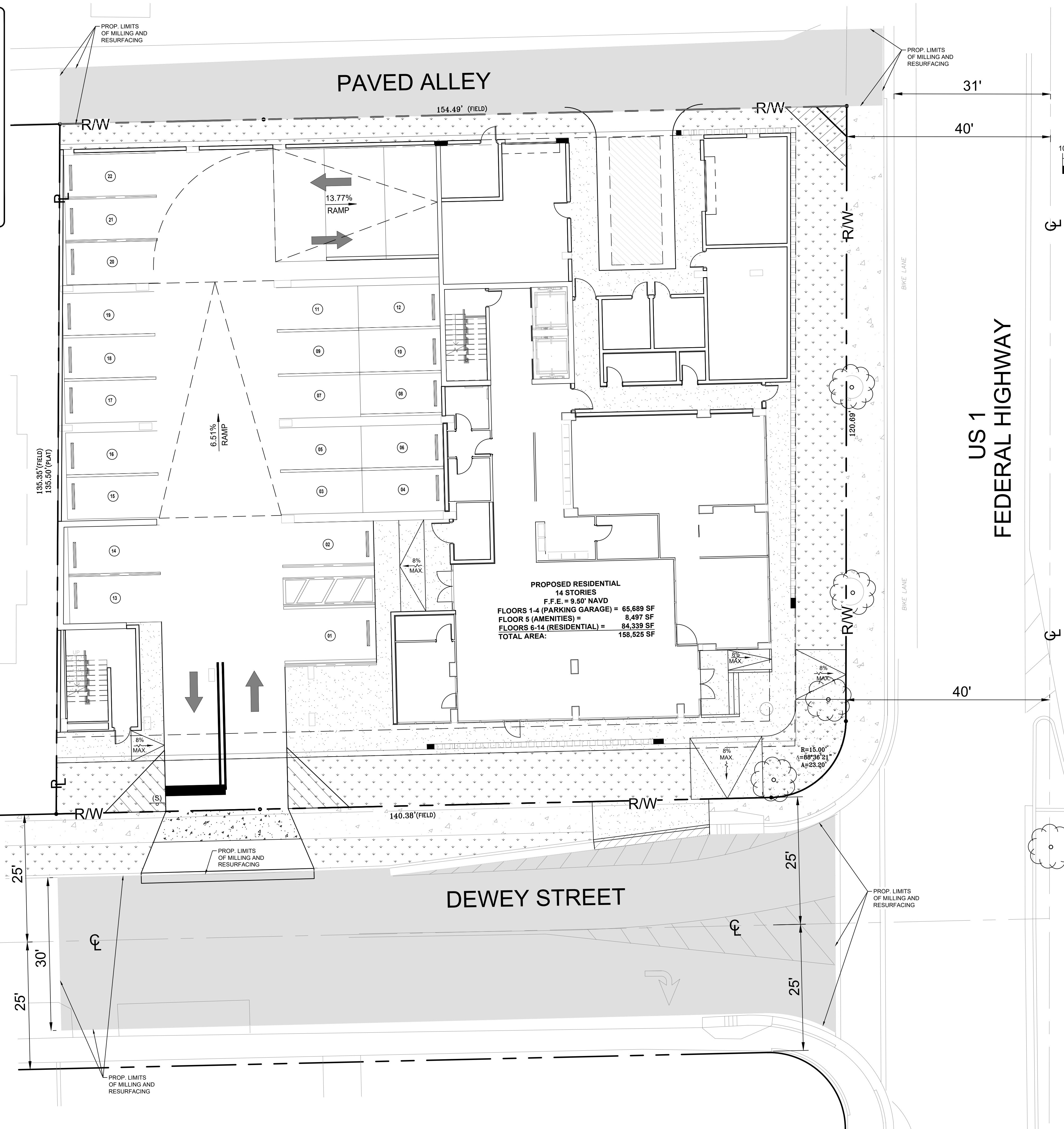
- (S) STOP SIGN (R1-1)
- (HC) HANDICAP PARKING SIGN
- (EVP) ELECTRIC VEHICLE PARKING
- (CS) CHARGING STATION
- PROPOSED PARKING SPACE COUNT
- ACCESSIBLE PARKING SPACE
- PROPOSED SOLID DIRECTIONAL ARROW

HATCH LEGEND

- PROP. CONCRETE SIDEWALK
- PROP. GRASS (REFER TO LANDSCAPE PLANS FOR DETAILS)
- PROP. LIMITS OF MILLING AND RESURFACING

MILLING & RESURFACING NOTE

FULL ROAD WIDTH OF DEWEY STREET AND THE ALLEY ADJUTING TO THE PROJECT SHALL BE MILLED AND RESURFACED



Graphic Scale in Feet

PERMIT SET

MUST BE ON JOBSITE AT ALL TIMES DURING CONSTRUCTION

NOTICE

INSPECTION REQUIRED
24 HRS. PRIOR TO COMMENCING ANY WORK IN THE PUBLIC R/W CONTACT THE BROWARD COUNTY HIGHWAY CONSTRUCTION AND ENGINEERING DIVISION AT 954-577-4689 FOR INSPECTION.

NOTE: APPROVAL OF THIS PLAN DOES NOT CONSTITUTE A PERMIT FOR CONSTRUCTION.

A PERMIT FOR CONSTRUCTION MUST BE OBTAINED FROM THE BROWARD COUNTY HIGHWAY CONSTRUCTION AND ENGINEERING DIVISION PRIOR TO COMMENCING CONSTRUCTION IN THE PUBLIC RIGHT OF WAY.

ALL MATERIALS USED AND INSTALLATIONS WITHIN THE PUBLIC RIGHT OF WAY OR EASEMENTS SHALL BE IN ACCORDANCE WITH BROWARD COUNTY HIGHWAY CONSTRUCTION AND ENGINEERING DIVISION SPECIFICATIONS.

BROWARD COUNTY HIGHWAY CONSTRUCTION AND ENGINEERING DIVISION

- PLAN CONSISTENT WITH PLAT REQUIREMENTS
- PUBLIC RIGHT OF WAY APPROVAL FOR PAVING, GRADING AND DRAINAGE

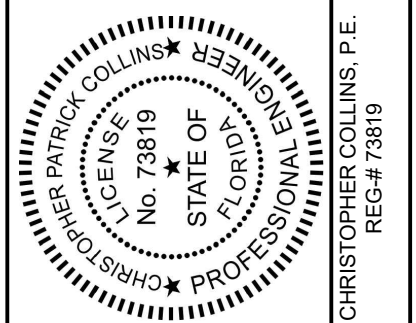
BY: DATE:

DOES NOT INCLUDE APPROVAL OF PAVEMENT MARKING & SIGNS



DESIGN

666 N.E. 125th STREET,
SUITE 247
NORTH MIAMI, FL 33161
Phone: 305.720.2079
C.O.A. 33221



2/26/2024

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Revision Date

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Revision Date

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Revision Date

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Revision Date

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Revision Date 02/26/2024

Comment

TAC Rev 1

Designed by: C.P.C.

Drawn by: A.T.S.

Checked by: C.P.C.

Approved by: C.P.C.

Scale: 1" = 10'

Date: 12/18/2023

Job No.: C010

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Plans for

THE GEORGE

950 S FEDERAL HWY.
HOLLYWOOD, FLORIDA 33020

PAVEMENT PLAN

Sheet No.

C-6.0



Call 811 or visit sunshine811.com two full business days before digging to have buried facilities located and marked.
Check positive response codes before you dig!

STRUCTURE TABLE									
STRUCTURE	TYPE	SIZE	RIM	BOTTOM	INVERTS				COMMENTS
					N	S	E	W	
S-1	MH	42-INCH	7.86	0.65			2.65	2.65	WEIR ELEV. = 5.19
S-2	INLET	42-INCH	7.86	-3.50	2.65	5.47	0.00	2.65	SKIMMER (N) (W)
S-3	INLET	42-INCH	7.15	2.65		4.65			YARD DRAIN
S-4	INLET	42-INCH	7.40	2.65	4.65	4.65			YARD DRAIN
S-5	INLET	42-INCH	7.66	2.65			4.65		YARD DRAIN
S-6	INLET	42-INCH	7.66	2.65			4.65		YARD DRAIN
S-7	MH	48-INCH	8.76	0.65	2.65	2.65		2.65	N/A
W-1	WELL				SEE DETAIL				
T-1	TRENCH	6-INCH	7.47			5.47			PEDESTRIAN FRIENDLY

ALL ELEVATIONS ARE IN NAVD88.

ADA NOTE:

CONSULTANT HAS IDENTIFIED AND USED THE CURRENT VERSION OF ALL LOCAL, STATE, AND FEDERAL ACCESSIBILITY GUIDELINES FOR SITE ACCESSIBILITY. THE SITE HAS BEEN DESIGNED IN ACCORDANCE WITH, BUT NOT LIMITED TO, CITY, COUNTY, AND STATE ACCESSIBILITY CODES, AND THE 2010 AMERICANS WITH DISABILITIES ACT STANDARDS FOR ACCESSIBLE DESIGN (2010 ADA STANDARDS), AS AMENDED.

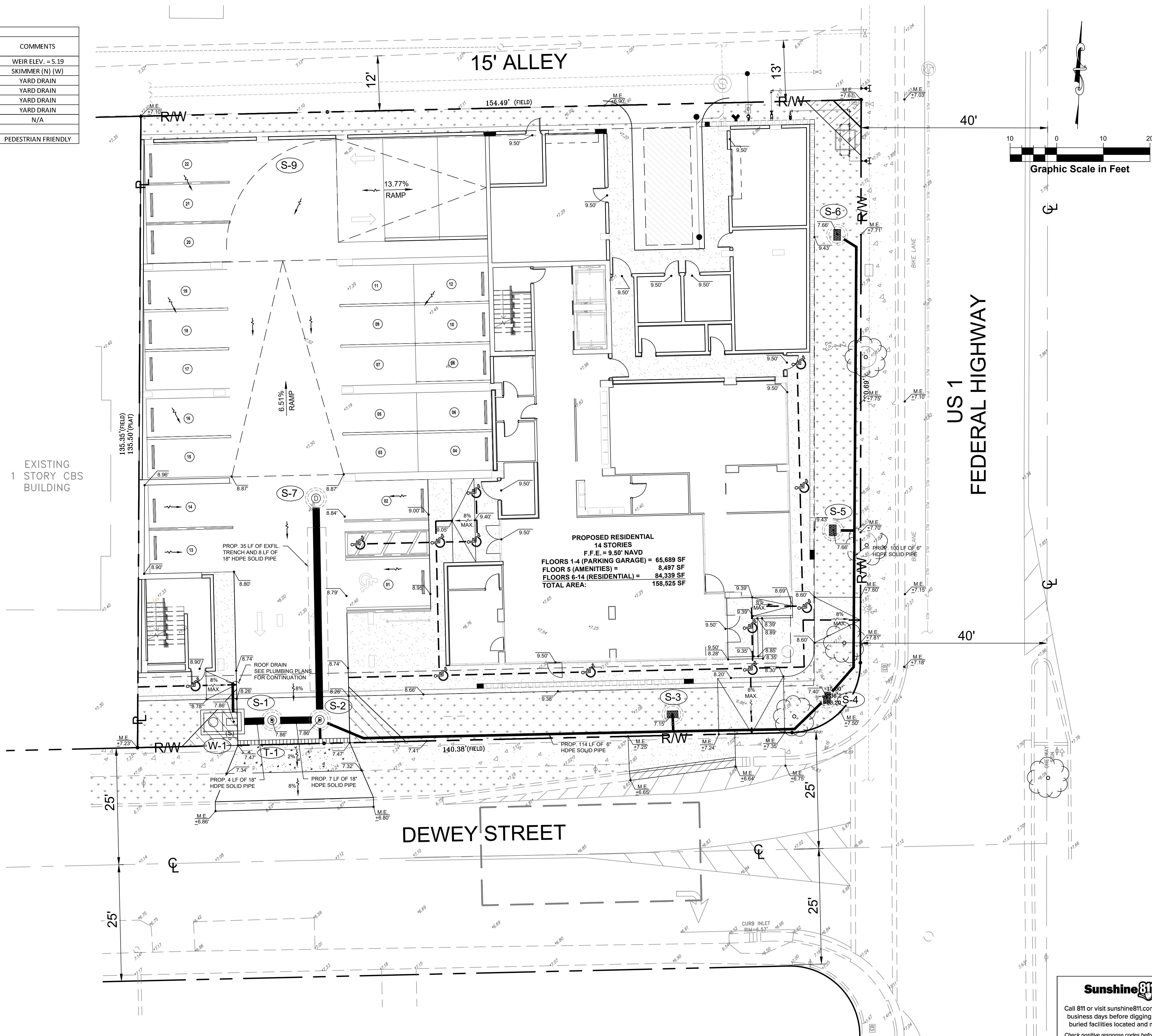
PROPOSED DRAINAGE SYMBOLS LEGEND

- (T.C.) TOP OF CURB, ELEV.
- (E.P.) EDGE OF PAVT. ELEV. OR GROUND ELEV.
- X% PROP. FLOW DIRECTION AND SLOPE
- X12.28 EXIST. GRADE ELEVATION
- PROP. STORM SEWER PIPE
- PROP. INLET WITH INFILTRATION TRENCH
- ACCESSIBLE ACCESS ROUTE

HATCH LEGEND

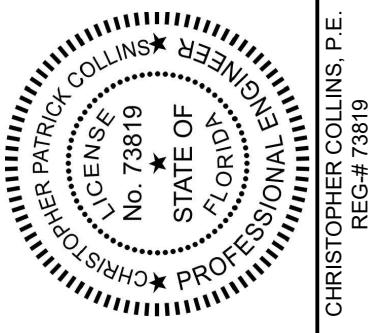
- PROPOSED 4" CONCRETE SIDEWALK
- PROPOSED 6" CONCRETE SIDEWALK
- PROPOSED GRASS (REFER TO LANDSCAPE PLANS FOR DETAILS)

EXISTING
1 STORY CBS
BUILDING



DESIGN

666 N.E. 125th STREET,
SUITE 247
NORTH MIAMI, FL 33161
Phone: 305.720.2079
C.O.A. 33221



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TAC Rev 1

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Plans for

THE GEORGE

950 S FEDERAL HWY,
HOLLYWOOD, FLORIDA 33020

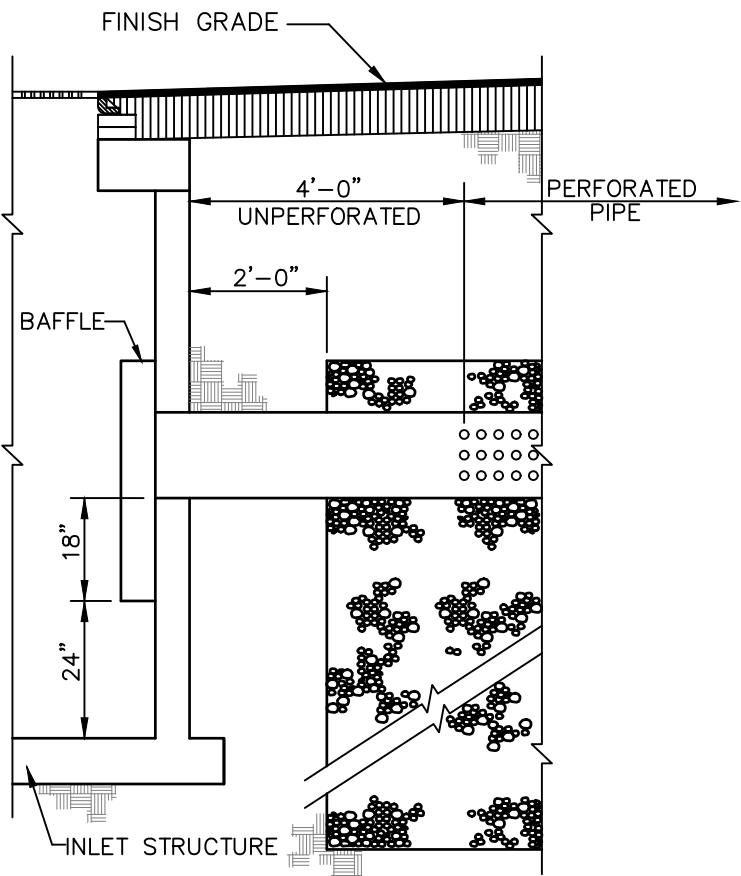
GRADING &
DRAINAGE
PLAN

Sheet No.

C-9.0



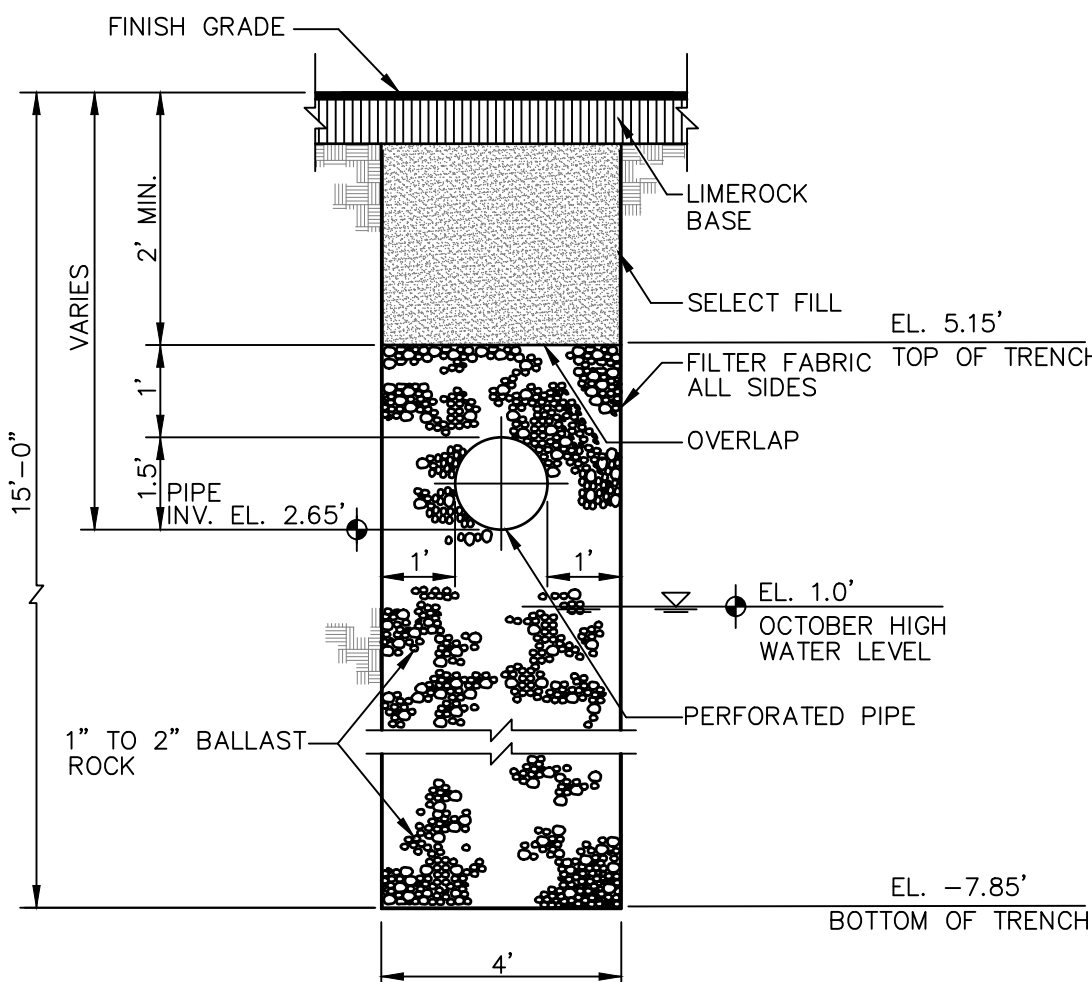
Call 811 or visit sunshine811.com two full business days before digging to have buried facilities located and marked.
Check positive response codes before you dig!



- NOTES:
1. PROVIDE BAFFLE WHEREVER PIPE ENTERS FRENCH DRAIN.
 2. BAFFLE SHALL BE AS SHOWN ON DETAIL IN THIS SHEET OR APPROVED EQUAL.
 3. SUMP 2'-0" (TYP.) EXCEPT AT POLLUTION CONTROL STRUCTURES SUMP TO BE 3'-6".

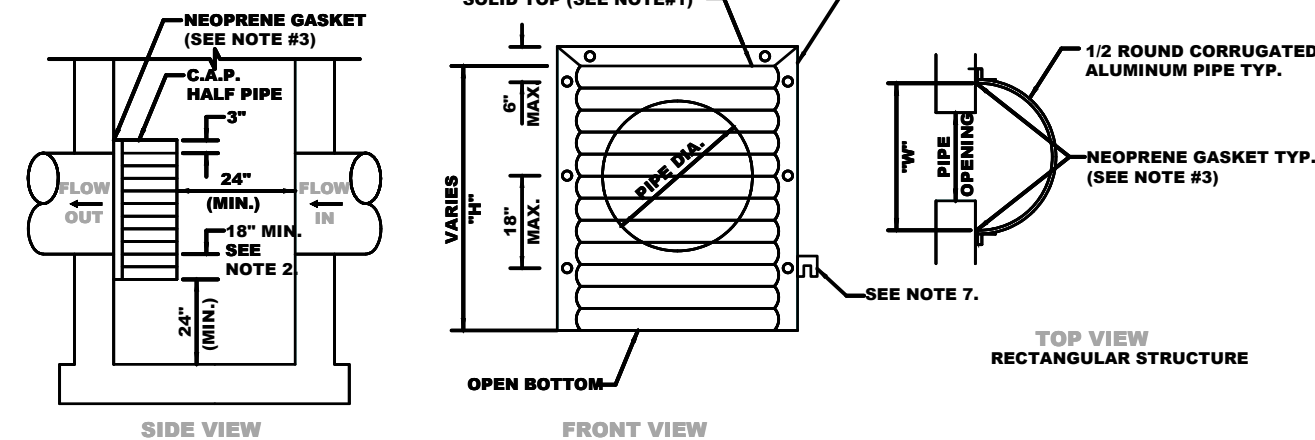
EXFILTRATION TRENCH CONNECTION DETAIL

N.T.S.



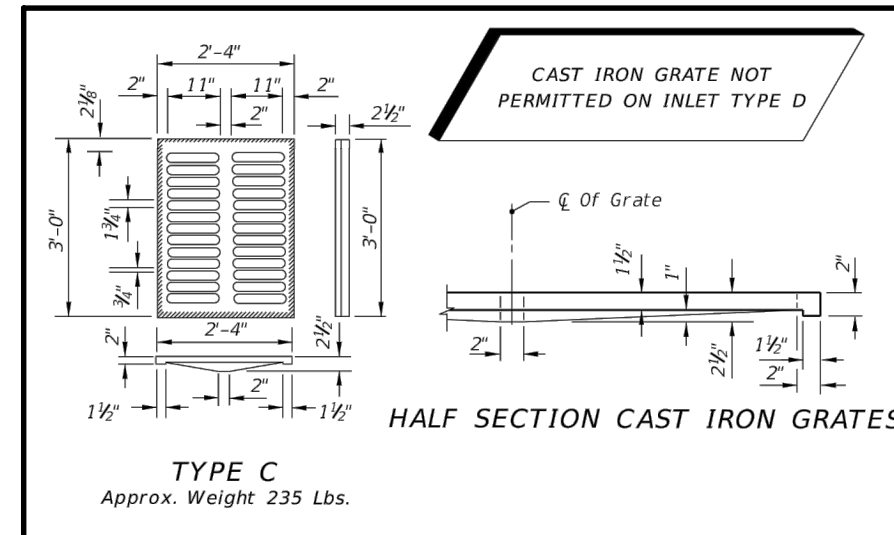
18" EXFILTRATION TRENCH SECTION

N.T.S. ALL ELEVATIONS ARE IN NAVD88.



POLLUTION RETARDANT BAFFLE DETAIL

N.T.S.

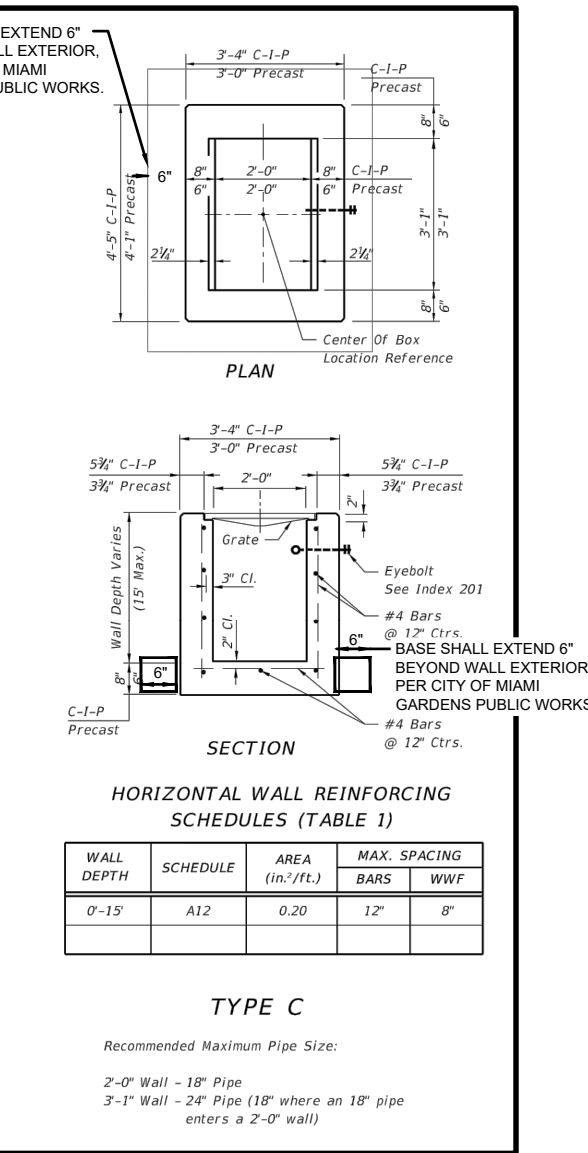


NOTE:

SEE FDOT INDEX 232 FOR
COMPLETE DETAIL, NOTES AND
SPECIFICATIONS.

FDOT TYPE C INLET

N.T.S.



DRAINAGE WELL STRUCTURAL NOTES:

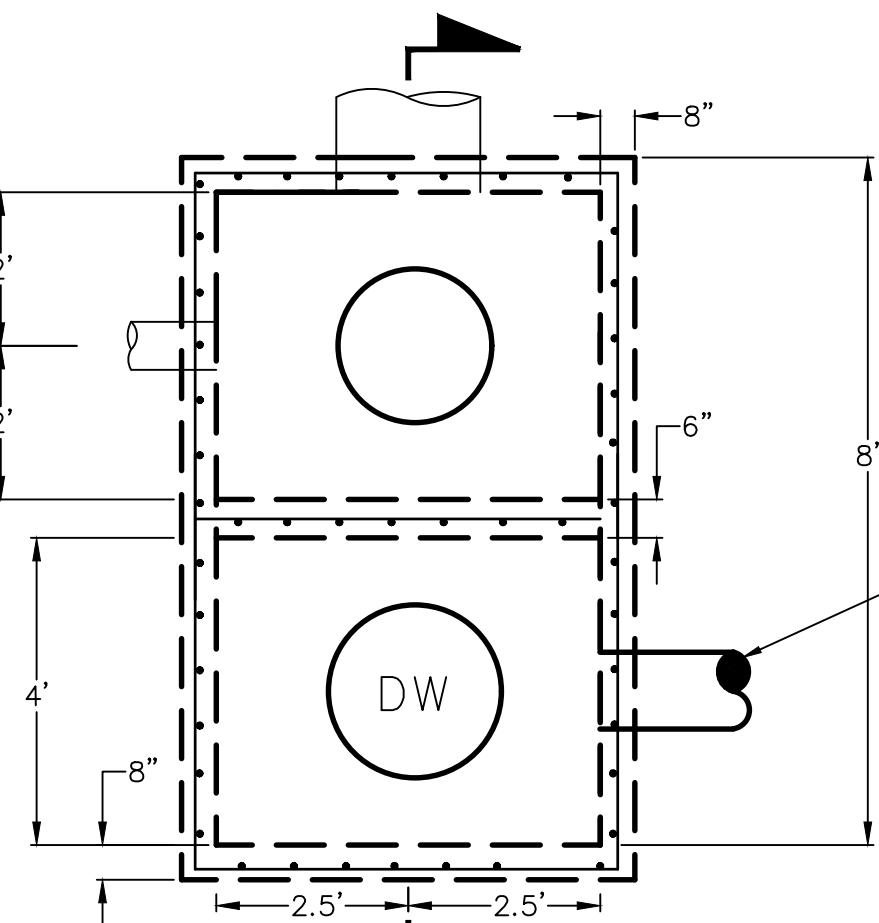
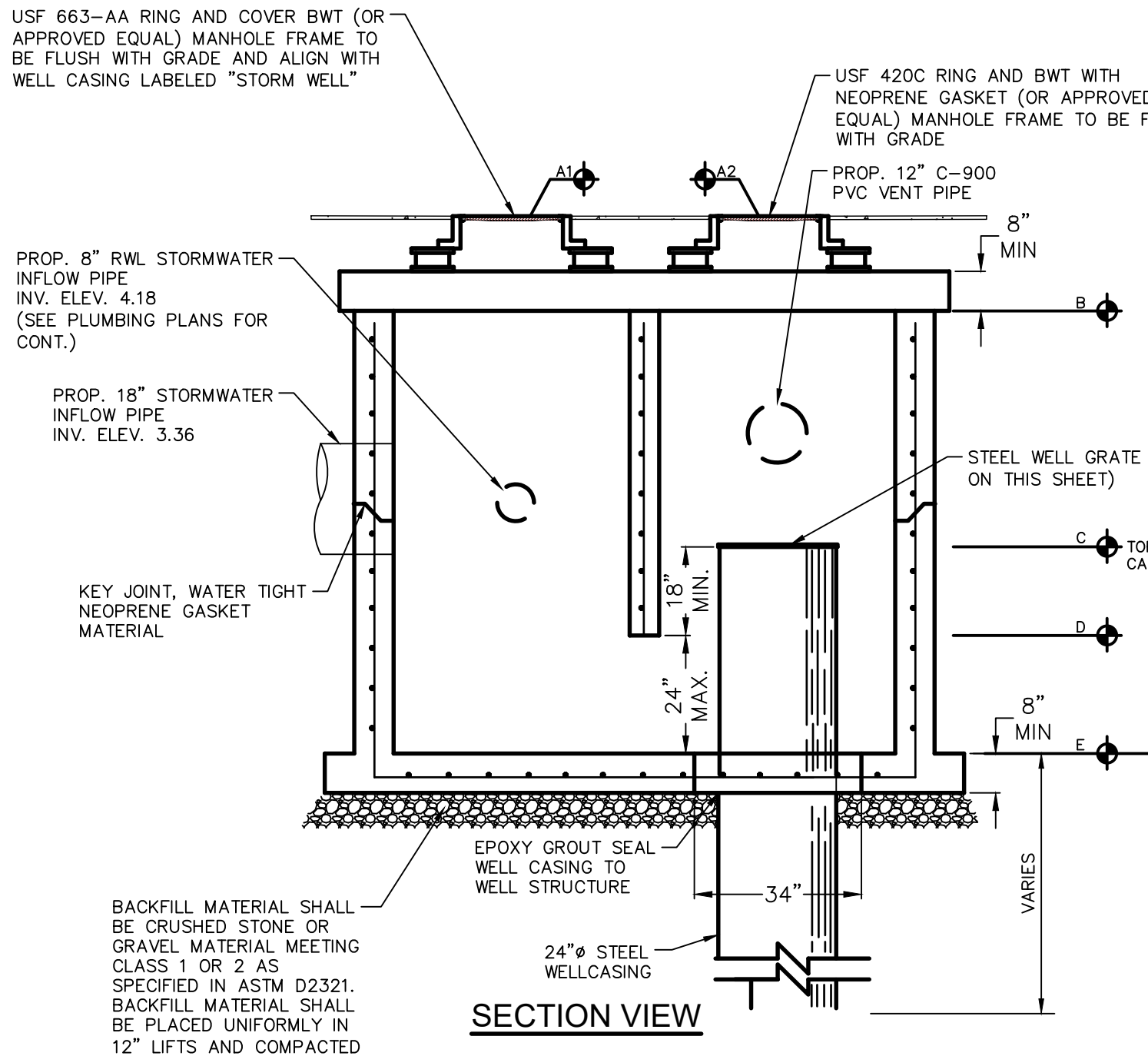
1. DESIGN CRITERIA: DESIGN, FABRICATION AND ERECTION OF PRECAST CONCRETE SHALL BE IN ACCORDANCE WITH ACI 318-99 BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE AND PCI DESIGN HANDBOOK. DESIGN OF CAST-IN-PLACE CONCRETE SHALL BE IN ACCORDANCE WITH ACI 318-99.
2. DESIGN LIVE LOADS (FOR EXTERIOR WELLS OUTSIDE THE BUILDING FOOTPRINT): THE TOP SLAB OF DRAINAGE WELL STRUCTURE SHALL BE DESIGNED TO CARRY TRAFFIC LOADS (HS20-44 LOADING).
3. GEOTECHNICAL CRITERIA (FOR EXTERIOR WELLS OUTSIDE THE BUILDING FOOTPRINT): SOIL BEARING PRESSURE UNDER STRUCTURE ASSUMED TO BE AT MINIMUM 2000 PSF. PRIOR TO INSTALLATION OF DRAINAGE STRUCTURE THE SOIL BEARING CAPACITY OF THE FOUNDATION MUST BE CONFIRMED BY THE CONTRACTOR, THROUGH A CERTIFIED GEOTECHNICAL LABORATORY.
4. CONCRETE: CONCRETE SHALL BE NORMAL WEIGHT, AND SHALL ATTAIN A 28, DAY COMPRESSIVE STRENGTH OF 4000 PSI AND SHALL MEET THE REQUIREMENT OF ASTM C478. CONCRETE COVER FOR REINFORCEMENT SHALL BE 2" EXCEPT FOOTING BOTTOM BARS SHALL HAVE 3" COVER. FOR DRAINAGE WELLS INSIDE THE BUILDING FOOTPRINT, XYPEX ADMIX C-500 SHALL BE ADDED TO THE CONCRETE MIX FOR WATER PROOFING (BY CONTRACTOR).
5. REINFORCING: REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60, AND SHALL BE OF U.S. MANUFACTURE.
6. MANHOLE RING & COVER: ALL MANHOLE RINGS AND COVERS, WITHIN PRIVATE PROPERTY, SHALL BE WATER TIGHT BOLTED AND HAVE THE WORD "STORM SEWER" CAST ON COVER.
7. KOR-N-SEAL PIPE TO MANHOLE CONNECTORS TO BE INSTALLED BETWEEN THE STEEL WELL CASINGS AND THE DRAINAGE WELL FLOOR SLAB. CONTRACTOR TO SUBMIT SHOP DRAWING. (FOR DRAINAGE WELLS INSIDE THE BUILDING FOOTPRINT ONLY)
8. INTERIOR SURFACES OF DRAINAGE WELL STRUCTURES TO RECEIVE TWO COATS OF XYPEX CONCENTRATE WATER-PROOF COATING. CONTRACTOR TO SUBMIT SHOP DRAWING. (FOR DRAINAGE WELLS INSIDE THE BUILDING FOOTPRINT ONLY)
9. ALL PIPE CONNECTIONS TO DRAINAGE WELL STRUCTURES WILL BE MADE WITH LINK SEAL PIPE-TO-MANHOLE CONNECTORS. CONTRACTOR TO SUBMIT SHOP DRAWING. (FOR DRAINAGE WELLS INSIDE THE BUILDING FOOTPRINT ONLY)
10. SHOP DRAWINGS: CONTRACTOR SHALL SUBMIT SHOP DRAWINGS, SIGNED AND SEALED BY A FLORIDA LICENSED ENGINEER, FOR STRUCTURES, XYPEX CONCRETE ADMIXTURE, KOR-N-SEAL, XYPEX CONCENTRATING WATER-PROOF COATING AND LINK SEAL TO THE ENGINEER FOR REVIEW PRIOR TO THE CONSTRUCTION OF STRUCTURE.

DRAINAGE WELL NOTES:

1. DEEP WELL TO BE CONSTRUCTED PER DRAINAGE WELL SCHEDULE. CONTRACTOR IS TO VERIFY THE FOLLOWING MINIMUM WELL CAPACITY:
 - a. 500 GALLONS PER MINUTE PER FOOT OF HEAD.
2. ALL FINISHED WELLS SHALL ACHIEVE A MINIMUM DRAW-DOWN RATE OF 500 GPM PER FOOT OF HEAD. THE WELL DRILLER SHALL RELY ON THEIR OWN EXPERIENCE OF THE LOCAL GEOLOGY OR THAT OF A GEOLOGIST/HYDROLOGIST (RETAINED BY THE WELL DRILLER) TO DETERMINE THE OPTIMUM DEPTH OF THE WELL CASING AND OPEN WELL IN ORDER TO ACHIEVE THE MAXIMUM POSSIBLE DRAW-DOWN RATE.
3. THE DEPTH OF THE WELL SHALL COMPLY WITH ALL FDEP REQUIREMENTS FOR UNDERGROUND INJECTION FOR CLASS V WELLS CARRYING STORM RUNOFF, FOR BID AND COST PURPOSES, THE CONTRACTOR SHALL PROVIDE A MINIMUM DEPTH OF WELL CASING OF 100 FEET, AND MINIMUM UNCASSED DEPTH OF 50 FEET. IF REQUIRED, CONTRACTOR SHALL DRIVE THE CASING UP TO 50% DEEPER THAN THE MINIMUM DEPTH AT NO ADDITIONAL COST TO THE CONTRACT. IF THE ABOVE PARAMETERS CAN NOT BE OBTAINED AT THE SPECIFIED DEPTH, CONTRACTOR TO NOTIFY ENGINEER OF RECORD BEFORE CONTINUING TO DRILL DEEPER. NO PAYMENT SHALL BE MADE TO CONTRACTOR FOR DRIVING CASING DEEPER THAN THE RECOMMENDED DEPTH WITHOUT PRIOR APPROVAL FROM THE ENGINEER OF RECORD.
9. STEEL WELL SCREEN TO BE INSTALLED OVER 24" DEEP WELL. STEEL GRATE TO BE HOT DIPPED GALVANIZED AFTER FABRICATION. COST TO BE INCLUDED IN THE PRICE OF CASING.
10. DEVELOPER OF WELL TO ASSURE 1500 P.P.M. SALINITY. THE WELL CASING SHALL PENETRATE A ZONE CONTAINING A MINIMUM OF 10,000 mg/L OF TOTAL DISSOLVED SOLIDS (TDS) IN ACCORDANCE WITH RULE 62-528.200(6), FAC.
11. WELL TESTING TO COMPLY WITH ASTM D 5472-93 AND D-4050-96 AND BE PERFORMED BY A THIRD PARTY FLORIDA REGISTERED GEOLOGIST.
12. ALL WELL BOX CONSTRUCTION JOINTS SHALL BE WATER TIGHT. WELL BOX MUST BE DESIGNED FOR UPLIFT FORCES (MIN 3 PSI) AND SIGNED AND SEALED BY A REGISTERED FLORIDA PROFESSIONAL ENGINEER.
13. DRAINAGE WELL CASING SHALL BE 24-INCH DIAMETER WELDED OR SEAMLESS STEEL PIPE CONFORMING TO ONE OF THE FOLLOWING STANDARDS
 - a. ASTM A53/A53M-99B (1999)
 - b. ASTM A135-01 (2001)
 - c. ASTM A252-98 (1998)
 - d. ASTM A589-96 (1996)
 - e. AMERICAN PETROLEUM INSTITUTE (API) 5L-2000 (2000)
7. DRAINAGE WELL CASING SHALL ALSO CONFORM TO THE 2000 AMERICAN NATIONAL STANDARD INSTITUTE FOR WELDED AND SEAMLESS WROUGHT STEEL PIPE (ANSI/ASME B36.10M-2000).
8. DRAINAGE WELL CASING SHALL BE STENOILED WITH THE APPLICABLE STANDARD.
9. DRAINAGE WELL CASING SHALL BE NEW OR IN LIKE NEW CONDITION, SHALL BE FREE OF BREAKS, CORROSION AND DENTS, SHALL BE STRAIGHT AND TRUE AND NOT OUT OF ROUND. IF ANY DAMAGE IS CAUSED TO THE CASING DURING CONSTRUCTION, THE CASING SHALL BE REPLACED BY THE CONTRACTOR AT NO EXPENSE TO THE OWNER.
10. ALL EXPOSED WELL CASING SURFACES SHALL BE COATED WITH BITUMINOUS PAINT.
11. ALL WELLS WITH RIM ELEV. BELOW VENT ELEV. NEED TO BE WATERTIGHT, BOLTED RIM TO FRAME AND SECURED TO THE TOP SLAB.
12. ALL DRAINAGE WELLS WITHIN BUILDING FOOTPRINT SHALL NOT BE SUBJECT TO OVERHEAD LOADS.
13. ALL PIPES INTERCONNECTING THE WELL STRUCTURES, INFLOW PIPES & WELL VENT PIPING SHALL BE PRESSURE PIPE AND ALL JOINTS SHALL BE GASKETED TO WITHSTAND MINIMUM 20 PSI PRESSURES. ALL EXPOSED PIPE JOINTS AND FITTINGS SHALL BE SECURED TO RESIST MOVEMENT.

DRAINAGE WELL TESTING NOTES:

- GRAVITY DRAINAGE WELL SPECIFIC CAPACITY TESTING PROTOCOL (PUMP-IN TEST):
- THE FOLLOWING IS A PROTOCOL FOR TESTING CAPACITY OF EACH GRAVITY DRAINAGE WELL, ANY DEVIATION FROM THIS PROTOCOL MUST BE REVIEWED AND APPROVED IN WRITING BY THE ENGINEER OF RECORD (EOR). THE DRILLER WILL PROVIDE ALL MATERIALS AND PERSONNEL REQUIRED TO CARRY OUT THE PROTOCOL. AT NO POINT DURING THE PUMP-IN TEST IS THE SYSTEM TO BE PRESSURIZED. THE WELL CASING SHALL REMAIN OPEN TO THE AIR DURING THE TESTING OF THE WELL.
1. WATER USED TO TEST THE WELL SHALL COME FROM A POTABLE WATER SUPPLY (I.E. FIRE HYDRANT).
 2. TOTALIZER(S) AND/OR FLOW METER(S) WILL BE USED TO MEASURE AND RECORD THE RATE AT WHICH WATER IS DISCHARGED INTO THE WELL. THE DISCHARGE RATE USED TO TEST THE WELL SHALL BE EQUAL TO OR GREATER THAN 250 GALLONS PER MINUTE.
 3. THE FLOW RATE SHALL NOT VARY BY MORE THAN 5% OVER THE TESTING PERIOD. THE DRILLER WILL PROVIDE A LOG DOCUMENTING FLOW RATES DURING THE TEST AT AN INTERVAL NO LESS THAN ONCE PER MINUTE.
 4. WATER WILL BE DISCHARGED INTO THE WELL BELOW THE STATIC WATER LEVEL THROUGH AN OPEN END HOSE OR MACHINE SLOTTED, SCHEDULE 40 PVC WELL SCREEN.
 5. WATER LEVEL FLUCTUATIONS WITHIN THE WELL WILL BE MEASURED WITH A CALIBRATED PRESSURE TRANSDUCER WITH AN APPROPRIATE PSI RATING, WATER LEVEL INDICATOR, OR OTHER APPARATUS APPROVED BY THE EOR WITH A PRECISION OF 0.01 FT.
 6. WATER LEVEL DATA FOR STATIC, PUMPING, AND RECOVERY CONDITIONS SHALL BE RECORDED NO LESS THAN ONCE PER MINUTE THROUGHOUT THE PUMP-IN AND RECOVERY PHASES OF THE TEST.
 7. DATA WILL BE COLLECTED DURING THE PUMPING CYCLE FOR A MINIMUM OF 30 MINUTES AND UNTIL THE LEVEL OF WATER WITHIN THE WELL HAS STABILIZED (WITHIN 5%) FOR A PERIOD OF AT LEAST 15 MINUTES.
 8. AT THE CONCLUSION OF PUMP-IN PHASE OF THE TEST, DISCHARGE INTO THE WELL SHALL CEASE AND THE WATER LEVEL IN THE WELL WILL BE ALLOWED TO RETURN TO STATIC CONDITIONS. WATER LEVELS WILL BE MEASURED DURING THE RECOVERY PERIOD, NO LESS THAN ONCE PER MINUTE, AND WILL BE LOGGED UNTIL THE WATER LEVEL HAS RETURNED TO STATIC LEVEL AND HAS STABILIZED FOR NO LESS THAN 5 MINUTES.
 9. DRILLER WILL USE THE DATA COLLECTED TO CALCULATE AND REPORT THE WELL SPECIFIC CAPACITY TO FDEP. ALL DATA COLLECTED WILL BE PROVIDED TO THE EOR.

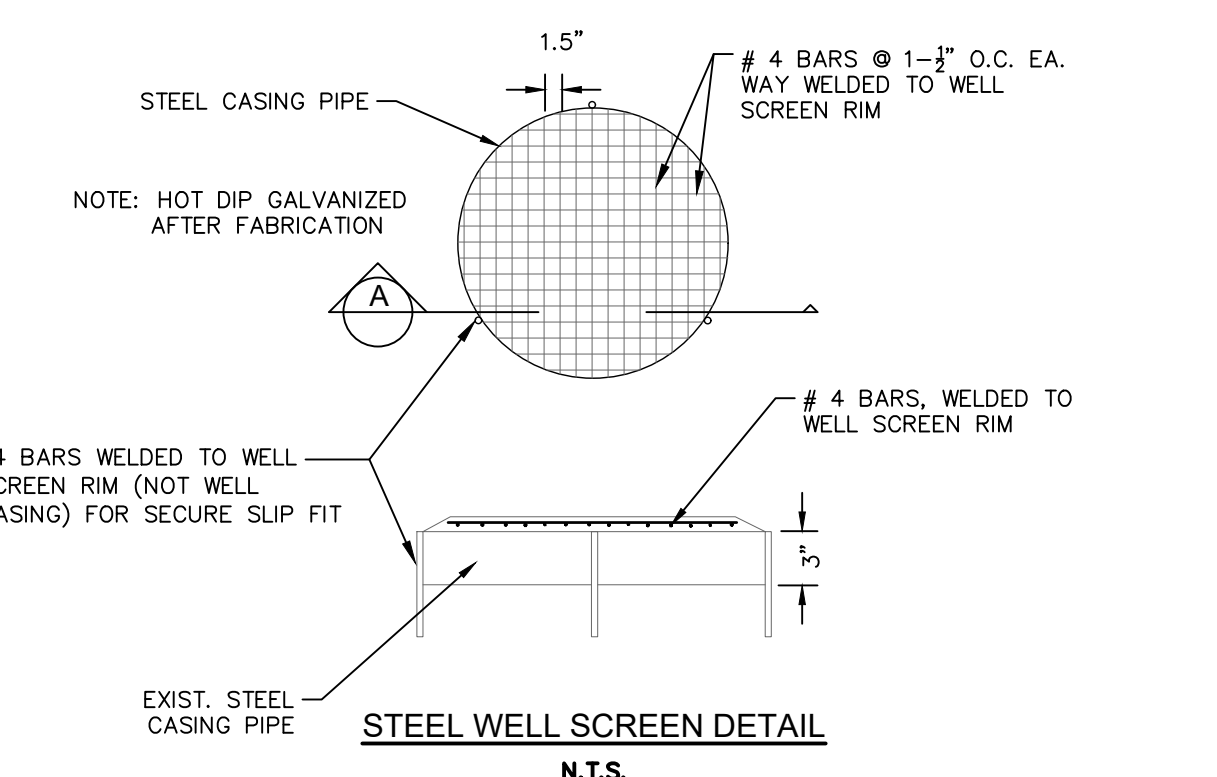


STORM DRAINAGE WELL #1

N.T.S.

DRAINAGE WELL ELEVATION TABLE (ALL ELEVATIONS ARE IN NAVD88)						
WELL NUMBER	A1*	A2*	B	C	D	E
1	8.76	7.86	6.86	4.25	1.25	-0.75

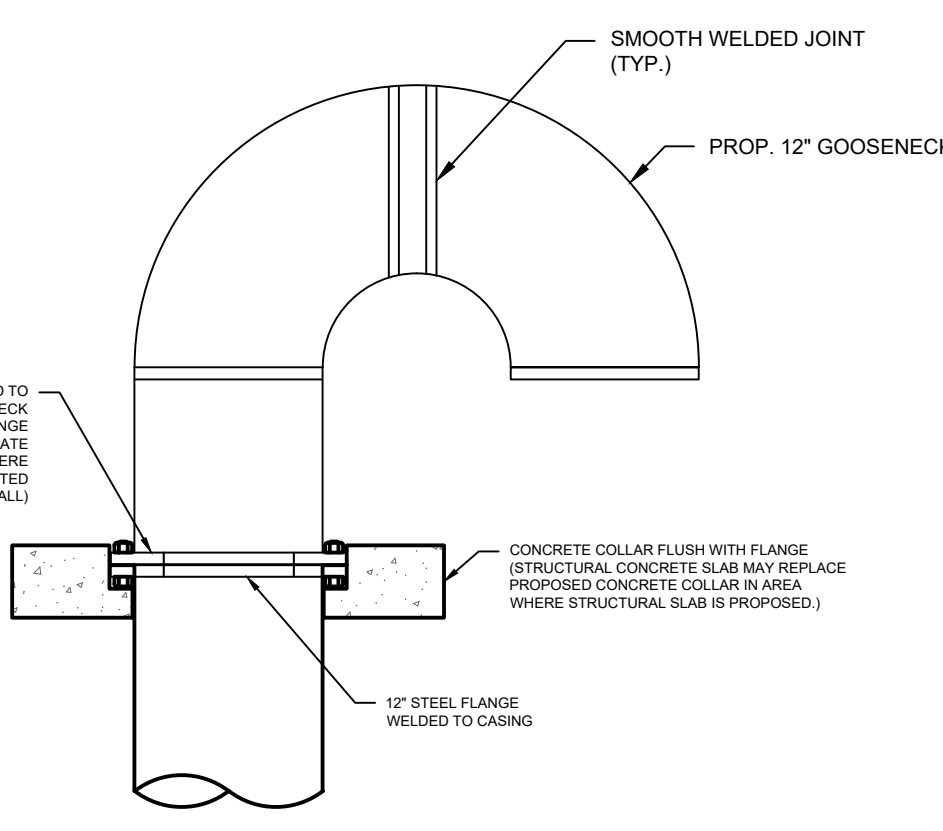
*A1 AND A2 ARE TO CENTER OF FRAME



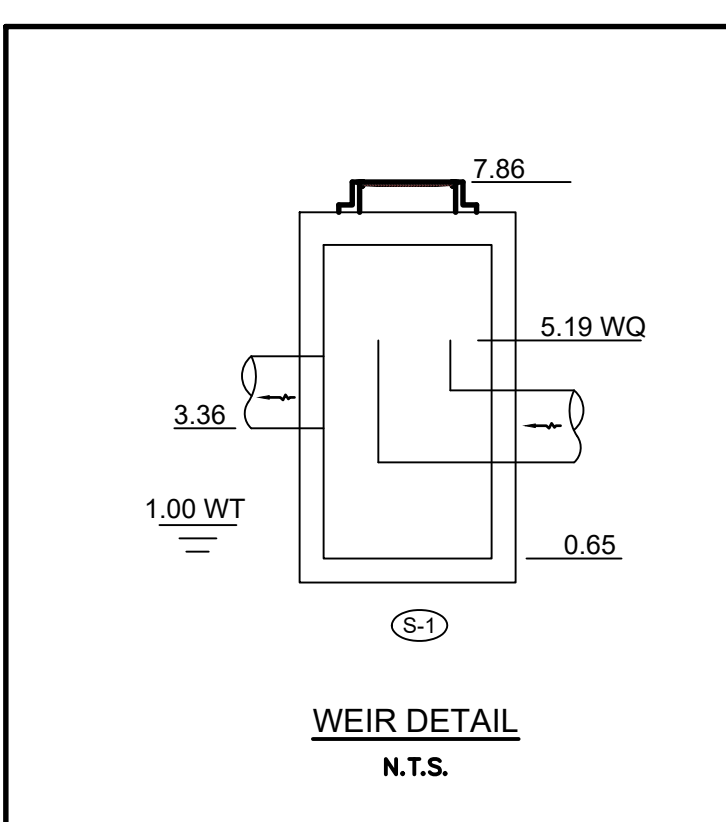
TEMPORARY SOLID WATERTIGHT CAP DETAIL

N.T.S.

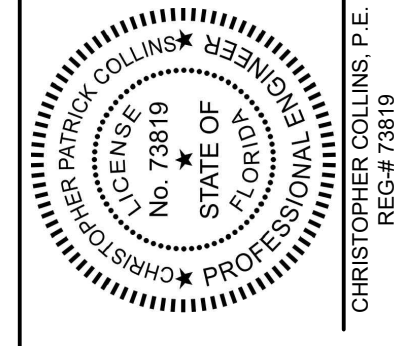
1. CASING SHALL BE CAPPED IMMEDIATELY AFTER INSTALLATION. CAP SHALL NOT BE REMOVED UNTIL WELL IS CLEARED FOR USE.
2. INJECTION WELLS SHALL NOT BE USED FOR DEWATERING



DRAINAGE WELL LOCATION TABLE		
WELL NUMBER	LATITUDE	LONGITUDE
1	26°00'11.9"N	80°08'36.1"W



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SUITE 247
NORTH MIAMI, FL 33161
Phone: 305.720.2079
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Comment
TAC Rev 1

Designed by: C.P.C.
Drawn by: A.T.S.

Checked by: C.P.C.
Approved by: C.P.C.

Scale: N/A
Date: 05/01/2023

Job No.: C010
© 2024

Plans for

THE GEORGE
950 S. FEDERAL HWY.
HOLLYWOOD, FLORIDA 33020

GRADING & DRAINAGE DETAILS

Sheet No.

C-9.1



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Check positive response codes before you dig!

NOTE

WWS APPROVAL OF THIS SET OF DRAWINGS IS RELIANT UPON THE DRAWINGS CLEARLY SHOWING ALL EXISTING AND PROPOSED ABOVE GROUND STRUCTURES, ASPHALT, PAVING, LANDSCAPING, WALLS, FENCES, UNDERGROUNDS PIPING, UNDERGROUND STRUCTURES, DUCT BANKS, TRANSFORMERS, POLES, STORM WATER STORAGE AREAS, PAVERS, ELECTRIC CABLE, AND OTHER UTILITY FACILITIES WITHIN RIGHTS OF WAY AND EXISTING AND PROPOSED POTABLE WATER/ RECLAIMED WATER/ WASTE WATER EASEMENTS. WWS HEREBY AUTHORIZES ONLY THE ITEMS SHOWN ON THESE DRAWINGS TO BE WITHIN SAID POTABLE WATER/ RECLAIMED WATER/ WASTEWATER EASEMENTS.

NOTE

CONSTRUCTION AND MATERIALS SHALL BE IN ACCORDANCE WITH WATER AND WASTEWATER SERVICES (WWS) STANDARDS AND SPECIFICATIONS. PLANS ARE IN ACCORDANCE WITH WWS MINIMUM DRAWING REQUIREMENTS DATED MARCH 2021. STATE PLANE COORDINATES WILL BE SUPPLIED FOR THE DESIGN AND RECORD DRAWINGS.

NOTE

RECORD DRAWINGS SHALL INCLUDE COLOR PHOTOGRAPHS OF ALL CONNECTIONS TO EXISTING WWS INFRASTRUCTURE AS WELL AS CRITICAL UTILITY CROSSINGS AND WHERE SPECIFICALLY REQUIRED ON THE DESIGN DRAWINGS. ALL PHOTOGRAPHS INCLUDED IN THE RECORD DRAWINGS WILL ALSO BE PROVIDED TO WWS IN JPEG FORMAT ON CD OR DVD MEDIA. SEE "MINIMUM DRAWING REQUIREMENT FOR PIPING PROJECTS".

NOTE

ELEVATIONS AS SHOWN WERE CONVERTED FROM THE NATIONAL GEOGETIC VERTICAL DATUM OF 1929 (NGVD 29) TO THE NORTH AMERICAN VERTICAL DATUM (NAVD 88) USING THE FOLLOWING CONVERSION: NGVD 29 VERTICAL DATUM (IN FEET) - 1.56' = NAVD 88 VERTICAL DATUM. THE CONVERSION FACTOR WAS OBTAINED FROM THE NATIONAL GEOGETIC SURVEY WEBSITE ORTHOMETRIC HEIGHT CONVERSION PROGRAM USING THE LATITUDE AND LONGITUDE OF THE PROJECT LOCATION TO DETERMINE A CONVERSION FACTOR FOR THIS SPECIFIC LOCATION.

NOTE

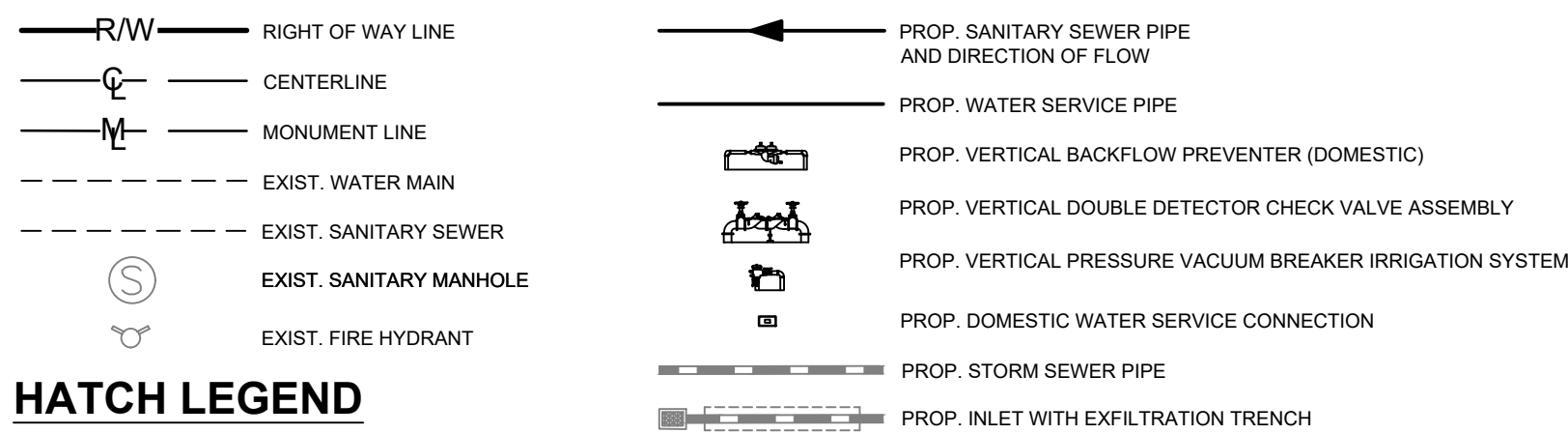
1. WATER SUPPLY AND ANY NEW HYDRANTS SHALL BE IN PLACE PRIOR TO ACCUMULATION OF COMBUSTIBLE MATERIALS PER NFPA 1 (2018 ED.) SECTION 16.4.3.1.1.
2. ALL UNDERGROUND FIRE MAIN WORK MUST BE COMPLETED BY FIRE PROTECTION CONTRACTOR HOLDING A CLASS I, II, OR V LICENSE PER FS 633.102.

NOTES

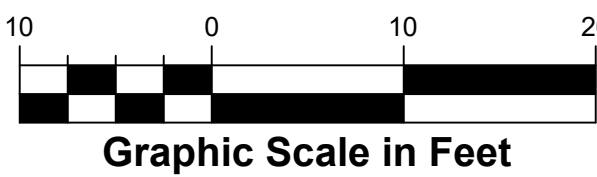
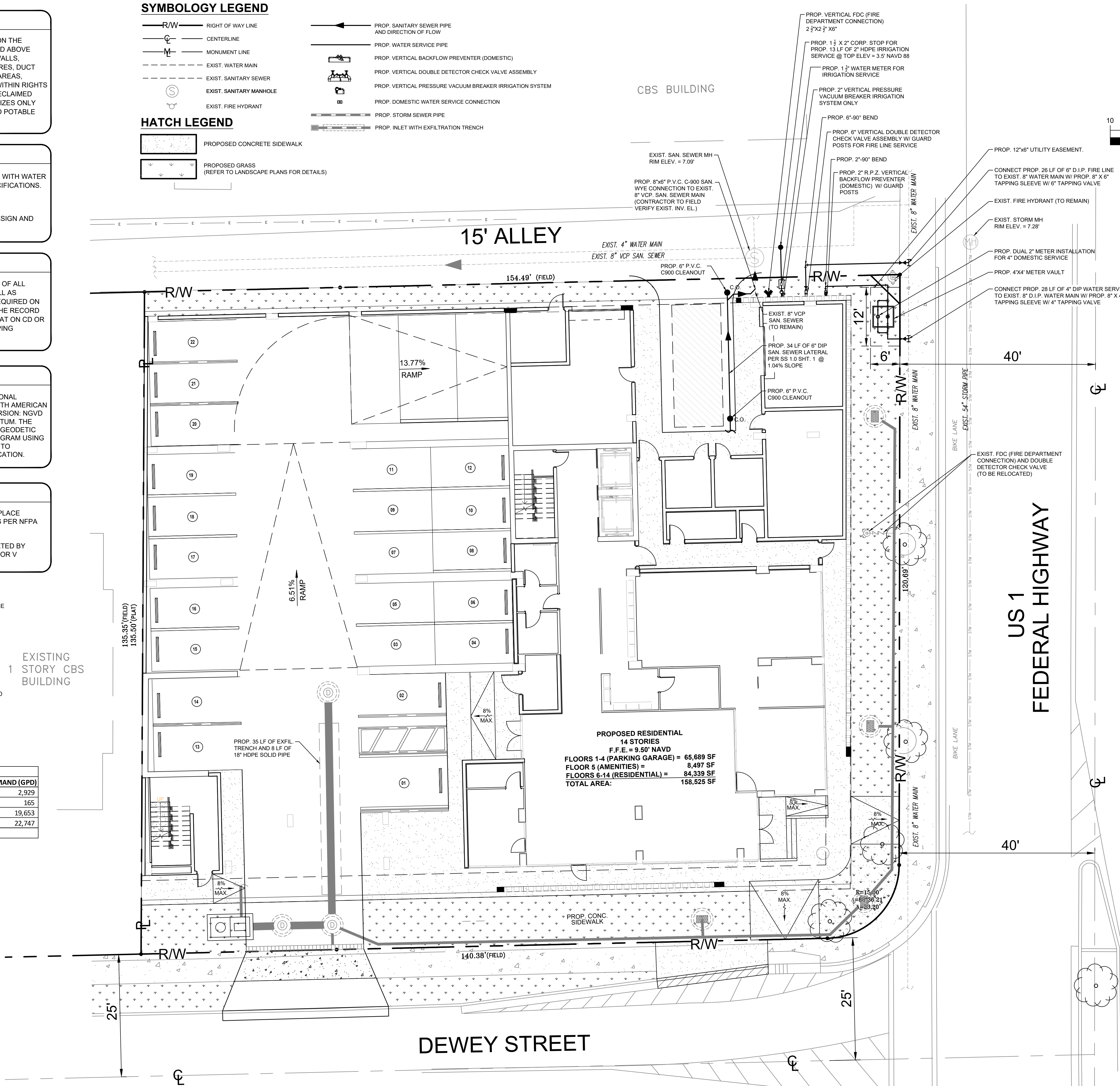
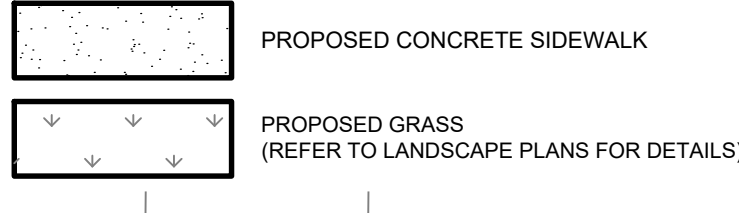
1. ALL CONSTRUCTION WORK AND MATERIALS SHALL BE IN ACCORDANCE WITH BROWARD COUNTY WATER AND WASTEWATER SERVICES (BOWWS) STANDARDS AND SPECIFICATIONS.
2. ALL EXISTING MAINS BEING IMPACTED BY THIS PROJECT AND ALL PROPOSED WATER AND SEWER FORCE MAINS AND FITTINGS, TO BE RESTRAINED.
3. 2.5 DEGREE MAXIMUM DEFLECTION, TYPICAL.
4. CONTRACTOR TO VERIFY LOCATION AND INVERT ELEVATION OF EXISTING WATER, FORCE MAIN AND ALL EXISTING UTILITIES PRIOR TO COMMENCEMENT OF WORK.
5. ALL REMOVED FIRE HYDRANTS MUST BE RETURNED TO BOWWS.

WATER AND SEWER DEMAND CALCULATION				
USE	UNITS	QTY.	ERU	DEMAND (GPD)
OFFICE	SF	1,450	0.212/1000	2,929
FITNESS CENTER	SF	1,650	10 GPD/100 SF	165
APARTMENTS	UNIT	113	0.707	19,653
TOTAL DEMAND				22,747
ERU (EQUIVALENT RESIDENTIAL UNIT FACTOR) = 246 GPD				

SYMBOLOLOGY LEGEND



HATCH LEGEND



PERMIT SET
MUST BE ON JOBSITE AT ALL TIMES DURING CONSTRUCTION

NOTICE
INSPECTION REQUIRED
24 HRS. PRIOR TO COMMENCING ANY WORK IN THE PUBLIC ROW CONTACT THE BROWARD COUNTY HIGHWAY CONSTRUCTION AND ENGINEERING DIVISION AT 954-577-4600 FOR INSPECTION.

NOTE:
APPROVAL OF THIS PLAN DOES NOT CONSTITUTE A PERMIT FOR CONSTRUCTION.
A PERMIT FOR CONSTRUCTION MUST BE OBTAINED FROM THE BROWARD COUNTY HIGHWAY CONSTRUCTION AND ENGINEERING DIVISION PRIOR TO COMMENCING CONSTRUCTION IN THE PUBLIC RIGHT OF WAY.
ALL MATERIALS USED AND INSTALLATIONS WITHIN THE PUBLIC RIGHT OF WAY OR EASEMENTS SHALL BE IN ACCORDANCE WITH BROWARD COUNTY HIGHWAY CONSTRUCTION AND ENGINEERING DIVISION SPECIFICATIONS.

BROWARD COUNTY HIGHWAY CONSTRUCTION AND ENGINEERING DIVISION

☐ PLAN CONSISTENT WITH PLAT REQUIREMENTS
☐ PUBLIC RIGHT OF WAY APPROVAL FOR PAVING, GRADING AND DRAINAGE

BY: _____ DATE: _____
DOES NOT INCLUDE APPROVAL OF PAVEMENT MARKING & SIGNS

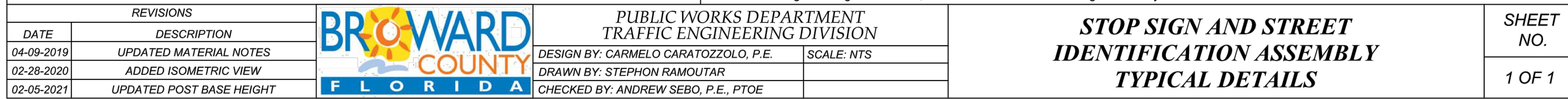
Revision Date	
Comment	
Revision Date	
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Revision Date	
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Revision Date	
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Revision Date 02/26/2024	
Comment	
TAC Rev 1	
Designed by:	C.P.C.
Drawn by:	A.T.S.
Checked by:	C.P.C.
Approved by:	C.P.C.
Scale:	1" = 10'
Date:	05/01/2023
Job No.:	C010
© 2024	
Plans for	

THE GEORGE
950 S FEDERAL HWY.
HOLLYWOOD, FLORIDA 33020

UTILITY CONNECTION PLAN

Sheet No.
C-10.0

Sunshine811
Call 811 or visit sunshine811.com two full business days before digging to have buried facilities located and marked.
Check positive response codes before you dig!

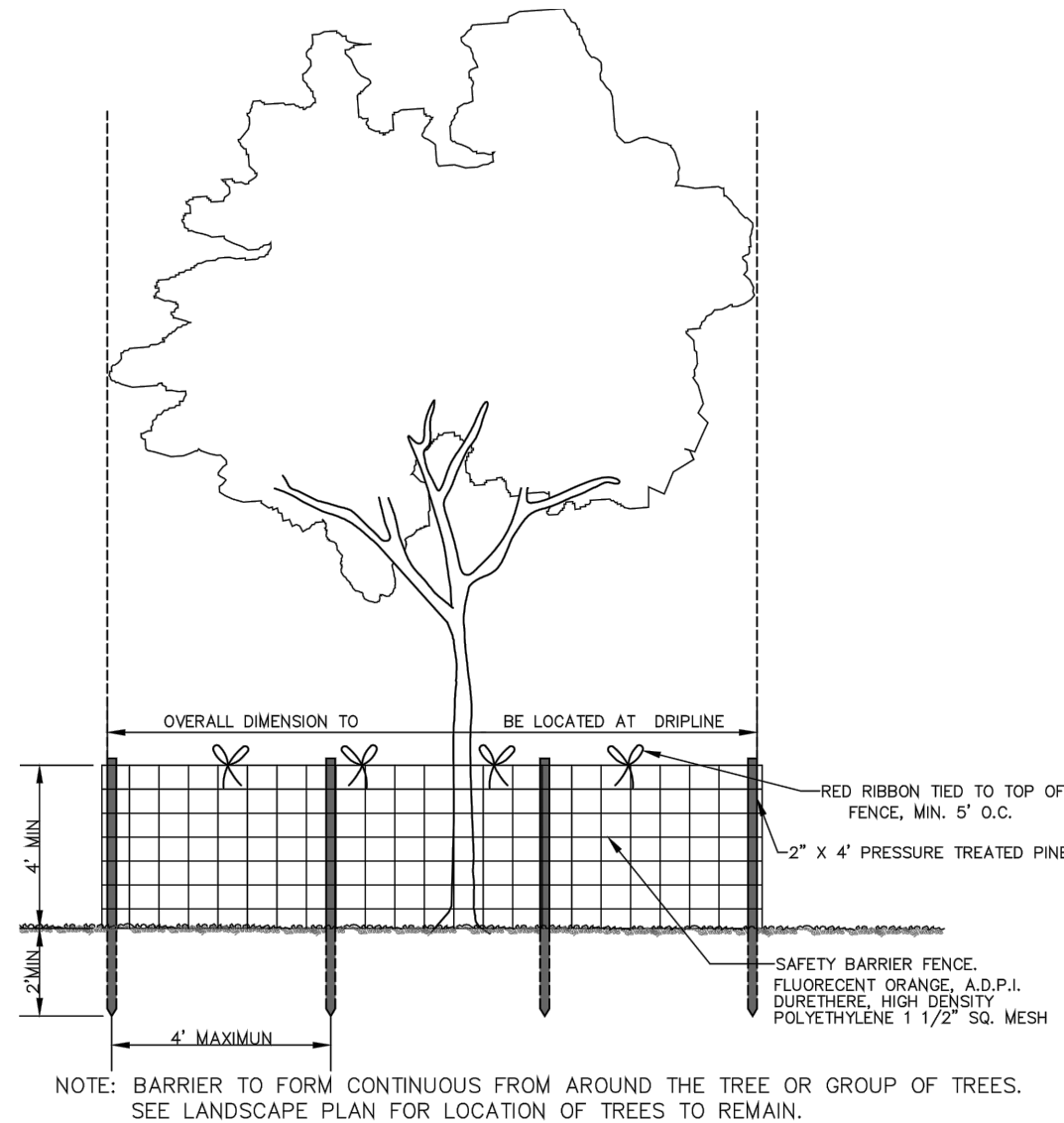


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TREE DISPOSITION NOTES

- CONTRACTOR TO VISIT SITE AND REVIEW PLANS PRIOR TO SUBMITTING A PROPOSAL TO OWNER. CONTRACTOR SHALL VERIFY SITE AND TREE INFORMATION, AND BRING ANY DISCREPANCIES WITHIN THE PLANS TO THE ATTENTION OF THE LANDSCAPE ARCHITECT FOR CLARIFICATION PRIOR TO SUBMITTING A PROPOSAL. BY PLACING A BID OR SUBMITTING A PROPOSAL TO DO THE WORK HEREIN CONTRACTOR ACKNOWLEDGES HE HAS REVIEWED THE PLANS, VISITED THE SITE AND FOUND NO MAJOR CONFLICTS.
- THESE PLANS WERE PREPARED BASED ON THE BEST INFORMATION AVAILABLE AT THE TIME OF DESIGN. ALL FINAL PLANS SHALL BE COORDINATED WITH FINAL APPROVED SITE PLAN.
- CONTRACTOR TO VERIFY ALL UNDERGROUND UTILITIES BEFORE WORK COMMENCES AND SHALL PROTECT ALL UNDERGROUND/ABOVE GROUND UTILITIES AND EXISTING CONDITIONS-TO-REMAIN DURING CONSTRUCTION.
- THE TREE REMOVAL WORK HEREIN WILL REQUIRE MITIGATION IN ACCORDANCE WITH CITY OF HOLLYWOOD ARTICLE 9 TREE MITIGATION REQUIREMENTS.
- CONTRACTOR TO COMPLETELY REMOVE ALL PARTS OF TREES SPECIFIED FOR REMOVAL ON THE TREE DISPOSITION PLAN. GRIND ALL TRUNKS/ROOT SYSTEMS OR TREES TO BE REMOVED A MINIMUM OF 18" DEPTH IN THEIR ENTIRETY AND FILL AND COMPACT WITH SUITABLE CLEAN SOIL TO FINAL GRADE.
- LANDSCAPE ARCHITECT RESERVES THE RIGHT TO MAKE CHANGES IN MATERIAL, QUANTITIES AND PROJECT SCOPE TO CONTRACTED WORK.
- IN THE EVENT OF DISPUTE, THE LANDSCAPE ARCHITECT'S INTERPRETATION SHALL BE FINAL.
- ALL WORK TO BE IN ACCORDANCE WITH ALL APPLICABLE REGULATIONS.
- CONTRACTOR TO LEAVE SITE COMPLETELY CLEAN, RESTORED, AND FREE OF DEBRIS. CONTRACTOR TO REPAIR IN FULL ANY DAMAGE CAUSED BY WORK OR MOBILIZATION.
- FOR ALL TREES TO BE REMOVED, CONTRACTOR TO TAKE PROPER CARE IN REMOVAL TO NOT CAUSE DAMAGE TO EXISTING SITE FEATURES, CONDITIONS, INFRASTRUCTURE, OR THE GENERAL PUBLIC AND PASSERSBY. COMPLETELY REMOVE TREES AND PROPERLY DISPOSE OF REMAINS OFF-SITE.
- NO TREES SHALL BE REMOVED OR RELOCATED UNTIL A CITY OF HOLLYWOOD TREE REMOVAL PERMIT IS ISSUED. APPLICATIONS ARE AVAILABLE IN ROOM 308 OF CITY HALL OR VIA WEBSITE DOWNLOAD AT WWW.HOLLYWOODFL.ORG > DEPARTMENTS > ENGINEERING > ENGINEERING SERVICES
- ALL TREES TO REMAIN SHALL BE PROTECTED IN PLACE BY A TREE PROTECTION BARRIER FENCE ERECTED TO THE EXTENTS OF THE CANOPY DRIPLINES. SEE LANDSCAPE DETAILS SHEET FOR ADDITIONAL INFORMATION. FAILURE TO MAINTAIN THE BARRIERS MAY RESULT IN DAMAGE TO TREES SPECIFIED TO REMAIN, ESPECIALLY ONES CLOSE TO ACTIVE CONSTRUCTION, WHICH MAY RESULT IN TREE MITIGATION COSTS, ADDITIONAL PERMITTING TIME AND COSTS, OR REJECTION OF TREES AT TIME OF FINAL INSPECTION. TREE PROTECTION BARRIERS ARE TO REMAIN IN PLACE, TO THE EXTENTS OF THE TREE DRIPLINES, FOR THE ENTIRE DURATION OF CONSTRUCTION.

TREE PROTECTION FENCE DETAIL

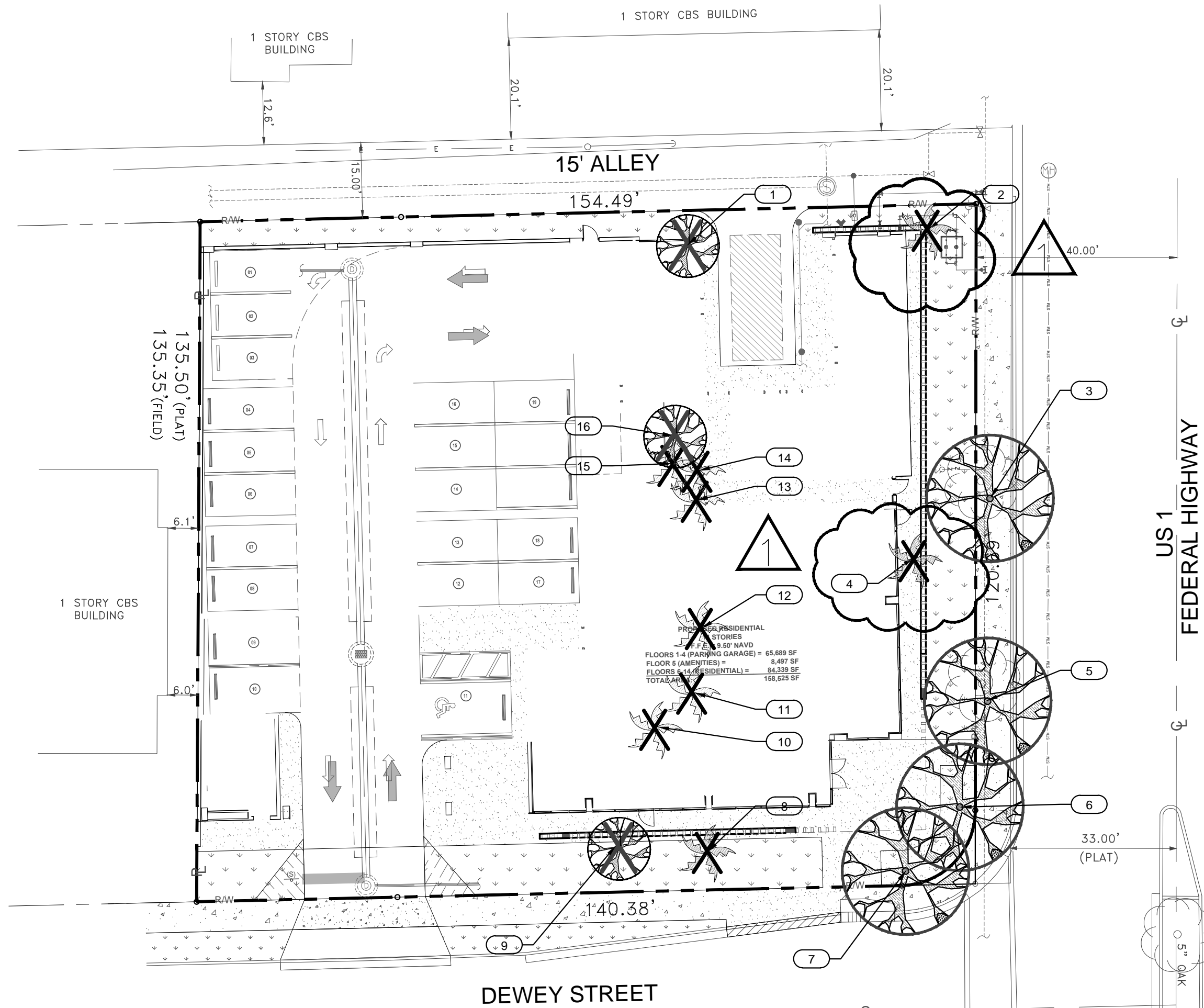


TREE MITIGATION & REPLACEMENT CHART

REGULATING DOCUMENT: HOLLYWOOD LAND DEVELOPMENT REGULATIONS		
ARTICLE 9		
	REQUIRED	PROVIDED
MONOCOTS (PALMS)	1:1 REPLACEMENT @ 9 PALMS	9 PALMS
DICOTS – REMOVALS	INCH PER INCH CALIPER REPLACEMENT @ 72"	72" = 36 2" DBH 12' HT. TREES OR \$350 PER REQUIRED TREE VALUE DONATED TO CITY OF HOLLYWOOD TREE TRUST FUND \$12,600.00 DUE TO SITE CONSTRAINT DONATION TO TREE TRUST FUND OF \$12,600.00

TREE SYMBOL LEGEND

- EXISTING TREE TO REMAIN**
SEE TREE PROTECTION FENCING DETAIL FOR FENCING TO BE ERECTED AND MAINTAINED DURING ENTIRE CONSTRUCTION PERIOD
- EXISTING TREE TO BE REMOVED**
REMOVE ENTIRE ROOT SYSTEMS AND FILL/LIGHTLY COMPACT/GRADE WITH SUITABLE SOIL. PROPERLY DISPOSE OF ALL PARTS OFF SITE
- EXISTING PALM TO BE REMOVED**
COMPLETELY REMOVE ENTIRE ROOT SYSTEMS AND FILL/LIGHTLY COMPACT/GRADE WITH SUITABLE SOIL. PROPERLY DISPOSE OF ALL PARTS OFF SITE
- EXISTING PALM TO REMAIN** SEE TREE PROTECTION FENCING DETAIL FOR FENCING TO BE ERECTED AND MAINTAINED DURING ENTIRE CONSTRUCTION PERIOD



TREE DISPOSITION PLAN

TREE DISPOSITION TABLE

No.	BOTANICAL NAME	COMMON NAME	DBH	HT. x SPR.	CONDITION	DISPOSITION	MITIGATION REQ'D
1	Swietenia mahagoni	Mahogany	18"	32' x 36'	85%	REMOVE	YES
2	Phoenix canariensis	Canary Island Date Palm	20"	10' x 12'	70%	REMOVE	YES
3	Quercus virginiana	Live Oak	12"	28' x 30'	80%	REMAIN	NO
4	Phoenix canariensis	Canary Island Date Palm	20"	28' x 8'	50%	REMOVE	YES
5	Quercus virginiana	Live Oak	15"	27' x 28'	75%	REMAIN	NO
6	Quercus virginiana	Live Oak	15"	26' x 26'	70%	REMAIN	YES
7	Quercus virginiana	Live Oak	25"	25' x 26'	70%	REMAIN	YES
8	Phoenix canariensis	Canary Island Date Palm	16"	8' x 10'	75%	REMOVE	YES
9	Persea americana	Avocado Tree	18"	26' x 26'	75%	REMOVE	YES
10	Veitchia merrillii	Christmas Palm	10"	12' x 12'	70%	REMOVE	NO
11	Veitchia merrillii	Christmas Palm	10"	12' x 12'	80%	REMOVE	NO
12	Veitchia merrillii	Christmas Palm	10"	8' x 10'	75%	REMOVE	YES
13	Veitchia merrillii	Christmas Palm	10"	12' x 12'	80%	REMOVE	YES
14	Veitchia merrillii	Christmas Palm	10"	12' x 12'	80%	REMOVE	YES
15	Veitchia merrillii	Christmas Palm	10"	12' x 12'	85%	REMOVE	NO
16	Swietenia mahagoni	Mahogany	36"	32' x 34'	75%	REMOVE	NO
TOTAL DIAMETER INCHES TO BE MITIGATED = 72" + 8 PALMS SEE SHEET SHEET L-201 FOR TREE PROTECTION FENCING DETAILS							

'N' - INDICATES A SPECIES NATIVE TO FLORIDA

TAC SUBMISSION 06/05/23

PROJECT #	23-103
DATE	04-22-23
SCALE	AS NOTED
DESIGNED BY	WDB
DRAWN BY	WDB
CHECKED BY	WDB

REVISIONS PER SITE PLAN AND REVIEW COMMENTS	02-26-24	DATE	BY
1			

THE GEORGE
950 S. FEDERAL HWY
HOLLYWOOD, FL

GREEN EARTH
LANDSCAPE ARCHITECTURE
HOLLYWOOD, FLORIDA
E-MAIL: dale.bryant@greenearthla.com PHONE: 954-538-1825

TREE DISPOSITION PLAN

SHEET NUMBER
L-100

LICENSED PROFESSIONAL
William Dale Bryant
FL LICENSE NUMBER
LA6666943

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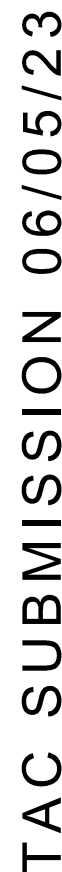
- TABULAR DATA CODE CHART (GROUND LEVEL)

*NOTE: ADDITIONAL LANDSCAPING AND TREES TO BE INCLUDED ON AMENITY DECK

*N

*N

BELOW IS AN AUTHORIZED DIGITAL SIGNATURE AND SEAL OF THE PROFESSIONAL LISTED. BY WAY OF THIS SIGNATURE I ATTEST THAT THE DOCUMENTS HEREIN, IF UNDER A VERIFIED ELECTRONIC SEAL, WERE PREPARED, SIGNED AND AUTHORIZED FOR SUBMITTAL BY ME ON THE DATE OF SIGNING, WITH ALL AUTHORITY AND RIGHTS GRANTED TO ME BY THE STATE OF FLORIDA SS-481.303 PART II AND ASSOCIATED REFERENCES THEREIN.



LICENSED PROFESSIONAL	William Dale Bryant FL LICENSE NUMBER LA6666943
-----------------------	---

[illegible]

PROJECT # 23-103
DATE 04-22-23
SCALE AS NOTED
DESIGNED BY WDB
DRAWN BY WDB
CHECKED BY WDB

THE GEORGE
950 S. FEDERAL HWY
HOLLYWOOD, FL

GREEN EARTH
LANDSCAPE ARCHITECTURE
HOLLYWOOD, FLORIDA



AMENITY LEVEL LANDSCAPE PLANTING PLAN

SHEET NUMBER
L-201

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LANDSCAPE NOTES & SPECIFICATIONS

- All proposed plant material shall be a minimum of Florida No. 1 or better as described in Grades and Standards for Nursery Plants, Parts I and II, latest edition published by the Florida Department of Agriculture and Consumer Services.
- All planted areas shall contain a three (3) inch layer of shredded sterilized Eucalyptus or Melaleuca mulch such as Flori-mulch equivalent. Keep mulch a min. 12" away from tree trunks and reduce thickness to 1-1/2" over all root systems. Create water-retentive soil/mulch tree rings/saucers around all new trees per landscape details. All newly-planted or relocated trees are to be set at a final grade so that rootballs/surficial root systems are approximately 3" above surrounding grades, or 10% of the depth of rootballs at time of planting/setting and settling.
- All sod or seed shall be certified true to nomenclature and varietal, weed and insect free, healthy and full in solid pieces at time of delivery and installation.
- Contractor shall be responsible for locating all existing utilities or other obstructions which may interfere with plant installation.Call Sunshine One service for underground utility locations 48 hours prior to any excavation or digging 1-800-432-4770. Verify all existing or new utilities and potential obstructions/conflicts prior to installation, and bring all potential conflicts to the immediate attention of the Landscape Architect.
- Corner clip and visibility triangles shall be maintained for clear sight visibility in accordance with all regulatory agency roadway safety design and guidelines regarding unobstructed and clear sight requirements.
- The landscape plans were designed to comply with Best Management Practices (BMP) including but not limited to the following: acknowledging and designing within Federal, State, County, Municipal, Sovereign and otherwise, rules and regulations regarding human and environmental safety; implementing accepted sound horticultural design practices with specific consideration to specie water needs, arrangement, spacing, sunlight tolerances, scale, urban/residential aesthetic design, heat gain reduction natural site opportunities and constraints, and in line with Florida Friendly Landscape Design principles; to comply with all prevailing regulatory requirements and restrictions, and intent.
- A Tree Permit will be required prior to any construction, native vegetative removal within the dripline of a tree, and/or site tree work.
- Local Utility and Fire Rescue Clearance Zones shall be provided around all above ground or at grade meters and equipment.
- Where overhead power lines exist, all proposed trees and palm setbacks shall conform with the Florida Power and Light (FPL) Guidelines for setbacks from overhead utility lines.
- Plant quantities are provided for convenience. In the event of discrepancies between plant list counts and plan, plan quantities and layout shall take precedence based on symbols and center-to-center spacing specified on plant list.
- All landscape planting areas shall have amended planting soil of uncompacted coarse loam that is a minimum of 12 inches deep. Soils shall be appreciably free of gravel, stones, rubble or trash. All compacted soil, contaminated soil or roadbase fill shall be removed. Amended planting soil shall be premixed off site and shall be a ratio by volume mixture of 50% pulverized topsoil/Florida muck organic content and 50% clean, screened sand, free of nematodes and vegetative growth. 10% sphagnum peat or other organic amenity is acceptable in the amended soil mix.
- All sod shall be Argentine cultivar of Bahiagrass sod (*Paspalum notatum* var. 'Argentine'). All sod shall be in solid, full pieces. Lay sod tight with staggered joints and roll if necessary to produce a smooth, even finish grade. Infill any gaps or voids in or between pieces with course sand. See Landscape Details for more information.
- All plant materials shall be guaranteed by the contractor for a period of one year to be in a thriving, healthy condition that is indicative of each species. Any plants that fail within that period shall be replaced immediately with like kind, size, and specs.
- Remove any and all prohibited exotic vegetation from site.
- All landscapes areas shall have an automatic irrigation system with rain sensor and programmable automatic controller as permitted by South Florida Water Management District, County, and Local Codes. Irrigation system shall provide 100% coverage with 50% overlap.
- Other than existing any existing trees specified to remain, all existing vegetation, turf, weeds, groundcover, and other deleterious materials are to be removed in their entirety prior to planting preparation. See landscape planting details and notes herein for amended soil mix and planting preparation.
- All pervious rights-of-way shall be prepared and sodded per detail on Landscape Details sheet. Contractor shall water by manual watering methods in sufficient volume and frequency all landscape materials until fully established immediately after installation, and shall supplement irrigation system before, during and after installation as needed to fully establish all plant materials until such time as they are self-sufficient, or until irrigation system is fully programmed and operational to provide necessary water for establishment.
- Contractor shall guarantee all plant materials to be in a healthy, thriving condition for a period of one (1) calendar year and shall replace any plant materials that are in a state of decline immediately with like kind and size. All staking & guys for trees and palms shall be removed approximately 9 months after planting or when fully established to be self-erect. This is to be performed by Contractor as part of the scope of work within this plan, and as hereby directed by the Landscape Architect of Record as an integral part of this plan.

TABULAR DATA CODE CHART

REGULATING DOCUMENT: HOLLYWOOD LAND DEVELOPMENT REGULATIONS		
ZONING LAND USE: FH-2 FEDERAL HIGHWAY MEDIUM-HIGH INTENSITY MIXED USE		
	REQUIRED	PROVIDED
1 TREE /1,000 SF PERVIOUS LANDSCAPE AREA @ 8,172	9 TREES	9 TREES
PERIMETER BUFFERS 1 TREE/20 LF OF BUFFER + LANDSCAPE ELEMENT MIN. 42" HT. @ 262	13 TREES + CONTINUOUS 42" HEDGE	13 TREES + CONTINUOUS 42" HEDGE
STREET TREES 1/30 LF OF STREET FRONTAGE @ 262 LF	9 TREES	9 TREES
TREE PRESERVATION/MITIGATION MONOCOTS (PALMS)	1:1 REPLACEMENT @ 9 PALMS	*9 PALMS
DICOTS - REMOVALS	INCH PER INCH CALIPER REPLACEMENT @ 72"	72" = 36 2" DBH 12' HT. TREES OR \$350 PER REQUIRED TREE VALUE DONATED TO CITY OF HOLLYWOOD TREE TRUST FUND \$12,600.00 DUE TO SITE CONSTRAINT DONATION TO TREE TRUST FUND OF \$12,600.00

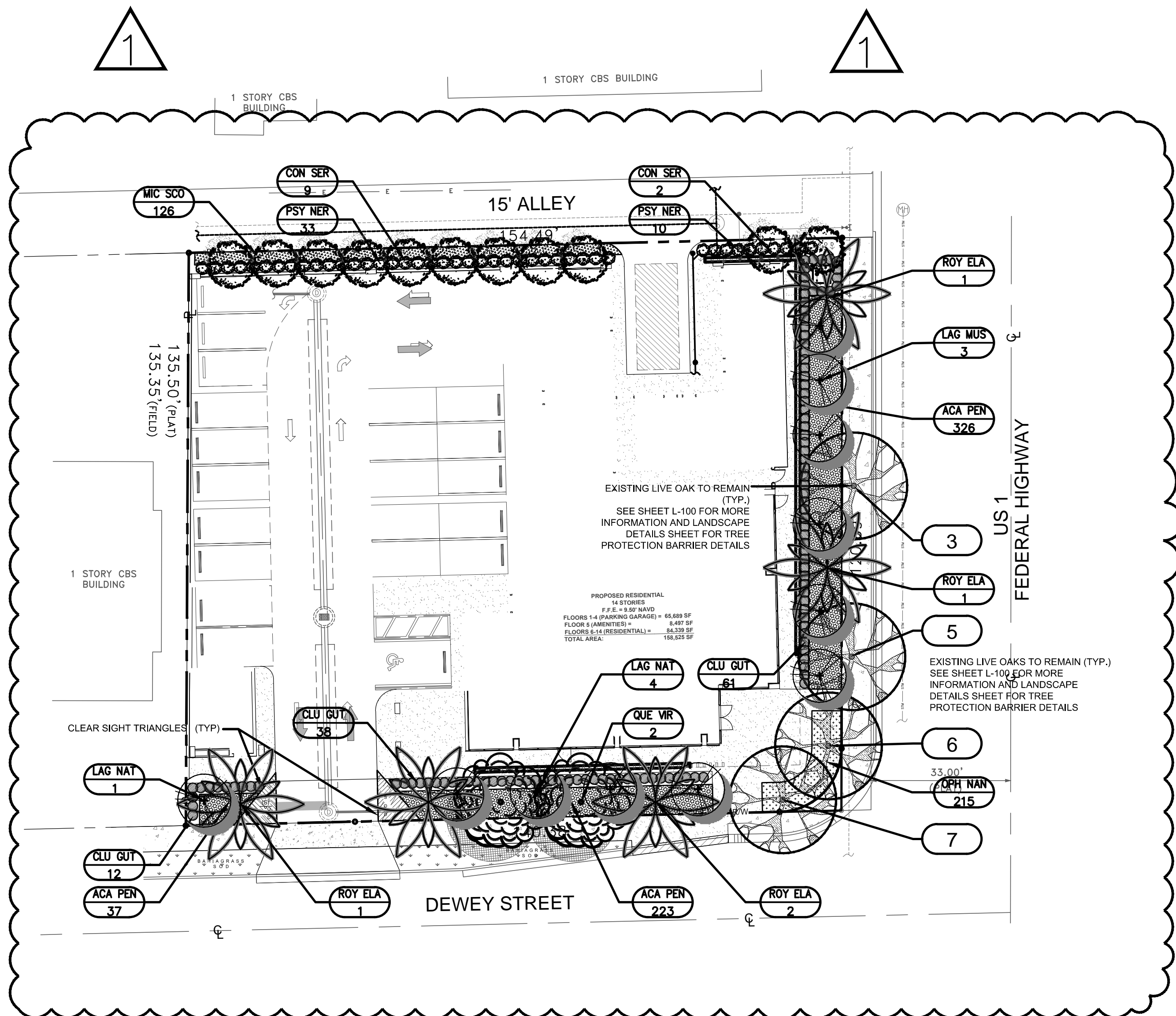
*NOTE: ADDITIONAL LANDSCAPING AND TREES TO BE INCLUDED ON AMENITY DECK

PLANT LIST AND SPECIFICATIONS

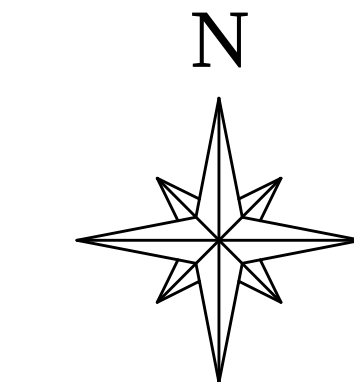
TREE AND PALMS						
SYMBOL	QUANTITY	LATIN NAME	COMMON NAME	SIZE	SPREAD	DESCRIPTION
N CON SER	9	Conocarpus erectus 'Sericeus'	SILVER BUTTONWOOD	2" DBH/12' HT.	5'	FIELD GROWN, SINGLE STRAIGHT STANDARD, HEAVY BRANCHING, STRONG CENTRAL LEADER
N LAG NAT	11	Lagerstroemia x 'Muskogee'	MUSKOGEE CRAPE MYRTLE	2" DBH/12' HT.	5'	FIELD GROWN, SINGLE STRAIGHT STANDARD, HEAVY BRANCHING, STRONG CENTRAL LEADER
N QUE VIR	2	Quercus virginiana	LIVE OAK	3" DBH/16' HT.	7'	FIELD GROWN, STRAIGHT HEAVY TRUNKS, FULL CROWNS
N ROY ELA	5	Roystonea elata	ROYAL PALM	18" GW	14'	FIELD GROWN, STRAIGHT HEAVY TRUNKS, NO PENCILING, FULL CROWNS
N CALLOUT	4	Quercus virginiana	EXISTING LIVE OAK	VARIES	VARIES	EXISTING TREES TO REMAIN (SEE SHEET L-100 TREE DISPOSITION PLAN)
SHRUBS AND GROUNDCOVER						
SYMBOL	QUANTITY	LATIN NAME	COMMON NAME	HEIGHT	SPREAD	DESCRIPTION
N ACA PEN	586	Acalypha pendula	DWARF CHENILLE	14" HT	16"	FULL, HEAVY FOLIAGE, SPACE 18" ON CENTER
N CLU GUT	111	Clusia guttifera	LITTLELEAF CLUSIA	42" HT	24"	FULL, HEAVY FOLIAGE, SPACE 24" ON CENTER
N MIC SCO	126	Microrum scolopendria	WART FERN	16" HT	18"	FULL, HEAVY FOLIAGE, SPACE 18" ON CENTER
N OPH NAN	215	Ophiopogon japonica 'Nana'	DWARF MONDO GRASS	8"	10"	FULL, LINER, STAGGERED 12" O.C. TO FILL PLANTER AREA
N PSY NER	43	Psychotria nervosa	WILD COFFEE	42" HT	30"	FULL, HEAVY FOLIAGE, SPACE 36" ON CENTER

N - ABOVE DENOTES FLORIDA NATIVE SPECIES

NOTE: ADDITIONAL LANDSCAPING AND TREES TO BE INCLUDED ON AMENITY DECK



LANDSCAPE PLANTING PLAN



0 20 40
SCALE FEET

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TAC SUBMISSION 06/05/23

LICENSED PROFESSIONAL
William Dale Bryant
FL LICENSE NUMBER
LA6666943

PROJECT #	DATE	SCALE	AS NOTED	DESIGNED BY	WDB	REVISIONS	DATE	BY
23-103	04-22-23	SCALE	AS NOTED	DESIGNED BY	WDB	REVISIONS	DATE	BY
REVISIONS PER SITE PLAN AND REVIEW COMMENTS							02-26-24	WDB

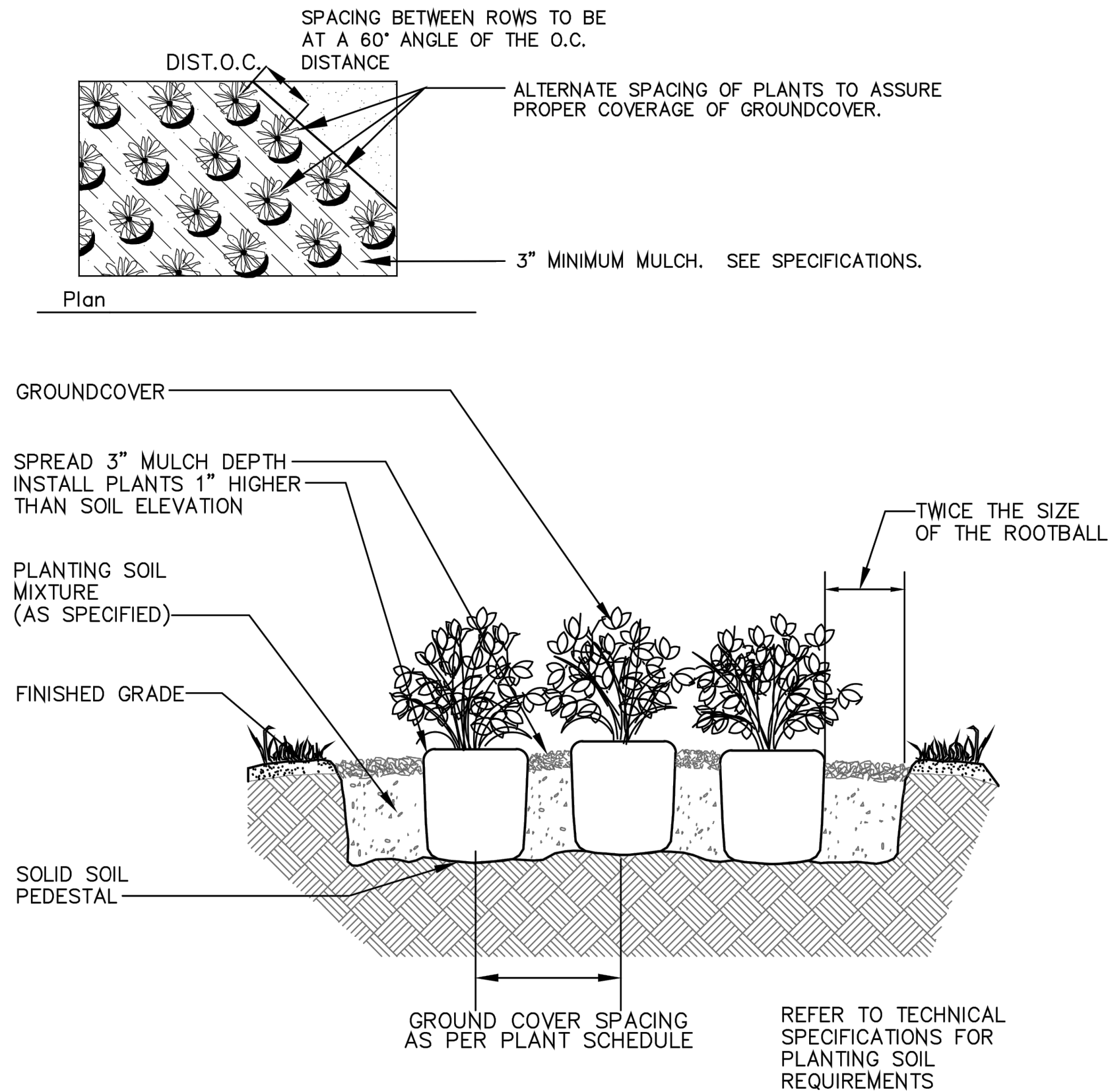
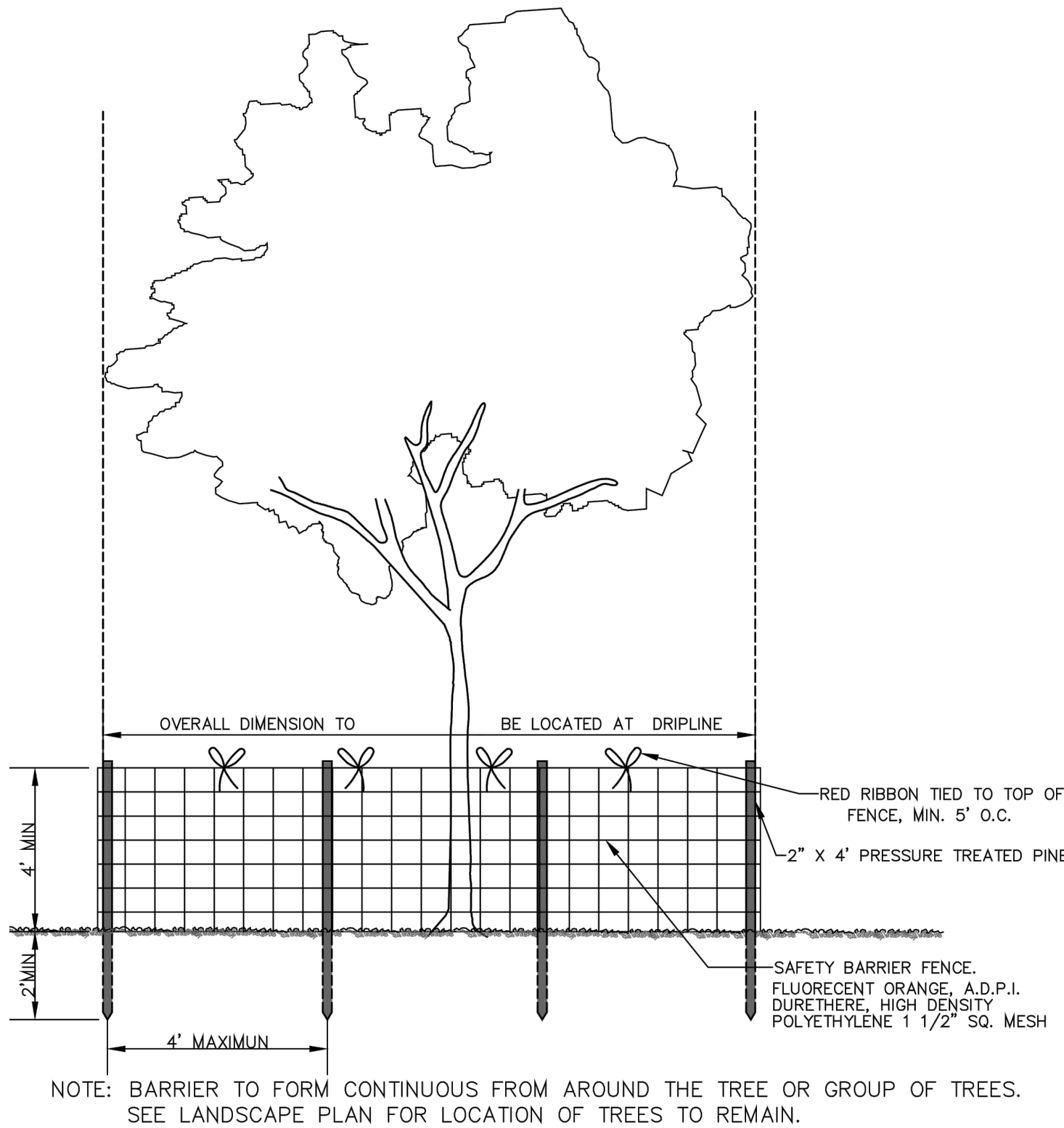
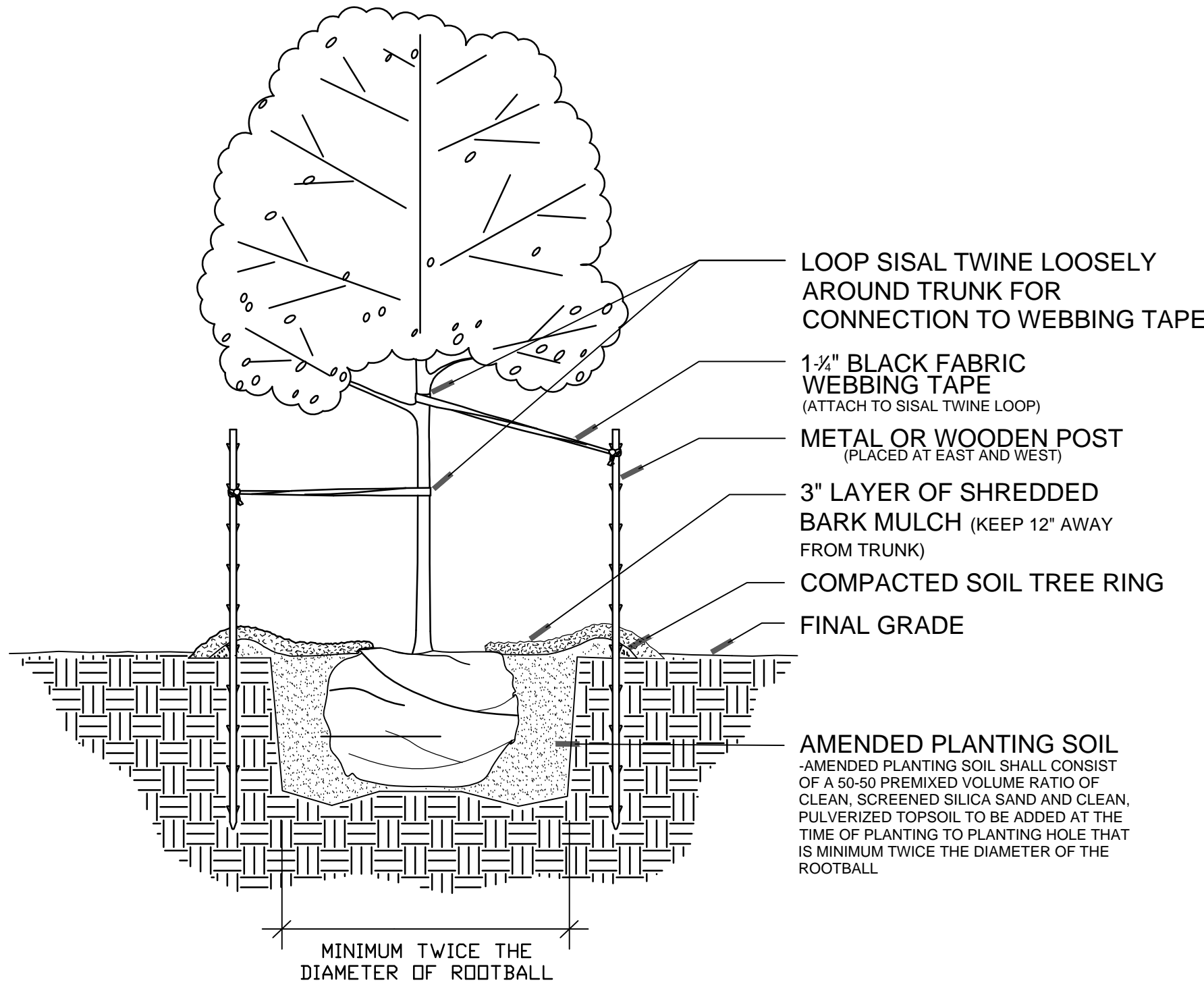
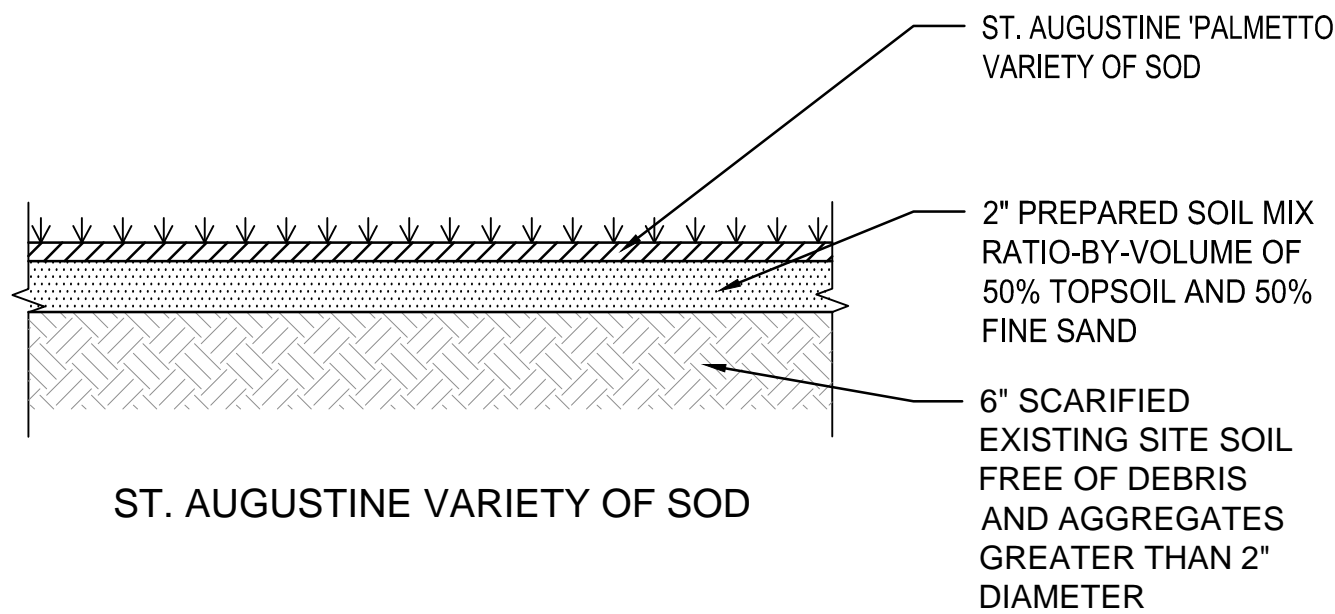
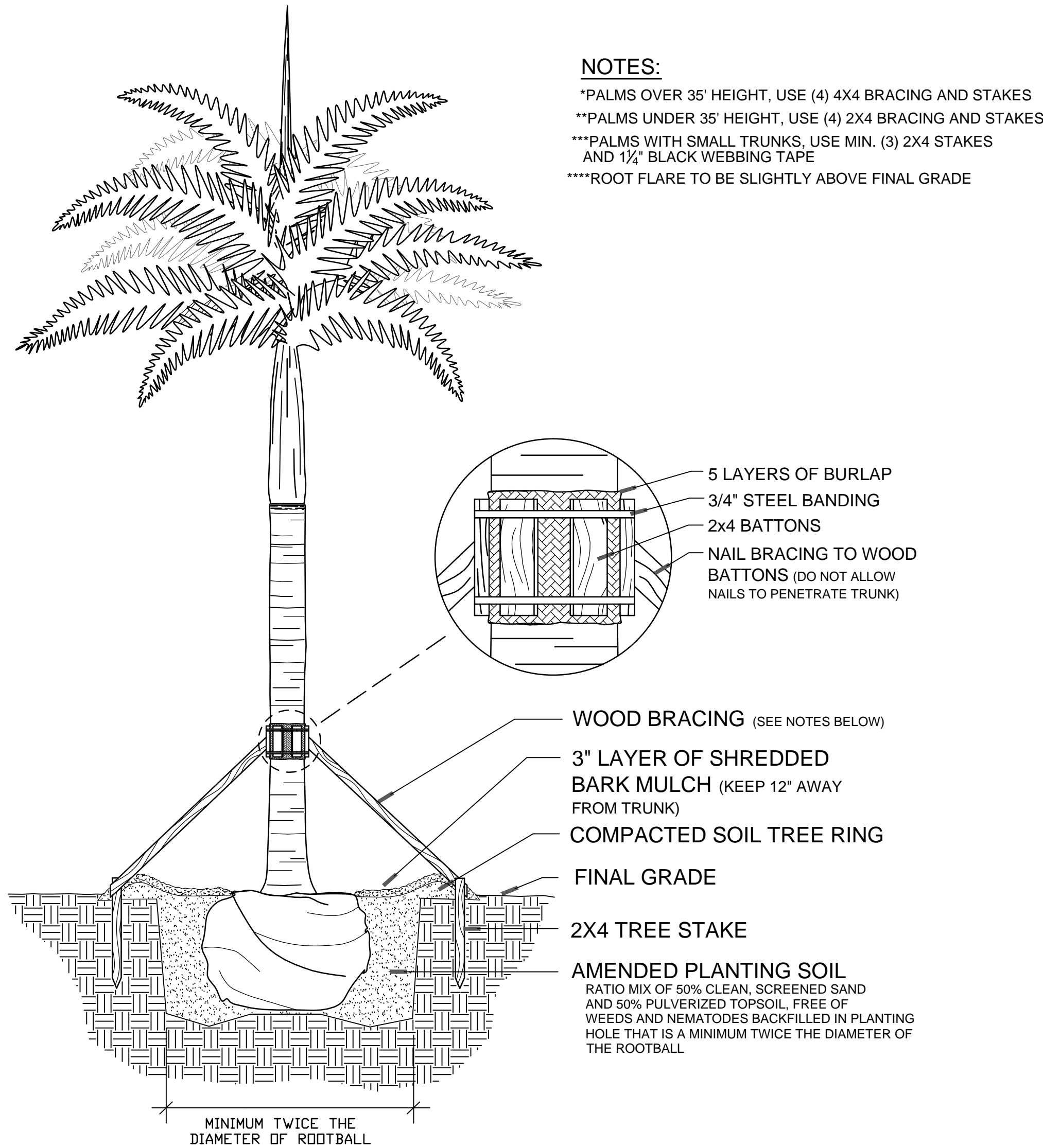
THE GEORGE
950 S. FEDERAL HWY
HOLLYWOOD, FL

GREEN EARTH
LANDSCAPE ARCHITECTURE
HOLLYWOOD, FLORIDA
E-MAIL: dale.bryant@greenearthla.com PHONE: 954-638-7423

LANDSCAPE
PLANTING PLAN

SHEET NUMBER
L-200

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TAC SUBMISSION 06/05/23

PROJECT #	23-103	DATE	04-22-23	SCALE	AS NOTED	DESIGNED BY	WDB	DRAWN BY	WDB	CHECKED BY	WDB	SHEET NUMBER	REVISION	NO.	DATE	BY

THE GEORGE
950 S. FEDERAL HWY
HOLLYWOOD, FL
BROWARD COUNTY
FLORIDA

GREEN EARTH
LANDSCAPE ARCHITECTURE
HOLLYWOOD, FLORIDA
E-MAIL: dale.bryant@greeneearth.com PHONE: 954-638-9825

LANDSCAPE DETAILS
SHEET NUMBER
L-202



June 1, 2023

Conkreta Group

2980 NE 207 Street, Suite 409
Aventura, FL 33180

Attention: Mr. Jonathan Brief, CEO
Email: ibk@conkretagroup.com
Phone: (305) 503-7899

Re: Report of Geotechnical Engineering Services
14-Story Building
950 Federal Hwy.
Hollywood, FL 33020
PACIFICA Project No.: 320-23214

Dear Mr. Brief:

Pacifica Engineering Services, LLC. (PACIFICA) has completed a geotechnical engineering study for the above-referenced project. The scope of geotechnical services was completed in general accordance with PACIFICA Proposal No. 610-12013657 dated May 4, 2023. Authorization to proceed was given via signature of the proposal on May 4, 2023.

PACIFICA appreciates the opportunity to provide geotechnical engineering services on this project and looks forward to an opportunity to participate in construction-related aspects of the development. If you have any questions or should additional information, be required, please do not hesitate to contact our office at (561) 419-8460.

Sincerely,

Pacifica Engineering Services

Florida Certification of Authorization License No. 32328

A handwritten signature in blue ink, appearing to read "Maximo Peralta".

Maximo Peralta Alvarez, P.E.
Senior Geotechnical Engineer
FL License No. 84213

A handwritten signature in blue ink, appearing to read "Wesley C. Foster".

Wesley C. Foster, P.E.
Principal Engineer
FL License No. 83239

TABLE OF CONTENTS

1.0 GENERAL PROJECT INFORMATION	1
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APPENDIX:

Sheet 1:	Site Vicinity Map
Sheet 2:	Boring Location Plan – Aerial
	Boring Location Plan – Site
	Boring Logs
	Percolation Test Results
	Moment vs. Depth Graphs from Lateral Pile Analyses

1.0 GENERAL PROJECT INFORMATION

The project is located at 950 Federal highway, Hollywood, Florida. It is understood that plans for this project include the design and construction of a fourteen-story building. A site vicinity map which shows the general location of the project site is located on Sheet 1 of the Appendix.

Structural loading was not provided at the time of this report; therefore PACIFICA has assumed maximum columns and wall loads of 2,500 kips and 4 kips/linear foot, respectively. Similarly, site grading information was not provided, therefore PACIFICA has assumed existing site grades will be within 2 feet of final site grades.

If any of the information in this report is incorrect or has changed, please notify PACIFICA so that we may check the recommendations presented in this report. PACIFICA will not be held responsible if not given the opportunity to check the recommendations once final designs have been complete.

2.0 GEOTECHNICAL EXPLORATION

2.1 Geotechnical Boring

Four (4) Standard Penetration Test (SPT) geotechnical borings were performed to assess the subsurface conditions. The borings were advanced to a depth of 80 feet with respect to the site grades at the time of the geotechnical exploration. A boring location plan can be found on Sheet 2 of the Appendix.

The SPT borings were performed using a CME-55 truck mounted geotechnical drilling rig equipped with an automatic hammer utilizing mud rotary drilling techniques. The SPT samples were collected continuously in the upper 10 feet and at 5-foot intervals thereafter until final boring depth was reached.

After the samples were collected in the split barrel sampler they were bagged, labeled and transported back to the laboratory for description and limited testing. After the geotechnical borings were completed, they were backfilled using access auger cuttings and the ground surface was generally leveled.

2.2 Subsurface & Groundwater Conditions

The individual boring log included in the Appendix shows the various soil types and stratifications. The transition between soil strata may be gradual and not as definitive as it appears on the boring log. If the contractor cannot determine the soil strata during construction the geotechnical engineer should be consulted.

Groundwater was encountered at depths of 6.0 to 7.0 feet at the time of the geotechnical exploration. The groundwater table may vary due to high/low tide fluctuations, rainfall, runoff, droughts or the infiltration rate of the soil and therefore the contractor should verify the groundwater table prior to construction.

2.3 Percolation Tests

One percolation test was performed at boring location B-4 to a depth of 10 feet below grade at the time of the geotechnical exploration. The percolation tests were performed in general accordance with the South Florida Water Management District (SFWMD) procedures for the "Usual Condition Constant Head" Percolation Test. The boreholes were advanced using a 3-inch diameter casing. A 2-inch diameter perforated PVC pipe was placed in the boreholes prior to retrieving the casing. Water was then pumped into the boreholes in order to raise the water level as close to the ground surface as possible. Once the inflow equalized with the outflow rate, the average pumping rate and level of the water for this stabilized flow rate was recorded. The results of the percolation tests are included in the Appendix section of this report. Table 1 below shows the condensed results, the full results located in the Appendix should be used when the exfiltration trenches are being designed. It should be noted that the designer should apply an appropriate factor of safety to the reported values.

TABLE 1: PERCOLATION TEST RESULTS

Boring ID	Date	Depth of Test (ft)	Depth of Groundwater Prior to Test (ft)	Average Flow Rate (GPM)	K, Hydraulic Conductivity (CFS/ft²-ft)
B-4	05-16-2023	10	6.0	4.0	4.0E-04

3.0 GEOTECHNICAL CLASSIFICATION

The soil samples taken from the geotechnical boring were taken back to the laboratory and visually reviewed by a geotechnical engineer. The soil samples were classified using the Unified Soil Classification System (USCS) in general accordance with the American Society of Testing and Materials (ASTM) test designation D2487.

4.0 FOUNDATION RECOMMENDATIONS

4.1 Geotechnical Discussion

Based on the assumed relatively high structural loads in Section 1 of this report, proximity to adjacent buildings, and the results of the geotechnical exploration, the proposed structures could be supported on augercast piles. PACIFICA should be given an opportunity to review the final foundation design and construction sequences.

PACIFICA understands that miscellaneous lightly loaded structures may be required on this project. The details and locations of these structures are unknown at the time of this report. PACIFICA can provide structure and location specific recommendations if requested and provided the details and locations. For the purposes of this report, general recommendations for these lightly loaded structures are detailed in Section 4.4 and consist of shallow foundations.

4.3 Augercast Pile Recommendations

Augercast piles may be used to support the proposed structures. Augercast piles are known for their ease of construction when considering deep foundation systems. Additionally, augercast piles can be installed with minimal noise and vibration to the surrounding area and nearby foundation systems. The capacity of these foundations systems is reliant on the side (skin) friction that is developed along the grouted lateral area of the pile and soil strata.

The piles should be extended into the lower sand stratum. The estimated top depth of the lower sand stratum is about 50 feet below grade and is based on the geotechnical field exploration. The Augercast Pile contractor, with the assistance of a PACIFICA pile inspector should verify the depth of the competent lower sand stratum during the installation of the test piles.

Tables 2 and 3 were generated based on the geotechnical exploration and the provided loads referenced in Section 1 of this report. The following table lists allowable pile capacities for varying pile diameters and pile lengths. Once final foundations systems have been determined, PACIFICA should be notified to revise and/or amend the recommendations in this report in conjunction with the project Structural Engineer.

TABLE 2: AUGERCAST PILE ALLOWABLE AXIAL CAPACITIES

Diameter (in.)	Estimated Pile Length (ft.)	Allowable Pile Capacities (tons)		Vertical Spring Constant (kip/in.)	Min. Grout Strength (psi)
		Compression	Tension		
18	75	290	145	580	8,000
24	75	380	190	760	8,000

Notes:

1. Estimated Pile Lengths are based on site grades at the time of the geotechnical exploration and the anticipated depth of the lower sand stratum. Once structural plans are finalized PACIFICA should be retained to provide structure specific estimated pile lengths.
2. Estimated Pile Lengths should be met.
3. PACIFICA assumes that the structural design of the augercast piles will include appropriate reinforcing to resist compression, uplift and/or lateral forces as noted herein and should be designed by the structural engineer.
4. These capacities are based on the above referenced grout strength. The Structural Engineer should determine the minimum compressive strength for the grout in accordance with applicable standards.
5. Load tests will be required for each pile diameter used for the support of the proposed buildings. Load test requirements are detailed in subsection 6.1.4.

TABLE 3: AUGERCAST PILE ALLOWABLE LATERAL CAPACITIES

Diameter (in)	Allowable Lateral Pile Capacity for 0.375-inch Lateral Movement (tons)	Allowable Lateral Pile Capacity for 0.25-inch Lateral Movement (tons)
18	9.5	7.5
24	17	14

Notes:

1. Lateral pile capacities were calculated using the L-Pile computer program.

2. A fixed head condition was used to model the piles.
3. For 18-inch diameter piles: six (6) #6 steel reinforcement bars were evenly spaced circularly with 3 inches of cover.
4. For 24-inch diameter piles: six (6) #8 steel reinforcement bars were evenly spaced circularly with 3 inches of cover.
5. 60 kips per square inch (ksi) steel was used in the analyses.
6. These analyses were performed assuming a combined axial and lateral load condition.
7. These analyses assume that the recommended center to center spacing is three pile diameters.

Maximum moment versus depth graphs is provided in the Appendix Section of this report. The graphs were generated using the L-Pile computer program with the assumptions outlined in the noted of Table 3.

Settlement of the above augercast pile systems is anticipated to be on the order of one inch, with differential settlement between adjacent piles on the order of ½ inches. Settlement of the augercast pile systems is governed by the loads listed in Section 1 of this report, the allowable axial loads given in this section, and the results of the geotechnical exploration.

Augercast piles should be spaced at least three pile diameters, center to center, away from one another to reduce group effects. PACIFICA understands that a pile spacings of 2.5D are being considered. If pile spacings of 2.5D are used, then a reduction factor of 0.67 shall be used for the provided pile capacities. Full length steel reinforcement is required to provide some assurance that the augercast piles have been constructed with a continuous cross section. The full-length reinforcement should be installed immediately after grouting in the center of the augercast pile using centralizers, also known as spacers, located at intervals approved by the Geotechnical and Structural Engineer. The reinforcement should be designed by the Project Structural Engineer to resist uplift, lateral and compressive forces.

The 28-day compressive strength of the grout used in the augercast piles should be determined by the Structural Engineer.

4.4 Shallow Foundations for Miscellaneous Structures

Shallow foundations with an allowable bearing pressure of 3,000 pounds per square foot and placed at a minimum of 18 inches below finished floor elevation may be used to support one- to two-story miscellaneous structures with maximum loads less than 120 kips. Prior to placement of the shallow foundations, properly compacted existing granular soils or structural fill soils should be placed within the footprint of each shallow foundation. Recommendations in the following section should be followed. As a minimum, the foundation size should follow the Florida Building Code.

In order to prevent against punching shear failure, each individual shallow foundation should have a minimum width of 3 feet. Designers should use a friction factor of 0.55 for base shearing resistance and values calculated from this shearing resistance should be considered ultimate. Passive earth pressure resistance should be computed using an equivalent fluid pressure of 180 pounds per square foot per foot of depth, for granular materials. Additional soil values the designer may require for uplift design are the unit weight and submerged unit weight of the soil, 120 pcf and 58 pcf, respectively.

Settlement estimates were determined based on an assumed maximum column load of 120 kips for miscellaneous structures and the allowable bearing capacity detailed in the beginning of this

section. Please contact PACIFICA if the assumed loads are inaccurate. PACIFICA is not responsible nor will be held liable if the assumed loads in this report are inaccurate. Settlement estimates were determined from empirical relationships and based on experience with similar projects. Based on the geotechnical exploration, the required compaction recommendations in this report and the items listed in this section, an estimated settlement of 1 inch or less was calculated. Differential settlement between adjacent shallow foundations may be approximately $\frac{1}{2}$ the total estimated settlement. Due to the elastic characteristics of granular soils the settlement should be observed as soon as the live loads are applied. These settlements are usually considered to be tolerable for lightly loaded structures. However, if more stringent settlements are required then PACIFICA should be contacted to revise the recommendations given in this report.

4.2 Ground Floor Slab Recommendations

Ground floor slabs can bear directly on top of compacted structural fill material that is compacted based on requirements in Section 5.0. A modulus of subgrade reaction value of 150 pounds per cubic inch (pci) may be used for design. The underside of the ground floor slab should be lined with a vapor barrier at least 6 mil thick. A friction factor of 0.21 (ultimate) can be used for the soil to vapor barrier interface.

5.0 SITE PREPARATION RECOMMENDATIONS

5.1 General Site Preparation

The results of the field investigation and experience from similar projects have yielded the following recommendations detailed in steps. These steps should be carried out by an experienced contractor adhering to current standards. It should be noted that these steps should be performed with care to not damage any adjacent structures or any underground utilities.

- 1) Prior to construction activities on-site underground utilities should be identified and marked in the field. If any of the existing utility lines are in conflict with the proposed construction, then plans should be made to relocate these lines. If the utilities discovered are abandoned or out of service, they should be removed.
- 2) Topsoil, asphalt, concrete, unsuitable material or any other debris should be stripped to expose in-situ soils. If any old foundations from previous structures are encountered, then they should be removed in their entirety. If deep foundations are encountered the geotechnical engineer should be retained to assess the impact on the proposed structures and to make recommendations on mitigation.
- 3) The exposed on-site soils in the building(s) footprint should be properly compacted per the recommendations in the following section. The compaction should extend 5 feet beyond the perimeter of the building.
- 4) The contractor should prepare for heavy excavation activities as limerock and limestone was encountered at relatively shallow depths. If limestone is encountered, it may require special tools/buckets in order to break the material up.
- 5) Any unsuitable material or debris encountered should be removed and replaced with structural fill.
- 6) Any fill needed to bring up the site to the proper elevation, including any reference to structural fill in this report, should adhere to the recommendations given the section entitled Structural Fill Soils.
- 7) A representative of the geotechnical engineer should be present and properly document these activities.

- 8) Any other geotechnical related questions should be directed to the geotechnical engineer of record.

The contractor should adhere and be aware of all OSHA and any regulatory standards during construction activities. The contractor is responsible and held solely liable if these standards are not upheld.

5.2 Dewatering Recommendations

Dewatering may be required on this site if deeper excavations are warranted. Groundwater may be pumped out using pumps or other processes to at least 2 feet below and compaction activities. Well point systems may be used if deeper excavations are required for proper and safe construction. A specialty contractor should design these systems and adhere to any regulatory standards.

5.3 Compaction Recommendations

Once initial site clearing has been performed the exposed in-situ soils in the building(s) footprint should be properly compacted until the surface is firm and unyielding. The compaction should extend 5 feet beyond the perimeter of the building. Care should be taken when compacting adjacent to existing structures. A self-propelled vibratory roller no less than 40,000 pounds should be used to compact the exposed in-situ soils. The proof rolling should be observed by PACIFICA to identify and mitigate any weak subgrade conditions.

If any locations of in-situ soils overly deflect under the weight of the roller then the soils should be removed to a depth of 24 inches and replaced with properly compacted structural fill materials. The structural fill soils should be compacted to 95% of the Modified Proctor maximum dry density per ASTM D1557. Wetting of the subgrade soils may be used in order to achieve proper compaction.

5.4 Compaction Recommendations for Areas Close to the Existing Structures

Once initial site clearing has been performed, the exposed in-situ soils in the building(s) footprint should be properly compacted until the surface is firm and unyielding. The compaction should extend 5 feet beyond the perimeter of the building. Care should be taken when compacting adjacent to existing structures. The in-situ soils should be compacted using a self-propelled vibratory roller, in static mode, no less than 40,000 pounds. The proof rolling should be observed by PACIFICA to identify and mitigate any weak subgrade conditions.

If any locations of in-situ soils overly deflect under the weight of the roller then the soils should be removed to a depth of 24 inches and replaced with properly compacted structural fill materials. The structural fill soils should be compacted to 95% of the Modified Proctor maximum dry density per ASTM D1557. Wetting of the subgrade soils may be used in order to achieve proper compaction.

5.5 Structural Fill Soils

Structural fill soils should be inorganic and consist of granular material containing less than 12 percent passing the U.S. Standard No. 200 mesh sieve, a maximum particle size of 3 inches and have a Unified Soil Classification System (USCS) designation of GP, GW, GP-GM, GW-GM, SP, SW, SP-SM, or SW-SM. The structural fill material may be composed of either clean sands and/or limerock. The use of "Cyclone Sand" is not permitted.

Density tests should be performed by a qualified technician working under the supervision of the geotechnical engineer and be in accordance with the appropriate ASTM standards. The representative of the geotechnical engineer should be present and agree with the placement and compaction of all structural fill materials.

Loose lifts not exceeding 12 inches should be performed on all structural fill materials. The lifts should be compacted to 95% of the Modified Proctor (ASTM D1557). If a small vibratory plate or roller is used, then loose lifts should not exceed 8 inches.

5.6 Shallow Foundation Construction

Based on the results of the geotechnical exploration, the following recommendations apply to the soils beneath the building(s) foundations. In-situ or imported structural fill soils at the base of the footing excavations should be compacted to at least 95% of the Modified Proctor (ASTM D1557). If any locations of in-situ or imported structural fill soils overly deflect under the weight of the compactor/roller, then the soils should be removed to a depth of 24 inches and replaced with properly compacted structural fill materials.

6.0 CONSTRUCTION RECOMMENDATIONS

6.1 Augercast Pile Construction Recommendations

The following sections detail construction considerations and recommendations for the installation, construction and testing of the augercast pile foundation systems. PACIFICA recommends that a representative of the geotechnical engineering be retained to monitor and report the installation, construction, and testing of the augercast pile foundations systems.

6.1.1 Construction Verification Recommendations

Augercast Pile placement with respect to the construction drawings should be verified prior to the commencement of drilling. A placement tolerance of 3 inches is recommended. Plumbness should be limited to 2%, with respect to the vertical or design batter. The contractor should demonstrate their ability to check for plumbness and depth.

6.1.2 Drilling and Grouting Recommendations

Augercast piles are to be constructed by a contractor with 10 years of experience and can demonstrate they have installed augercast piles within the project vicinity. Piles are to be terminated at the planned tip depth or termination criteria by rotating hollow-stem continuous flight augers through the bearing stratum. Positive pressure during grouting is required before the auger can be slowly drawn up from the hole. Fluid and/or slurry return at the top of the hole is a good indication that positive pressure is being achieved. If the grout pressure suddenly drops the auger should be reinserted five feet below the level where the grout pressure dropped. Once positive pressure is achieved again the auger can continue being extracted from the hole.

Grout volumes upwards of 1.5 times the theoretical pile volume are not uncommon. If grout volumes less than 1.15, or greater than 1.5 are recorded, the Geotechnical Engineer should be contacted, and the installation of all augercast piles shall be stopped.

Ground subsidence may be observed if the grout expands laterally due to the soft soil strata and/or the weight of the grout column cause the grout to expand laterally. This is not uncommon and should be monitored and recorded. Ground subsidence of 6 inches within the first two hours of the commencement of grout placement can be typical and the contractor should pour fresh grout to achieve the required pile cut off elevation. If excess ground subsidence is observed within the first few hours or any time period thereafter, the Geotechnical Engineer should be contacted, and no further piles should be constructed.

Adjacent piles within six (6) pile diameters should not be installed on the same day or 24-hour period. The grout should be allowed to fully set so that the existing pile is not compromised by the installation of the new pile.

6.1.3 Augercast Pile Monitoring

Successful augercast piles are highly dependent on adequate installation, contractor expertise and successful monitoring. PACIFICA should be made available to review the pile installation plan and working drawings prior to the start of the augercast pile construction. Additionally, full-time observations are required for the entirety of the augercast pile installation and production. This full-time monitoring program will provide a level of assurance that the augercast piles were installed per the recommendations in this geotechnical report. Proper grout pump calibration is required prior to production pile installation. At least one set of six, two-inch cubes or three-inch diameter by six-inch high grout cylinders should be made for each 50 cubic yards of grout pumped.

6.1.4 Test Pile Program

PACIFICA recommends that at least one static compression load test and one static tensile load test be conducted for each pile diameter used on this project. Additionally, PACIFICA recommends one lateral load test for each pile diameter that is used for lateral resistance. The location of the test piles should be positioned at non-production locations. Full scale static compression tests should be performed pursuant to ASTM D1143. Static tensile load tests should be performed pursuant to ASTM D3689. Lateral Load Tests should be performed pursuant to ASTM D3966. All tests should be performed pursuant to the requirements of the aforementioned ASTM references and the Florida Building Code under the direct supervision of a Geotechnical Engineer from PACIFICA. The reaction piles that are required for the load tests may be positioned at production pile locations.

It is recommended that the load test piles be fully instrumented with strain gauges. Based on the geotechnical exploration, a pair of strain gauges attached to the reinforcing cage at the depths outlined in Table 4 is recommended. By utilizing the strain gauge data from the load test for each pile diameter, PACIFICA will identify the load distribution characteristics of the various subsurface materials.

TABLE 4: RECOMMENDED STRAIN GAUGE LOCATIONS

Strain Gauge Number	Pile Strain Gauge Level
1	One Foot Below Grade
2	10 Feet Depth
3	15
4	20
5	30
6	40
7	45

8	50
9	60
10	70
13	One Foot Above Pile Tip

Notes:

1. The depths/levels for the strain gauges are based on the grades at the time of the geotechnical investigation and the assumption that the cut off elevation will be at grade.

Once final pile elevations and lengths are determined PACIFICA should be retained to modify/edit the strain gauge locations given in the above table.

The strain gauge data will confirm the load bearing and load deformation characteristics of the subsurface strata. Therefore, it is critical that the cut off elevation of the test pile be the same as that of the production piles. Based on the results of the load tests, the geotechnical engineer will provide additional installation criteria (i.e., revised bearing pile tip elevation, production grout volumes, etc.) for the production piles, if necessary. Testing of the piles should include the standard method of loading to at least twice the allowable compressive design value.

7.0 PAVEMENT RECOMMENDATIONS

Both flexible and rigid pavement sections may be used for this project. The pavement sections require a sub-base consisting of one or multiple layers. Traffic loading has been assumed for this project and the following recommendations have been made in the following table. Once final traffic loads and estimates have been made a civil engineer should review these general pavement recommendations given in Table 5 to check the validity.

TABLE 5: GENERAL PAVEMENT SECTION RECOMMENDATIONS

Type of Pavements	Sections/Layers	Section/Layer Thickness (in)
Flexible	Florida DOT Asphalt Type S	2.0
	Crushed limerock compacted to 98% of the Modified Proctor. A minimum LBR of 100 is required.	8.0
	Stabilized sub-base fill compacted to 98% of the Modified Proctor. A minimum LBR of 40 is required.	12.0
Rigid	Florida DOT Portland Cement Concrete	6.0
	Stabilized sub-base fill compacted to 95% of the Modified Proctor. A minimum LBR of 40 is required.	12.0

Notes:

1. Sub-base fill materials should meet the requirements presented in the latest revisions of the FDOT "Specifications for Road and Bridge Construction", Section 911.

The above pavement sections and thicknesses were based on assumed values taken from similar projects. A Civil Engineer should perform a design once traffic loading and estimates are finalized. The design pavement section shall meet minimum city/county requirements.

In any areas where dumpsters or heavy equipment are to be stored for extended periods of time, it is recommended that the rigid pavement section be utilized. Periodic maintenance should be expected for the lifetime of these pavement systems.

8.0 REPORT LIMITATIONS

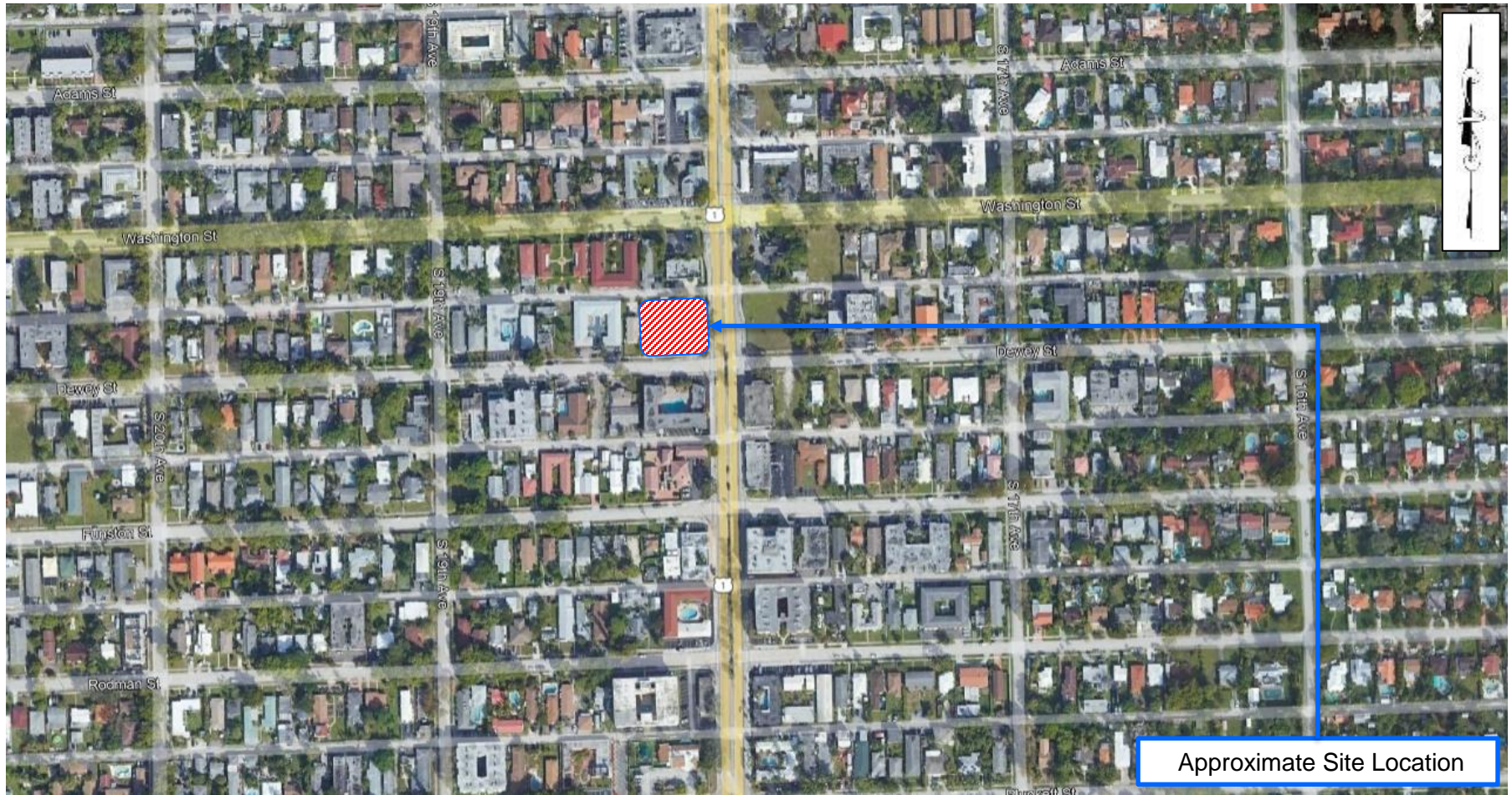
These geotechnical engineering services have been performed, findings obtained, and recommendations prepared in accordance with generally accepted geotechnical engineering principles and practices at the time of this report. PACIFICA is not responsible for the conclusions, opinions or recommendations made by others based on this data. No other warranties are implied or expressed.

After the plans and specifications are complete, PACIFICA should be provided the opportunity to review the final design and specifications, in order to verify that the earthwork and foundation recommendations are properly interpreted and implemented. At that time, it may be necessary to submit supplemental recommendations. If PACIFICA is not afforded the opportunity to participate in construction related aspects of foundation installation as recommended in this report, we can accept no responsibility for the interpretation of our recommendations made in this report or for foundation performance.

The scope of investigation was intended to evaluate soil conditions within the influence of the proposed foundations. The analyses and recommendations submitted in this report are based upon the data obtained from the soil borings performed at the locations indicated. If any subsoil variations become evident during the course of this project, a re-evaluation of the recommendations contained in this report will be necessary after we have had an opportunity to observe the characteristics of the conditions encountered. The applicability of the report should also be reviewed in the event significant changes occur in the design, nature or location of the proposed structures. The scope of our services did not include an environmental assessment for the presence or absence of hazardous or toxic materials in the soil and groundwater. Any statements in this report regarding odors, staining of soils, or other unusual conditions observed are strictly for the information of our client.

This report has been prepared for the exclusive use of Konkreta Group and their design consultants for the construction of the fourteen-story building located at 950 Federal Highway in Fort Lauderdale, Florida.

SITE VICINITY MAP



Approximate Site Location

GEOTECHNICAL ENGINEERING SERVICES

14-Story Building

950 S Federal Highway, Hollywood, Florida

DATE: 5/08/2023

DRAWN: MR

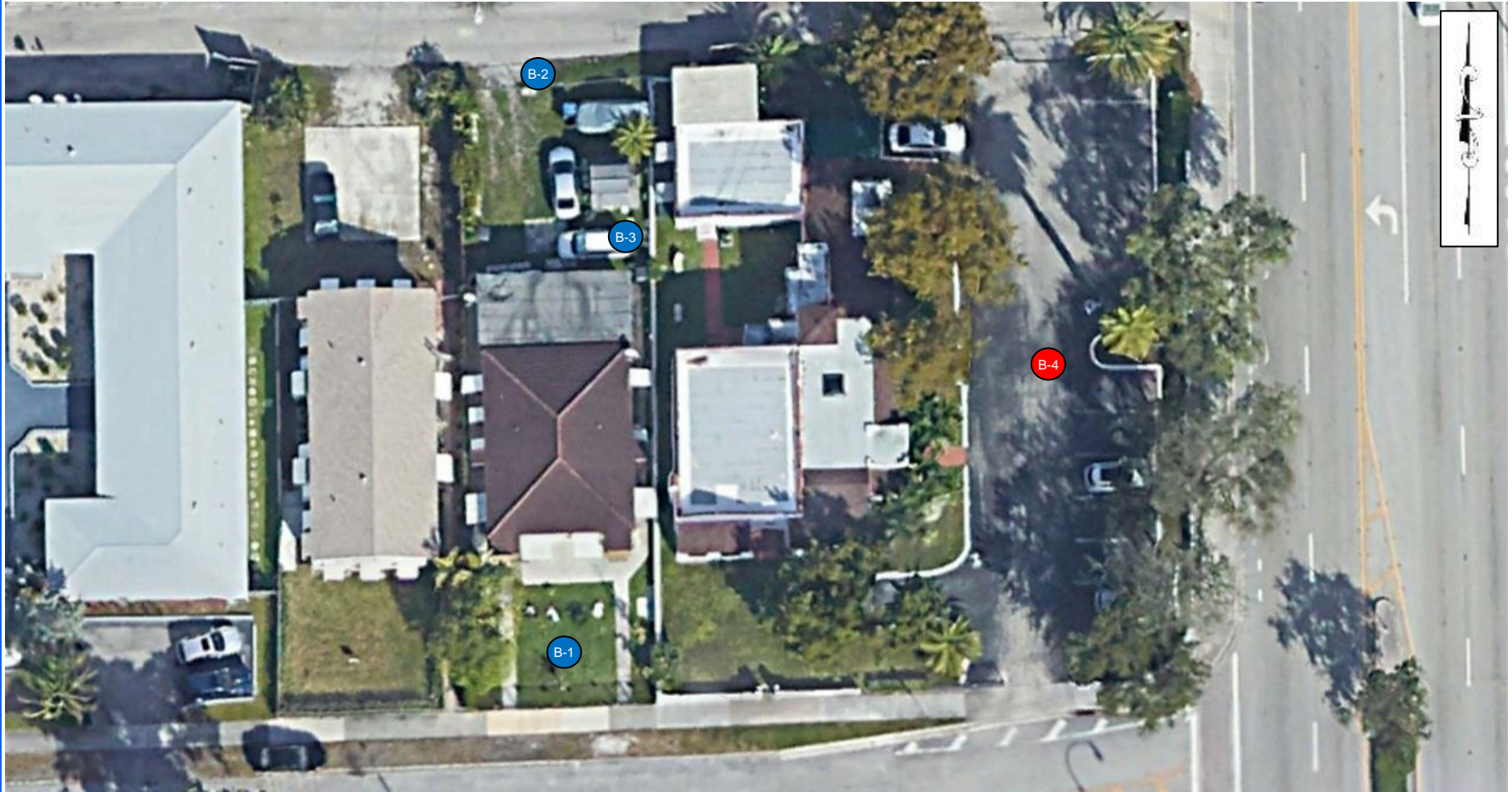
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SHEET NO.: 1

PACIFICA PROJ. NO: 320-23214

BORING LOCATION PLAN - AERIAL



B-#

Approximate SPT Boring Location

B-#

Approximate SPT Boring / Percolation Test Location

GEOTECHNICAL ENGINEERING SERVICES
14-Story Building
950 S Federal Highway, Hollywood, Florida

DATE: 5/08/2023

DRAWN: MR

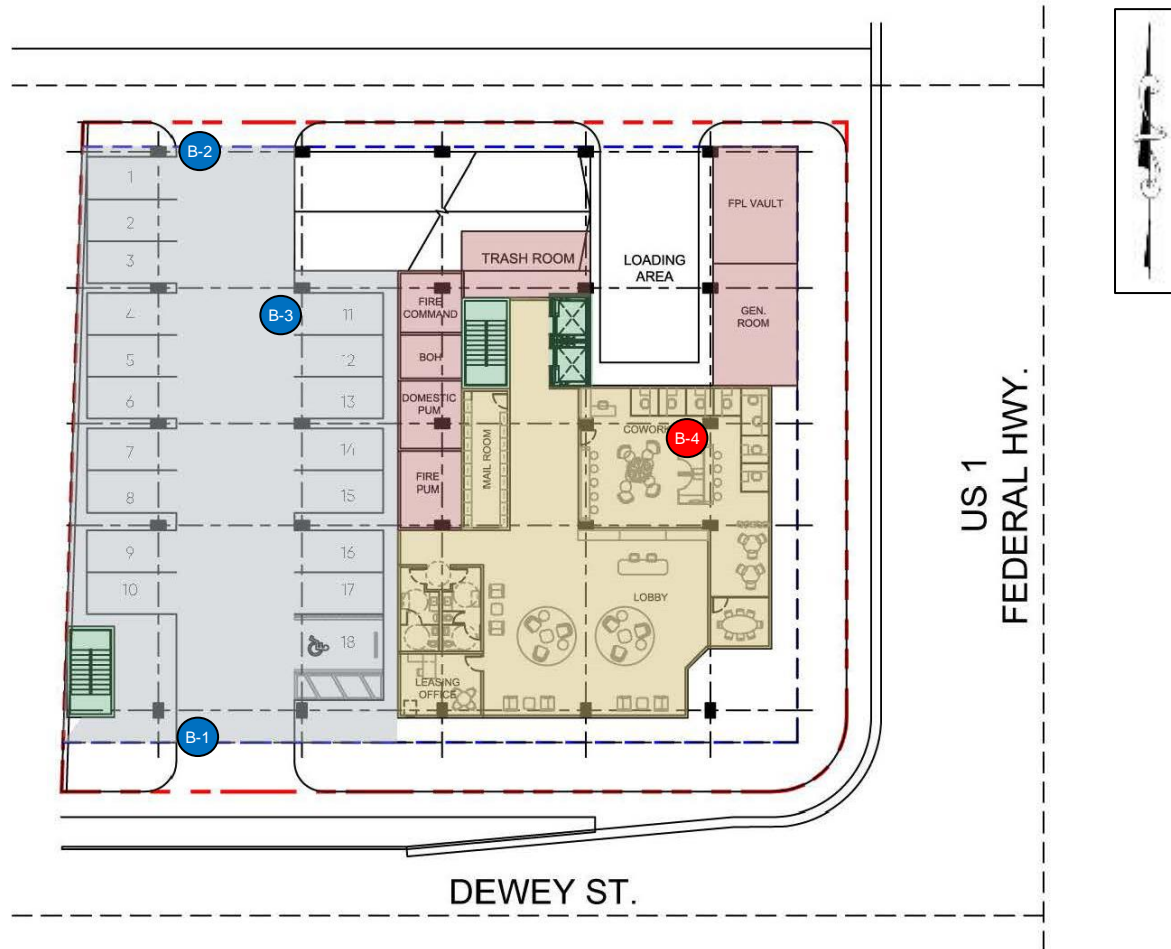
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PACIFICA PROJ. NO: 320-23214

CHKD: MP



BORING LOCATION PLAN - SITE



Approximate SPT Boring Location



Approximate SPT Boring / Percolation Test Location

GEOTECHNICAL ENGINEERING SERVICES

14-Story Building

950 S Federal Highway, Hollywood, Florida

DATE: 5/08/2023

DRAWN: MR

SHEET NO.: 3

PACIFICA PROJ. NO: 320-23214

CHKD: MP










































Test Boring Log

Client **Conkreta**
 Project **14-Story Building**
 Boring Location **See Boring Location Plan**
 Elev. Ref. **N/A**
 Remarks **The stratification lines represent approximate boundaries.
The transition may be gradual.**

Boring No. **B-1**
 Date Started **5/18/2023**
 Date Completed **5/18/2023**
 Project No. **320-23214**
 Sheet No. **1 of 2**
 Ground Water Depth **7.0 feet**

ELEV. (ft)	Depth (ft)	Graphic Log	DESCRIPTION OF MATERIALS	SAMPLE							
				No.	Type	USCS Classification	Blows	REC-OVERY	REC %	SPT N. Value	
	0.0										
	2.0		Gray Fine SAND	1		SP	3-2-1-1			3	
	4.0		Light Brown to Light Gray Fine SAND	2		SP	4-2-4-2			6	
	6.0			3			3-4-2-1			6	
	8.0		Light Brown Sandy LIMESTONE	4			7-9-5-7			14	
	10.0			5			8-11-6-9			17	
	12.0										
	14.0			6			6-9-7-5			16	
	16.0										
	18.0										
	20.0		7		10-13-11-9	24					
	22.0		Light Gray Fine SAND with Few Limestone			SP					
	24.0			8			7-9-5-4			14	
	26.0										
	28.0										
	30.0			9			9-11-8-6			19	
	32.0										
	34.0		Light Gray Fine SAND	10		SP	9-6-5-3			11	
	36.0										
	38.0										
40.0		11			7-5-3-5		8				
42.0											
44.0		12			9-6-8-7		14				
46.0											
48.0		Light Gray Sandy LIMESTONE									
50.0			13			21-17-14-10	31				

General Notes

Driller: **L.S.**
 Hammer Type: **Automatic**
 Rig Type: **CME55**
 Drilling Method: **SPT**



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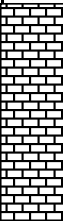



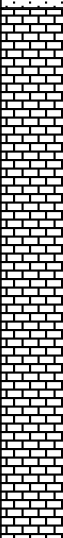




SPT Split Spoon Sampler
 Groundwater at Time of Drilling
 WOH= Weight of Hammer



Test Boring Log

Client **Conkreta**
Project **14-Story Building**
Boring Location **See Boring Location Plan**
Elev. Ref. **N/A**
Remarks **The stratification lines represent approximate boundaries.
The transition may be gradual.**

Boring No. **B-1**
Date Started **5/18/2023**
Date Completed **5/18/2023**
Project No. **320-23214**
Sheet No. **2 of 2**
Ground Water Depth **7.0 feet**

ELEV. (ft)	Depth (ft)	Graphic Log	DESCRIPTION OF MATERIALS	SAMPLE							
				No.	Type	USCS Classification	Blows	REC- OVERY	REC %	SPT N. Value	
	52.0		Light Gray Sandy LIMESTONE								
	54.0			14			29-50/1"				50/1"
	56.0										
	58.0										
	60.0		Light Gray Fine SAND with Trace Limestone	15		SP	10-12-14-11		26		
	62.0		Light Gray Sandy LIMESTONE								
	64.0			16		17-10-15-12	25				
	66.0										
	68.0										
	70.0			17		21-19-13-10	32				
	72.0										
	74.0			18		17-23-21-18	44				
	76.0										
	78.0										
	80.0			19		15-11-12-10	23				
				Boring Terminated at 80.0 feet							

General Notes

Driller: **L.S.**
Hammer Type: **Automatic**
Rig Type: **CME55**
Drilling Method: **SPT**



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(561) 419-8460

SPT Split Spoon Sampler
 Groundwater at Time of Drilling
WOH= Weight of Hammer



Test Boring Log

Client **Conkreta**
 Project **14-Story Building**
 Boring Location **See Boring Location Plan**
 Elev. Ref. **N/A**
 Remarks **The stratification lines represent approximate boundaries.
The transition may be gradual.**

Boring No. **B-2**
 Date Started **5/15/2023**
 Date Completed **5/15/2023**
 Project No. **320-23214**
 Sheet No. **1 of 2**
 Ground Water Depth **7.0 feet**

ELEV. (ft)	Depth (ft)	Graphic Log	DESCRIPTION OF MATERIALS	SAMPLE							
				No.	Type	USCS Classification	Blows	REC-OVERY	REC %	SPT N. Value	
<div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div></div>	0.0										
	2.0		Dark Gray Fine SAND with Trace Roots	1		SP	2-1-3-2			4	
	4.0		Dark Gray to Brown Fine SAND	2		SP	4-2-4-3			6	
	6.0			3			2-1-3-2			4	
	8.0			4			4-5-3-4			8	
	10.0		Light Brown to Light Gray Sandy LIMESTONE	5			6-9-7-6			16	
	12.0										
	14.0			6			10-12-13-10			25	
	16.0										
	18.0										
	20.0			7			13-17-10-9			27	
	22.0										
	24.0			8			8-6-4-7			10	
	26.0										
	28.0										
	30.0			9			4-3-2-4			5	
	32.0										
	34.0			10			2-4-6-3			10	
	36.0										
	38.0										
	40.0			11			9-10-12-8			22	
	42.0										
	44.0			12			11-7-6-4			13	
	46.0										
48.0											
50.0		13				9-4-8-6			12		

General Notes

Driller: **L.S.**
 Hammer Type: **Automatic**
 Rig Type: **CME55**
 Drilling Method: **SPT**



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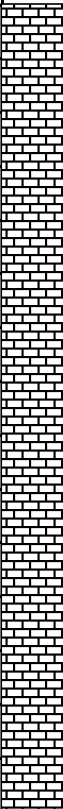






SPT Split Spoon Sampler
 Groundwater at Time of Drilling
 WOH= Weight of Hammer



Test Boring Log

Client **Conkreta**
 Project **14-Story Building**
 Boring Location **See Boring Location Plan**
 Elev. Ref. **N/A**
 Remarks **The stratification lines represent approximate boundaries.
The transition may be gradual.**

Boring No. **B-2**
 Date Started **5/15/2023**
 Date Completed **5/15/2023**
 Project No. **320-23214**
 Sheet No. **2 of 2**
 Ground Water Depth **7.0 feet**

ELEV. (ft)	Depth (ft)	Graphic Log	DESCRIPTION OF MATERIALS	SAMPLE													
				No.	Type	USCS Classification	Blows	REC-OVERY	REC %	SPT N. Value							
	52.0		Light Brown to Light Gray Sandy LIMESTONE														
	54.0			14			7-14-19-10			33							
	56.0																
	58.0																
	60.0			15			8-10-13-11			23							
	62.0																
	64.0			16			9-7-10-17			17							
	66.0																
	68.0																
	70.0			17			13-19-24-27			43							
	72.0																
	74.0			18			28-21-23-19			44							
	76.0																
	78.0																
	80.0			19			17-15-22-16			37							
							Boring Terminated at 80.0 feet										

General Notes

Driller: **L.S.**
 Hammer Type: **Automatic**
 Rig Type: **CME55**
 Drilling Method: **SPT**



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SPT Split Spoon Sampler
 Groundwater at Time of Drilling
 WOH= Weight of Hammer



Test Boring Log

Client **Conkreta**
 Project **14-Story Building**
 Boring Location **See Boring Location Plan**
 Elev. Ref. **N/A**
 Remarks **The stratification lines represent approximate boundaries.
The transition may be gradual.**

Boring No. **B-3**
 Date Started **5/13/2023**
 Date Completed **5/13/2023**
 Project No. **320-23214**
 Sheet No. **1 of 2**
 Ground Water Depth **7.0 feet**

ELEV. (ft)	Depth (ft)	Graphic Log	DESCRIPTION OF MATERIALS	SAMPLE							
				No.	Type	USCS Classification	Blows	REC-OVERY	REC %	SPT N. Value	
<div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div></div>	0.0										
	2.0		Gray Fine SAND	1		SP	2-1-2-3			3	
	4.0			2			3-4-3-2			7	
	6.0		Brown to Gray Fine SAND	3		SP	4-5-2-4			7	
	8.0			4			6-8-10-9			18	
	10.0			5			9-7-12-10			19	
	12.0										
	14.0			6			2-3-2-1			5	
	16.0										
	18.0										
	20.0			7			4-2-4-3			6	
	22.0										
	24.0			8			2-1-2-3			3	
	26.0										
	28.0										
	30.0			9			4-6-3-4			9	
	32.0										
	34.0			10			17-15-10-13			25	
	36.0										
	38.0										
	40.0		Light Gray Fine SAND with Few Limestone	11		SP	13-17-22-24			39	
42.0											
44.0	12			10-11-9-7	20						
46.0											
48.0		Light Gray Sandy LIMESTONE									
50.0	13			7-4-8-5		12					

General Notes

Driller: **L.S.**
 Hammer Type: **Automatic**
 Rig Type: **CME55**
 Drilling Method: **SPT**



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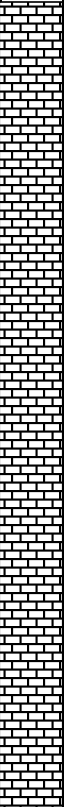






SPT Split Spoon Sampler
 Groundwater at Time of Drilling
 WOH= Weight of Hammer



Test Boring Log

Client **Conkreta**
Project **14-Story Building**
Boring Location **See Boring Location Plan**
Elev. Ref. **N/A**
Remarks **The stratification lines represent approximate boundaries.
The transition may be gradual.**

Boring No. **B-3**
Date Started **5/15/2023**
Date Completed **5/15/2023**
Project No. **320-23214**
Sheet No. **2 of 2**
Ground Water Depth **7.0 feet**

ELEV. (ft)	Depth (ft)	Graphic Log	DESCRIPTION OF MATERIALS	SAMPLE											
				No.	Type	USCS Classification	Blows	REC-OVERY	REC %	SPT N. Value					
	52.0		Light Gray Sandy LIMESTONE												
	54.0			14			10-17-19-11			36					
	56.0														
	58.0														
	60.0			15			12-14-10-19			24					
	62.0														
	64.0			16			20-17-12-13			29					
	66.0														
	68.0														
	70.0			17			19-24-28-20			52					
	72.0														
	74.0			18			11-17-15-10			32					
	76.0														
	78.0														
	80.0			19			21-24-19-14			43					
							Boring Terminated at 80.0 feet								

General Notes

Driller: **L.S.**
Hammer Type: **Automatic**
Rig Type: **CME55**
Drilling Method: **SPT**



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SPT Split Spoon Sampler
 Groundwater at Time of Drilling
WOH= Weight of Hammer



Test Boring Log

Client **Conkreta**
Project **14-Story Building**
Boring Location **See Boring Location Plan**
Elev. Ref. **N/A**
Remarks **The stratification lines represent approximate boundaries.
The transition may be gradual.**

Boring No. **B-4**
Date Started **5/12/2023**
Date Completed **5/12/2023**
Project No. **320-23214**
Sheet No. **1 of 2**
Ground Water Depth **6.0 feet**



ELEV. (ft)	Depth (ft)	Graphic Log	DESCRIPTION OF MATERIALS	SAMPLE							
				No.	Type	USCS Classification	Blows	REC-OVERY	REC %	SPT N. Value	
	0.0										
	2.0		0.5" Asphalt / Gray Fine SAND with Little Limerock Fill	1		SP	26-8-10-7			18	
	4.0		Light Gray to Gray Fine SAND	2		SP	7-5-3-4			8	
	6.0			3			4-3-4-0		7		
	8.0			4			9-7-10-2		17		
	10.0		Light Brown Sandy LIMESTONE	5			6-8-5-3			13	
	12.0										
	14.0			6		7-6-4-7		10			
	16.0										
	18.0										
	20.0			7		6-3-5-2		8			
	22.0										
	24.0			8		5-4-2-4		6			
26.0											
28.0											
30.0		9			2-3-2-5		5				
32.0											
34.0			Light Gray Fine SAND with Few Limestone	10		SP	3-5-7-4		12		
36.0											
38.0											
40.0		11			4-6-8-10			14			
42.0											
44.0			Gray Sandy LIMESTONE	12			7-10-6-5		16		
46.0											
48.0											
50.0		13			10-7-9-6			16			

General Notes

Driller: **L.S.**
Hammer Type: **Automatic**
Rig Type: **CME55**
Drilling Method: **SPT**



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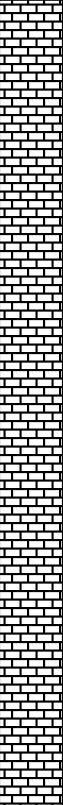






 SPT Split Spoon Sampler
 Groundwater at Time of Drilling
WOH= Weight of Hammer



Test Boring Log

Client **Conkreta**
Project **14-Story Building**
Boring Location **See Boring Location Plan**
Elev. Ref. **N/A**
Remarks **The stratification lines represent approximate boundaries.
The transition may be gradual.**

Boring No. **B-4**
Date Started **5/15/2023**
Date Completed **5/15/2023**
Project No. **320-23214**
Sheet No. **2 of 2**
Ground Water Depth **6.0 feet**

ELEV. (ft)	Depth (ft)	Graphic Log	DESCRIPTION OF MATERIALS	SAMPLE											
				No.	Type	USCS Classification	Blows	REC-OVERY	REC %	SPT N. Value					
	52.0		Gray Sandy LIMESTONE												
	54.0			14			6-8-5-7			13					
	56.0														
	58.0														
	60.0			15			11-16-19-24			35					
	62.0														
	64.0			16			19-21-14-11			35					
	66.0														
	68.0														
	70.0			17			38-43-50/3"			50/3"					
	72.0														
	74.0			18			10-18-13-10			31					
	76.0														
	78.0														
	80.0			19			9-17-12-18			29					
							Boring Terminated at 80.0 feet								

General Notes

Driller: **L.S.**
Hammer Type: **Automatic**
Rig Type: **CME55**
Drilling Method: **SPT**



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SPT Split Spoon Sampler
 Groundwater at Time of Drilling
WOH= Weight of Hammer



Percolation Test

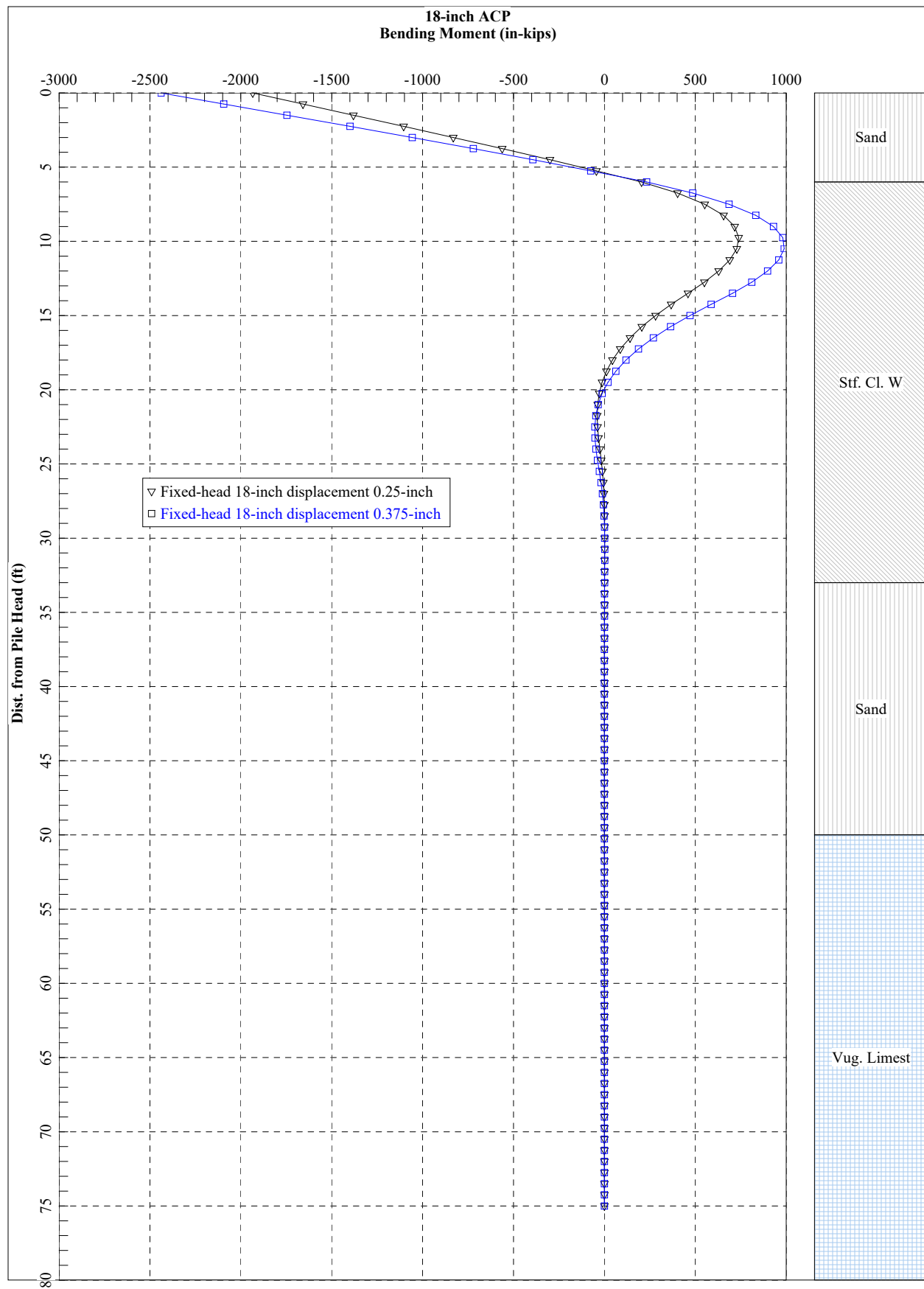
Client	Conkreta	Boring No.	P-4
Project	14-Story Building	Date Started	5/16/2023
Boring Location	See Boring Location Plan	Date Completed	5/16/2023
Elev. Ref.	N/A	PACIFICA Proj. No.	320-23214
Remarks			

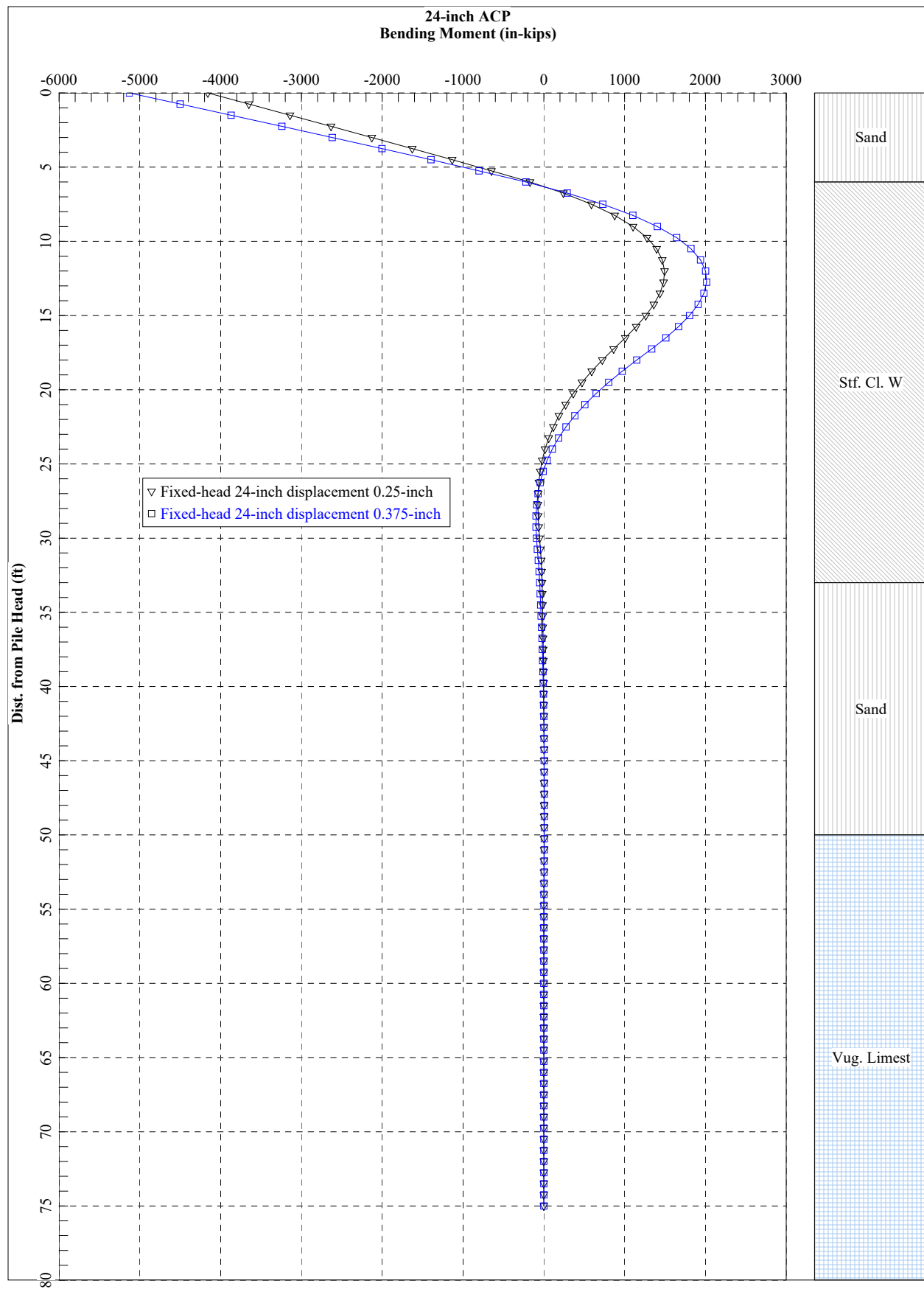
Subsurface Profile	
Depth (ft)	Soil Description
0-2	0.5" Asphalt / Gray Fine SAND with Little Limerock Fill
2-6	Light Gray to Gray Fine SAND
6-10	Light Brown Sandy LIMESTONE

Percolation Results								
Diameter		Depth of Hole (ft)	Depth of Groundwater Level Below Ground Surface (ft)		Hydraulic Head (ft)	Saturated Hole Depth (ft)	Average Flow Rate (gpm)	K, Hydraulic Conductivity cfs/ft ² -ft
Casing (in)	Perforated PVC (in)							
			Prior to Test	During Test				
3	2	10	6	0	6	4	4.0	4.0E-04

Note:

- (1) The above hydraulic conductivity values are for a french drain installed to the same depth as the borehole tests. The values represent an ultimate value. The designer should apply the appropriate factor of safety.
- (2) The hydraulic conductivity values were calculated based on the South Florida Water Management District's USUAL OPEN HOLE CONSTANT HEAD percolation test procedure as shown on the "Equations in SFWMD Permit Information Manual, Volume IV".
- (3) A diameter of two inches was used in the computation of the Hydraulic Conductivity value presented in the above table.





Ahmad Aljbour

From: Ahmad Aljbour
Sent: Wednesday, February 14, 2024 10:26 AM
To: 'Admin@TraceConsulting.us'; 'FPANELLAS@TRACECI.COM'
Cc: Chris Collins; Anabel Torres
Subject: C010 The George - FDOT 441582-1-52-01
Attachments: Civil Plans.pdf

Follow Up Flag: Follow up
Flag Status: Flagged

ShareSyncMessageS{"Links":[],"LocalTempFolderPath":null,"Password":null,"ServerFolderName":null}

Hello Frank,

We have been retained as the civil engineers for a new 14-story residential building located at 950 S. Federal Highway, Hollywood, Florida. Our project falls within your FDOT project (**State Road (SR) 5/US-1/Federal Highway From SR 824/ Pembroke Road to Johnson Street and From Johnson Street to SR 822/Sheridan Street 441582-1-52-01**) but we are not proposing any work within the FDOT right of way. The utilities will be provided to the new project via the alley abutting the northern property line.

The City of Hollywood issued the following comment during the site plan review process:

“US1 adjacent to this proposed development’s site will be undergoing a complete street reconstruction. Proposed site design and all US1 rights-of-way improvements under this project shall coordinate and be compatible with the proposed improvements and vision of the corridor. Please coordinate with FDOT project consultant, **Trace Consultants, Inc., Frank Panellas, PE**, with notification to City CRA and Engineering, Transportation and Mobility Division staff.”

Please see attached civil plans and inform us if any of our proposed work conflicts with the mentioned FDOT project.

Sincerely,

Ahmad Aljbour, E.I.
URBN Design
666 NE 125th Street, Suite 247
North Miami, FL 33161
305.720.2079 Ext. 152
<http://urbndesigngroup.com>

URBN Project#: _____



RunBrook – Green Building & Energy Testing
313 Datura Street
Suite 200
West Palm Beach, FL 33401

1 April 2024

Russell Long
City of Hollywood
PO Box 229045
Hollywood, FL 33022

RE: 950 S Federal Hwy, Hollywood, FL

Dear Mr. Long:

Our office has been contracted to serve as Sustainability Consultant on the above-mentioned project. In accordance with the City of Hollywood's green building ordinance (O-2011-06), the project will be pursuing certification under Florida Green Building Coalition's (FGBC) High-Rise rating system. Our office believes the project is well suited for certification. Below please find a list of ten (10) green building practices that the project will implement.

1. Stormwater Pollution Prevention to FGBC standards.
2. Erosion & Sediment Control measures to FGBC standards.
3. Selection of HVAC equipment with no CFC refrigerants.
4. High performance building envelope.
5. High performance HVAC equipment.
6. High efficacy lighting.
7. Low-flush and low-flow plumbing fixtures.
8. Low-VOC paints, caulking, and sealants.
9. High performance filtration media.
10. Considerations for occupant's thermal comfort via compliance with ASHRAE 55.

Sincerely,

Daniel J. Denis, President
LEED AP (BD+C) & Green Rater
FGBC Certifying Agent & Board of Directors Member
NGBS Verifier
EarthCraft Technical Advisor
RESNET HERS Rater