

FIGURE 2 – PRELIMINARY SITE PLAN



4. Evaluation and recommendations with respect to foundation support of the structure, including allowable soil bearing pressures, recommended bearing elevations, and other foundation design considerations.
5. Provide soil criteria and site preparation procedures for construction of the proposed structure.

2.0 OBSERVATIONS

2.1 Site Observation and Historical Data

The subject property is located at 3090 Sheridan Street in Hollywood, Florida. The site is occupied by multiple commercial buildings and associated paved parking lots that are to be demolished for the proposed construction. No soil staining or visual evidence of chemical or petroleum spillage was apparent. The recovered samples were not examined, either visually or analytically, for chemical composition or environmental hazards. UES would be pleased to perform these services if required.

UES reviewed historic aerials from 1952 through 2022. The aerials display the site containing heavy vegetations through 1995. The commercial buildings and associated paved parking were constructed by 1999. The site has remained relatively unchanged.

2.2 Laboratory Testing and Procedures

Soil samples recovered from UES's field exploration were returned to the laboratory. A geotechnical engineer visually examined and reviewed the field descriptions of the recovered soils in general accordance with ASTM D-2488. Samples were visually examined to accurately evaluate the subsurface soil properties and site geomorphic conditions. Based on UES's analysis of the recovered samples, additional testing was deemed unnecessary.

Representative samples of the soils encountered during the field exploration will be held in the laboratory for your inspection for 30 days unless UES is notified otherwise.

2.3 Geomorphic Conditions

The geology of the immediate vicinity, based on the USDA Soil Survey, is mainly representative of the ***Urban land complex, 0 to 2 percent slopes and Matlacha, limestone substratum-Urban land complex***, which consists mostly of gravel and sandy soils.



The South Florida region is a low probable area of sinkhole development or intense seismic activity. There are no known fault lines located on or near the project site. Based on the Seismicity Map of the State of Florida produced by B.G. Reagor 1987, the closest seismic activity occurred near Miami in 1945. This event registered an III on the Modified Mercalli Intensity Scale of 1931. This intensity is similar to vibrations like that due to passing of heavy or heavily loaded trucks.

2.4 Field Exploration

For purposes of this study, the following was performed:

- Eight (8) standard penetration test (SPT) boring (ASTM D-1586) to a depth of 20 feet below existing grade.
- Two (2) exfiltration tests to a depth of 10 feet below existing grade in accordance with the South Florida Water Management District method for open-hole constant head field testing.

Figure 3 shows the approximate test locations performed at the site. The SPT boring method was used as the investigative tool within the borings. Penetration tests were performed in substantial accordance with ASTM Procedure D-1586, "Penetration Test and Split-Barrel Sampling of Soils". This test procedure consists of driving a 1.4-inch I.D. split-tube sampler into the soil profile using a 140-pound automatic hammer falling 30 inches. The number of blows per foot, for the second and third 6-inch increments, is an indication of soil strength. The SPT borings were performed using a CME-75, a truck-mounted drill rig, equipped with an automatic hammer. The soil samples recovered from the soil borings were classified and stratified by a geotechnical engineer. Following completion of the field services, all boreholes were backfilled with excavated soil/rock, and the site generally cleaned, as required.

The results of the classification and stratification are encountered during UES's exploration are presented in **Appendix A** "Record of Test Boring". It should be noted that soil conditions might vary between what is depicted on the attached log and other areas of the site. The soil boring data reflect information from a specific test location only. Site specific survey staking for the test location was not provided for UES's field exploration. The boring location was determined in the field by a project engineer by measuring distances and estimating right angles from existing site features. The latitude, longitude, and elevation noted in UES's boring logs were taken from Google Earth.

Google Earth uses WGS-84 or Local Mean Sea Level (MSL) as datum. It should be noted that elevations may not always be correct if fill is added or site grades change to a site after Google captures the image. The boring location and elevations noted should, therefore, be considered approximate. The boring depths were confined to the zone of soil likely to be stressed by the proposed construction. The boring log depicts the observed soils in graphic detail. The Standard Penetration Test boring indicates the penetration resistance, or N-values logged during the drilling and sampling activities.



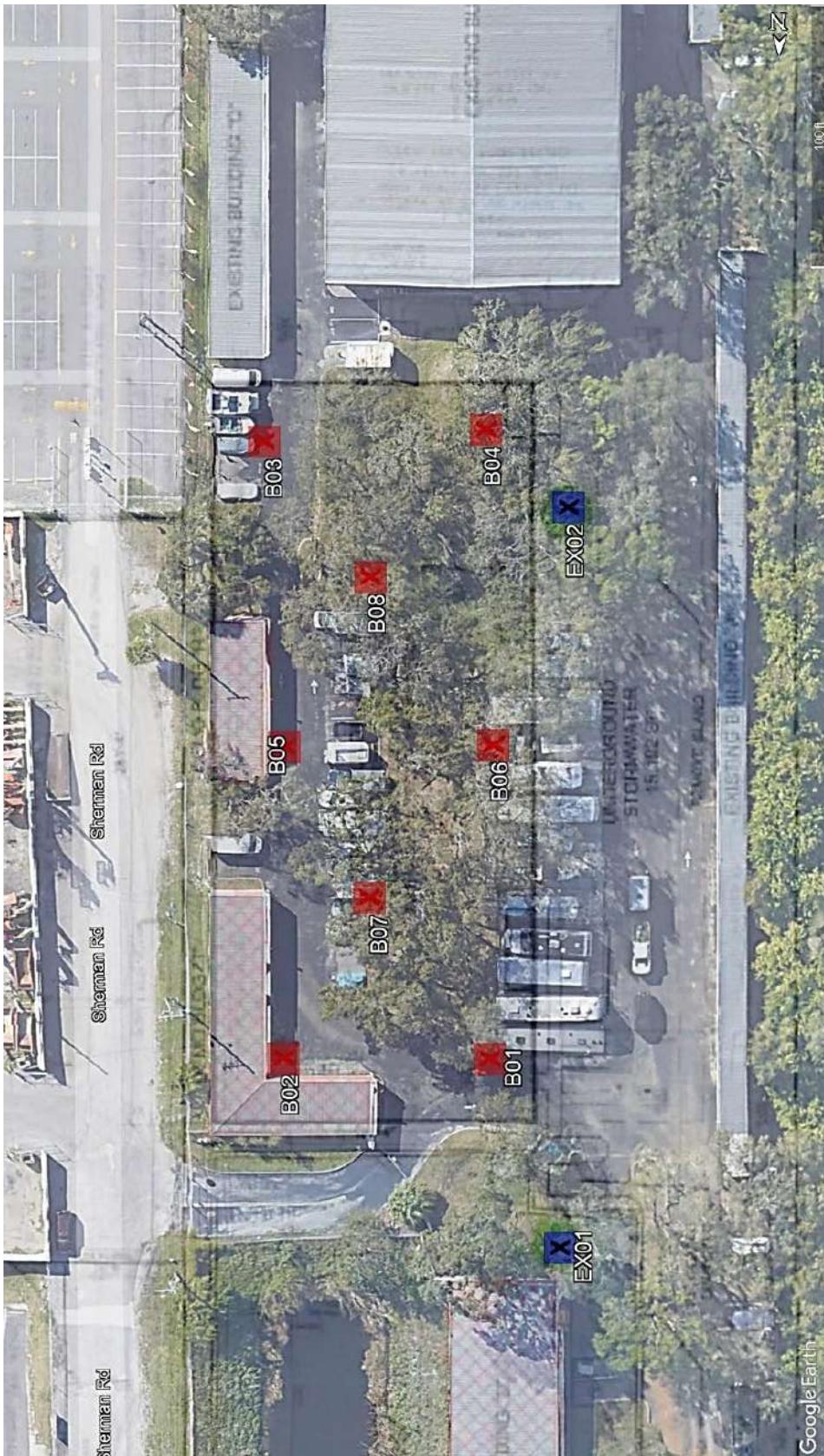


FIGURE 3 – TEST LOCATION PLAN



NOTE: BORING LOCATIONS WERE LOCATED USING A MEASURING TAPE AND EXISTING LANDMARKS AS REFERENCE POINTS. IN ADDITION, THE LATITUDE, LONGITUDE, AND ELEVATION NOTED ON THE BORING LOGS WERE TAKEN FROM GOOGLE EARTH. THEREFORE, LOCATIONS SHOWN ON THE PLAN ARE APPROXIMATE.



Please refer to **Appendix B** "Notes Related to the Test Borings" for further clarification of UES's field exploration. The classifications and descriptions shown on the log are generally based upon visual characterizations of the recovered soil samples. All soil samples reviewed have been depicted and classified in accordance with the Unified Soil Classification System symbols (i.e. SP, SP-SM, SC, etc.). See in **Appendix C** "Discussion of Soil Groups", for a detailed description of various soil groups.

3.0 SUBSURFACE CONDITIONS

The soils at the explored locations generally consisted of 1.5" to 2" of asphalt, with the exception of the location of B06 where a 5" layer of concrete was encountered, atop loose to very dense, fine to medium grained sand with variable amounts of limestone fragments from the ground surface to approximate depths of 6 to 8 feet below ground surface (BGS). The following layer consisted of weathered limestone with some sand to approximately 13 to 18 feet BGS. The borings then showed intermittent layers of medium dense to dense sand and sand with varying amounts of limestone fragments to the termination depth of the borings at 20 feet below ground surface (BGS).

For a more precise description of the conditions encountered within the soil test borings, refer to the "Record of Test Boring" logs included in **Appendix A**.

3.1 Hydrogeological Conditions

On the date of UES's field exploration (April 2022), groundwater was encountered at the approximate depth of 5'3" to 7' BGS during drilling operations, depending on the location. The groundwater table will fluctuate seasonally depending upon local rainfall, and other local influences. Higher temporary water levels may be possible at this site after extended periods of rain. This is based upon the existing static groundwater levels at the time of the exploration and anticipating groundwater table rise through the type of soils encountered during the exploration.

As part of UES's field exploration, UES performed two (2) exfiltration tests to a depth of 10 feet below existing grade in accordance with the South Florida Water Management District method for open-hole constant head field testing. The tests were conducted at the areas presented in **Figure 3**. The calculated hydraulic conductivity coefficients for the exfiltration tests are presented in **Appendix D**.

Exfiltration Test	Hydraulic Conductivity Result
EX-1	$3.13 \times 10^{-4} \text{ ft}^3/\text{sec}/\text{ft}^2\text{-ft}$
EX-2	$3.56 \times 10^{-4} \text{ ft}^3/\text{sec}/\text{ft}^2\text{-ft}$



No additional investigation was conducted in relation to any existing well field in the vicinity. Well fields may influence water table levels and cause significant fluctuations. If a more comprehensive water table analysis is necessary UES recommends contacting a registered professional specialized in hydrogeology.

4.0 FOUNDATION RECOMMENDATIONS

4.1 General

A foundation system for any structure must be designed to resist bearing capacity failures, have settlements within tolerable limits for the structure type, and resist environmental forces, which the foundation may be subjected to over the life of the structure. Environmental forces in Florida can include sinkholes, shrinking and swelling soils, and soil consolidation, among others. It is UES's opinion that these specific environmental forces have a low risk (on a scale of low, moderate, high) of detrimentally affecting shallow foundation performance at this site.

The soil bearing capacity is the soil's ability to support loads without punching into the soil profile. Bearing capacity failures are analogous to shear failures in structural design and are usually sudden and catastrophic. Based on the scope of the proposed project, it is UES's opinion that the soils at the site are generally suitable for shallow foundations to support the proposed structures once ground improvement techniques discussed herein have been completed.

The allowable amount of settlement that a structure may tolerate is dependent on several factors including uniformity of settlement, time rate of settlement, structural dimensions and properties of the structural materials. Generally, total, or uniform settlement does not damage a structure but may affect drainage and utility connections. These can generally tolerate movements of several inches for building construction. In contrast, differential settlement affects a structure's frame and is limited by the structural flexibility. Shallow foundations appear to be suitable for the proposed project and can be used to support the proposed structures at this site, provided that the existing soils and any fill soils are properly prepared as discussed herein. Shallow foundations can consist of conventional shallow continuous strip footings and isolated spread footings, or a monolithic turned down footing and slab type foundation.

4.2 Site Preparation Recommendations

UES's recommendations for site preparation assuming continuous strip footings and/or isolated footings or a monolithic foundation will be utilized to support the planned structure are noted below. This approach to improving and maintaining the site soils has been found to be successful on projects with similar soil conditions.



1. Initial site preparation for the proposed structure should consist of clearing the vegetation and stripping and removal of the topsoil to expose clean granular soils. Disposal of these materials shall be accomplished in accordance with local and municipal guidelines.
2. No information has been provided about previous or existing foundations at the site. Where existing foundations are encountered, they should be completely removed and replaced with compacted fill if they interfere with new foundations. Disposal of these materials shall be accomplished in accordance with local and municipal guidelines.
3. Existing underground drainage/utility pipes (if any) to be relocated shall be abandoned by either complete removal and subsequent soil backfilling in accordance with the recommendation provided herein or left in-place and filled with "flowable fill".
4. Following the site stripping, areas of surficial sand should be compacted prior to the placement of any fill. UES recommends a steel drum vibratory roller with a minimum static weight of 20,000 lbs. and minimum vibratory impact energy of 50,000 lbs. The roller should be operated at 2 mph making at least 10 perpendicular overlapping passes. Densification of the soils should be performed within the proposed development areas plus a 5-foot-wide perimeter extending beyond the outside edge of the same, where practical. Densification operations should continue until the subgrade soils are firm and unyielding. Any area of the exposed surface that deflects excessively under the weight of the compaction equipment should be excavated approximately 24 inches and be replaced with compacted structural fill. No section of the subgrade should receive less than 4 passes of the roller or until at least 98% maximum density (ASTM D 1557) is achieved for a depth of at least 1 foot below the excavated surface. Upon completion of the proof rolling, backfill shall be placed in maximum 12-inch loose lifts and compacted to a minimum density of **98 percent** of the Modified Proctor maximum dry density (ASTM D-1557). Fill to be compacted with a vibratory plate tamper or a small walk behind vibratory roller should be placed in lifts not exceeding six inches in loose thickness.
5. The bottom of footing excavations shall be compacted with a steel drum vibratory roller with a minimum static weight of 20,000 lbs. and minimum vibratory impact energy of 36,000 lbs.; (Dynapac CA-250D or equivalent) or equivalent equipment. The roller should be operated at 2 mph. Additional passes may be necessary if acceptable compaction is not achieved. Density tests shall be completed at footing subgrade to confirm that the soils have achieved a minimum degree of compaction of 95 percent of modified proctor maximum density (ASTM D-1557). The bottom of all footings shall be examined by the engineer or his representative to determine if the soil is free of all organic and/or deleterious materials, and if the required compaction and soil pressures are achieved or if additional compaction is required.



6. Fill material placement (if needed) should be inorganic (classified as SP/SW) containing not more than 5 percent (by weight) fibrous organic materials. Fill materials with silt-size soil fines in excess of 10% should not be used, this includes cyclone sand material. Place fill in maximum 12-inch lifts and compact each lift to a minimum density of 98 percent of the Modified Proctor maximum dry density (ASTM D-1557) with a vibratory roller. Perform compliance tests within the fill at a frequency of not less than one test per 2,500 square feet per lift in the building areas, or a minimum of 2 test locations per lift, whichever is greater.
7. It is likely that proof-rolling and any subsequent backfill compaction with the aforementioned equipment may induce ground vibrations that can affect the existing nearby structures. A representative from UES's office can monitor the vibration disturbance using seismograph equipment capable of recording ground velocities that can be used to determine if construction activity at the site is exceeding tolerable vibration levels on adjacent structures as established by the project structural engineer.
8. Depending on the depth of the footings, groundwater control may be required at this site for either excavation dewatering or removal of temporarily perched water from a rain event. Such water can be controlled by pumping from sumps located in ditches or pits. Groundwater should be maintained at least one foot below the bottom of any excavation made during construction operations, or at least two feet below the surface of any compaction operations.
9. The contractor shall take into account the final contours and grades as established by the plan when executing his backfilling and compaction operations.

Using vibratory compaction equipment can disturb adjacent structures. If vibration related disturbance to nearby structures may be of concern, vibration levels should be monitored during compaction operations. A representative from this office can monitor the vibration disturbance of adjacent structures; and a proposal for these services can be provided upon your request.

4.3 Design of Footings

Following proper site preparation as recommended in this report, it is UES's opinion that the proposed structures can be supported on conventional shallow foundations bearing on existing soils or newly placed engineered fills. A net allowable soil bearing pressure of **3,500 pounds per square foot (psf)** may be used for the design of isolated spread footings and continuous strip footings. Provided that the site is prepared in accordance as discussed in this report, foundation settlement is expected to be less than 1 inch total across the structure and 1/2-inch differential between adjacent columns or a horizontal distance of 20 feet.



Shallow foundations should be embedded a minimum of 18 inches into the bearing soils. The embedment shall be measured from the lowest adjacent exterior grade. Isolated column footings should be at least 30 inches in width and continuous strip footings should have a width of at least 18 inches, regardless of contact pressure. All footings and columns should be structurally separated from the floor slab so that minor differential foundation settlement can occur without causing damage to the slab-on-grade floor unless a monolithic slab-on-grade foundation is planned.

4.4 Floor Slabs

The ground floors slab may be supported directly on a granular fill pad following site preparation and foundation construction outlined in this report. For purposes of design, a modulus of subgrade reaction of 150 pounds per cubic inch may be used.

It should be noted that excessive moisture vapor transmission through concrete floor slabs can result in damage to floor coverings, as well as other deleterious effects to slab supported equipment or stored items. An appropriate moisture vapor retarder should be placed beneath the floor slab to reduce moisture vapor from entering the building through the slab. The retarder should be installed in general accordance with applicable ASTM procedures and manufacturer's installation instructions including sealing the membrane at lapped joints, around pipe penetrations and at the edges of foundations.

5.0 EXCAVATION CONDITIONS

In Federal Register, Volume 54, No. 209 (October 1989), the United States Department of Labor, Occupational Safety and Health Administration (OSHA) amended its "Construction Standards for Excavations, 29 CFR, part 1926, Subpart P". This document was issued to better insure the safety of workmen entering trenches or excavations. It is mandated by this federal regulation that all excavations, whether they be utility trenches, basement excavations or footing excavations, be constructed in accordance with the OSHA guidelines. It is UES's understanding that these regulations are being strictly enforced and if they are not closely followed, the owner and the contractor could be liable for substantial penalties.

The contractor is solely responsible for designing and constructing stable, temporary excavations and should shore, slope, or bench the sides of the excavations as required to maintain stability of both the excavation sides and bottom. The contractor's responsible person, as defined in 29 CFR Part 1926, should evaluate the soil exposed in the excavations as part of the contractor's safety procedures. In no case should slope height, slope inclination, or excavation depth, including utility trench excavation depth, exceed those specified in local, state, and federal safety regulations. UES are providing this information solely as a service to UES's client.

UES is not assuming responsibility for construction site safety or the contractor's activities; such responsibility is not being implied and should not be inferred.



6.0 REPORT LIMITATIONS

This consulting report has been prepared for the exclusive use of the current project owners and other members of the design team for the proposed project. This report has been prepared in accordance with generally accepted local geotechnical engineering practices; no other warranty is expressed or implied. The evaluation submitted in this report, is based in part upon the data collected during a field exploration, however, the nature and extent of variations throughout the subsurface profile may not become evident until the time of construction.

If variations then appear evident, it may be necessary to reevaluate information and professional opinions as provided in this report. In the event changes are made in the nature, design, or locations of the proposed structures, the evaluation and opinions contained in this report shall not be considered valid, unless the changes are reviewed and conclusions modified or verified in writing by UES. Lastly, in accepting this report, the client understands that the data obtained from the soil borings is intended for foundation analysis only and is not to be used for excavating or backfilling pricing estimates.

7.0 BASIS FOR RECOMMENDATIONS

The analysis and recommendations submitted in this report are based on the data obtained from the tests performed at the locations indicated on the attached **Figure 3**. This report does not reflect any variations, which may occur between borings. While the borings are representative of the subsurface conditions at their respective locations and for their vertical reaches, local variations characteristic of the subsurface soils of the region are anticipated and may be encountered. The delineation between soil types shown on the soil logs is approximate and the description represents UES's interpretation of the subsurface conditions at the designated boring locations on the particular date drilled.

UES should be provided the opportunity to review the final foundation specifications and review foundation design drawings, in order to determine whether UES's recommendations have been properly interpreted, communicated and implemented.

If UES is not afforded the opportunity to participate in construction related aspects of foundation installation as recommended in this report or any report addendum, UES will accept no responsibility for the interpretation of UES's recommendations made in this report or on a report addendum for foundation performance.

Any third-party reliance of UES's geotechnical report or parts thereof is strictly prohibited without the expressed written consent of UES. The SPT methodology (ASTM D-1586) used in performing UES's borings and for determining penetration resistance is specific to the sampling tools utilized and does not reflect the ease or difficulty to advance other tools, equipment or materials.



APPENDIX A
Record of Test Borings





GFA International, Inc.
1215 Wallace Drive
Delray Beach, FL 33444
561-347-0070

LOG OF BORING B01

PAGE 1 OF 1

CLIENT Mike Carter Construction, Inc.

PROJECT NUMBER 0630.2200058.0000

DRILLING CONTRACTOR Universal Engineering Sciences

DRILLER Jose Barrios

DRILL RIG CME-75

METHOD Standard Penetration Test

NOTE:

PROJECT NAME 3090 Sheridan Street - Hollywood

PROJECT LOCATION 3090 Sheridan Street, Hollywood FL 33021

HOLE DEPTH 25 ft

HOLE DIAMETER

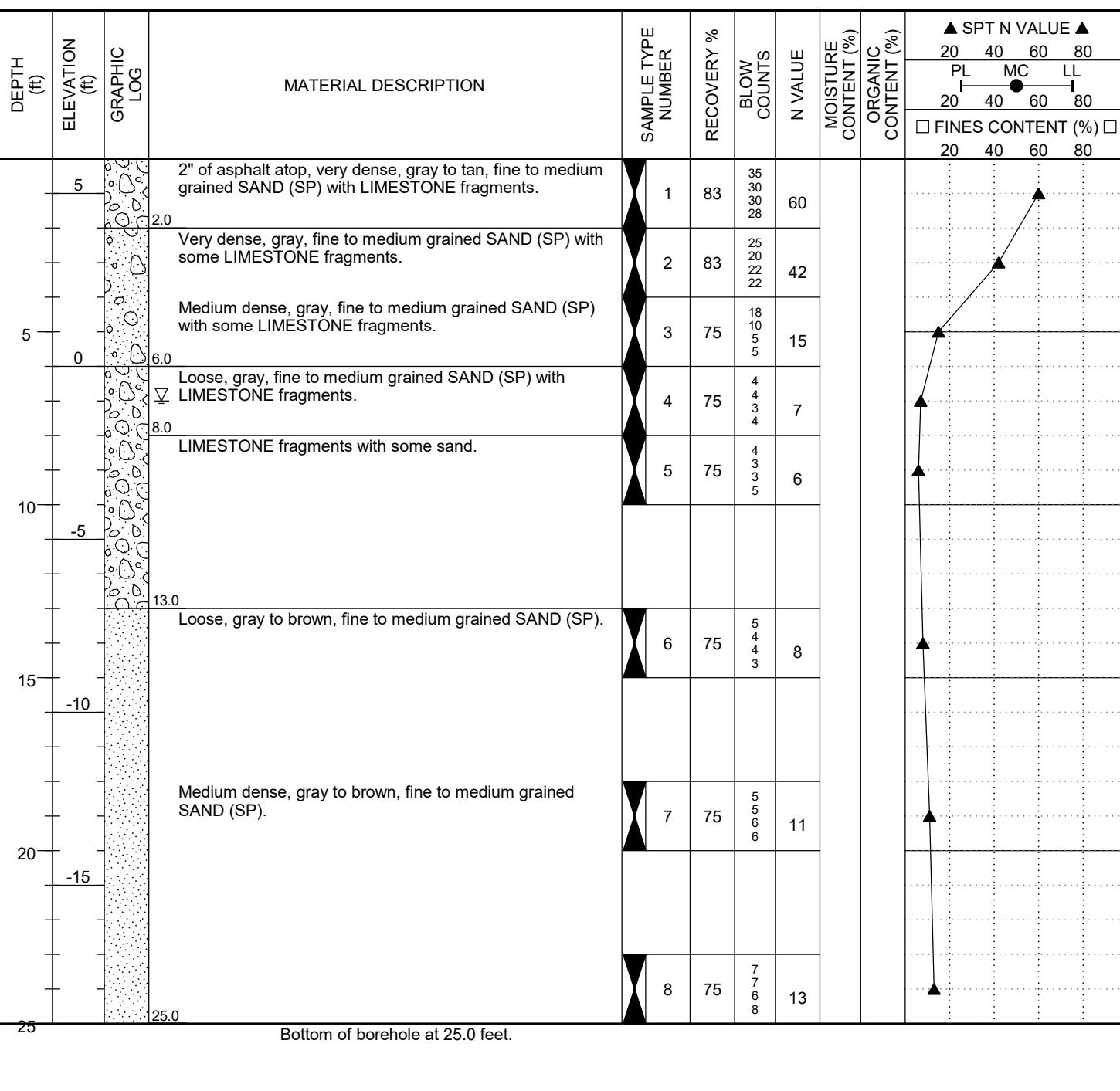
DATE STARTED 4/25/22

COMPLETED 4/25/22

GROUND WATER LEVEL: AT TIME OF DRILLING 7.00 ft / Elev -1.00 ft

LATITUDE 26.031006 LONGITUDE -80.169871

HAMMER TYPE 140# with 30 in Drop - Automatic Hammer





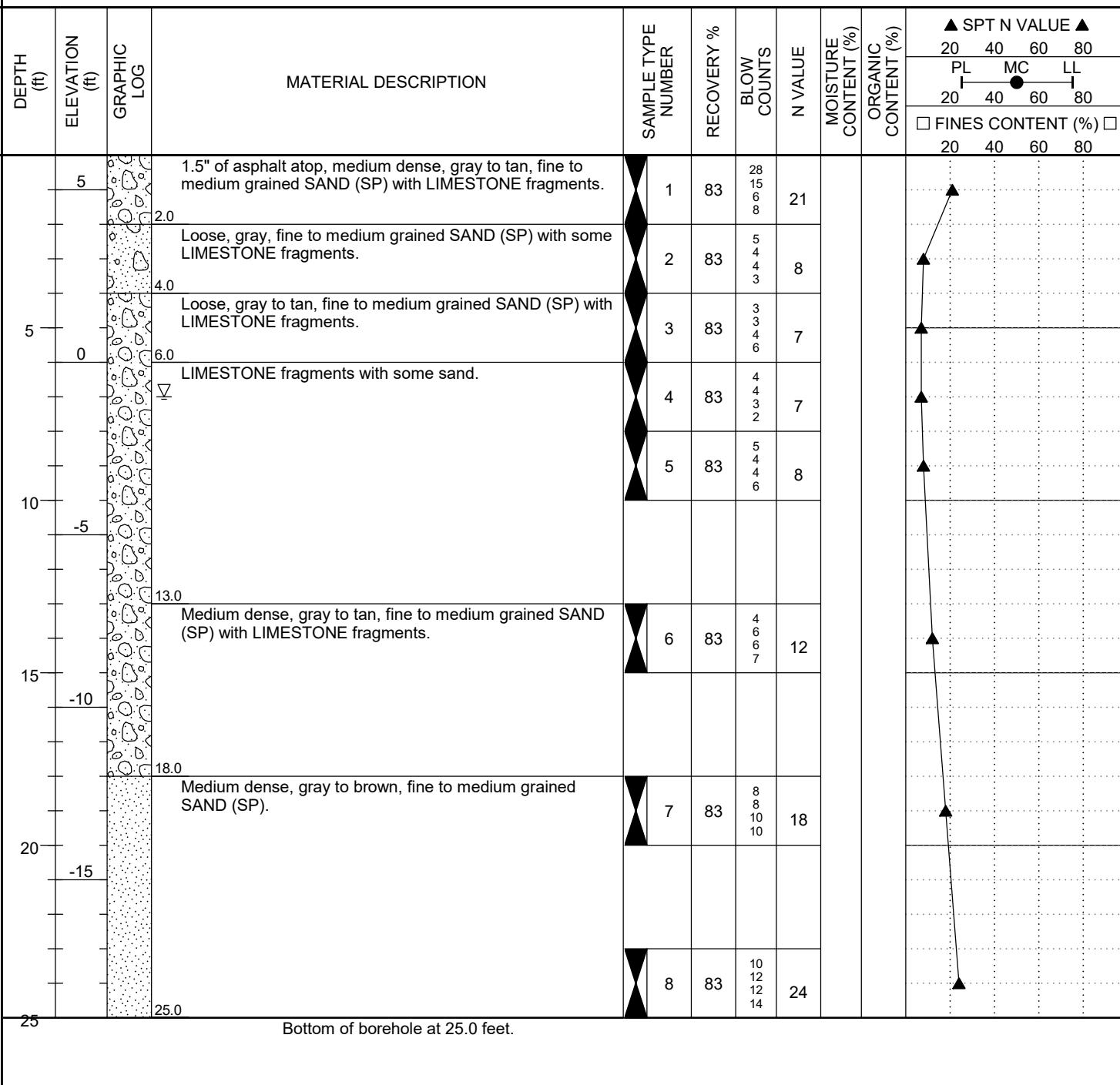
GFA International, Inc.
1215 Wallace Drive
Delray Beach, FL 33444
561-347-0070

LOG OF BORING B02

PAGE 1 OF 1

CLIENT Mike Carter Construction, Inc.
PROJECT NUMBER 0630.2200058.0000
DRILLING CONTRACTOR Universal Engineering Sciences
DRILLER Jose Barrios
DRILL RIG CME-75
METHOD Standard Penetration Test
NOTE:

PROJECT NAME 3090 Sheridan Street - Hollywood
PROJECT LOCATION 3090 Sheridan Street, Hollywood FL 33021
HOLE DEPTH 25 ft
HOLE DIAMETER
DATE STARTED 4/26/22
COMPLETED 4/26/22
GROUND WATER LEVEL: AT TIME OF DRILLING 7.00 ft / Elev -1.00 ft
LATITUDE 26.031009
LONGITUDE -80.169634
HAMMER TYPE 140# with 30 in Drop - Automatic Hammer





GFA International, Inc.
1215 Wallace Drive
Delray Beach, FL 33444
561-347-0070

LOG OF BORING B03

PAGE 1 OF 1

CLIENT Mike Carter Construction, Inc.

PROJECT NUMBER 0630.2200058.0000

DRILLING CONTRACTOR Universal Engineering Sciences

DRILLER Jose Barrios

DRILL RIG CME-75

METHOD Standard Penetration Test

NOTE:

PROJECT NAME 3090 Sheridan Street - Hollywood

PROJECT LOCATION 3090 Sheridan Street, Hollywood FL 33021

HOLE DEPTH 25 ft

HOLE DIAMETER

DATE STARTED 4/26/22

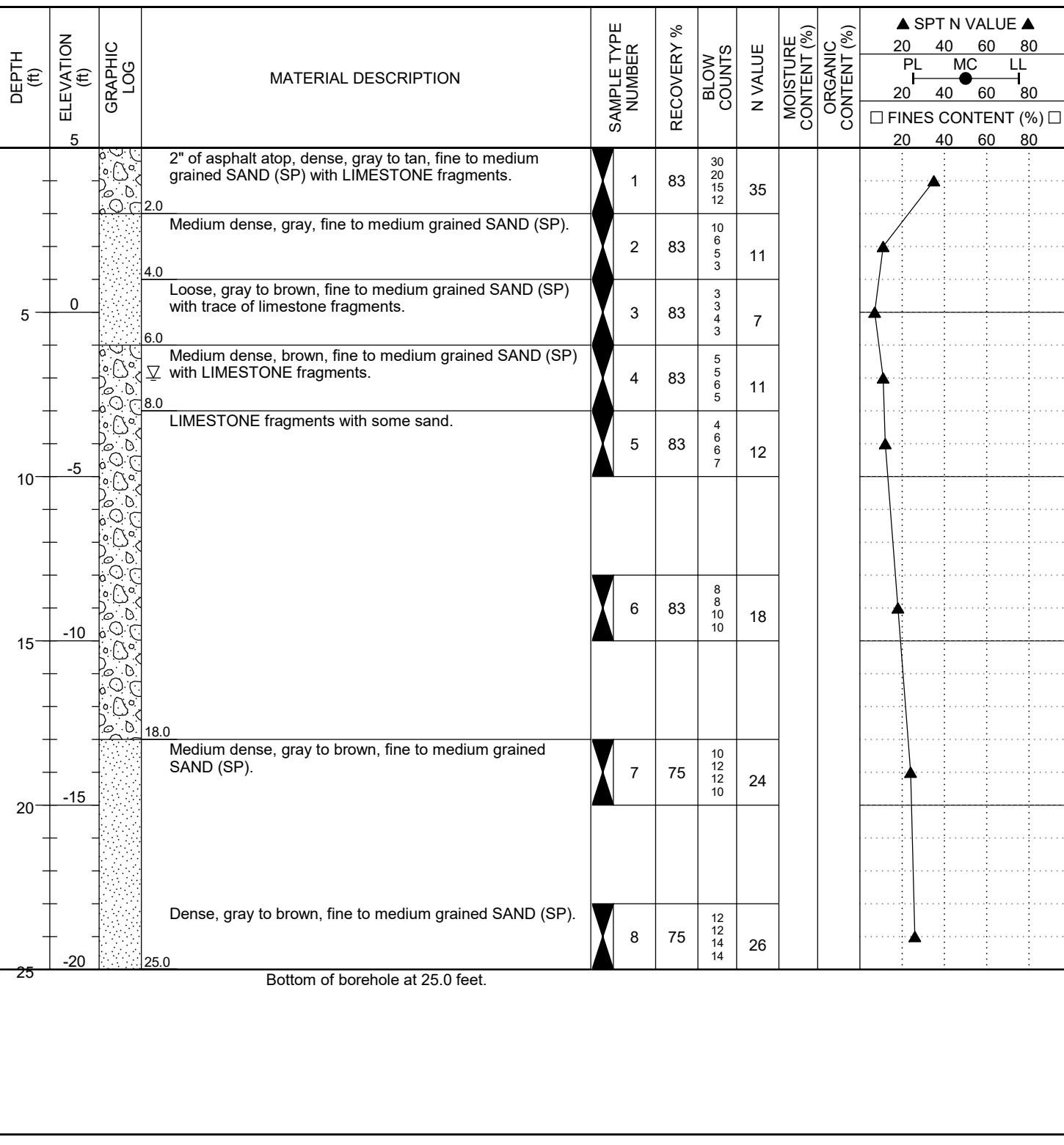
COMPLETED 4/26/22

GROUND WATER LEVEL: AT TIME OF DRILLING 7.00 ft / Elev -2.00 ft

LATITUDE 26.030367

LONGITUDE -80.169598

HAMMER TYPE 140# with 30 in Drop - Automatic Hammer





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1215 Wallace Drive
Delray Beach, FL 33444
561-347-0070

LOG OF BORING B04

PAGE 1 OF 1

CLIENT Mike Carter Construction, Inc.

PROJECT NUMBER 0630.2200058.0000

DRILLING CONTRACTOR Universal Engineering Sciences

DRILLER Jose Barrios

DRILL RIG CME-75

METHOD Standard Penetration Test

NOTE:

PROJECT NAME 3090 Sheridan Street - Hollywood

PROJECT LOCATION 3090 Sheridan Street, Hollywood FL 33021

HOLE DEPTH 25 ft

HOLE DIAMETER

DATE STARTED 4/25/22

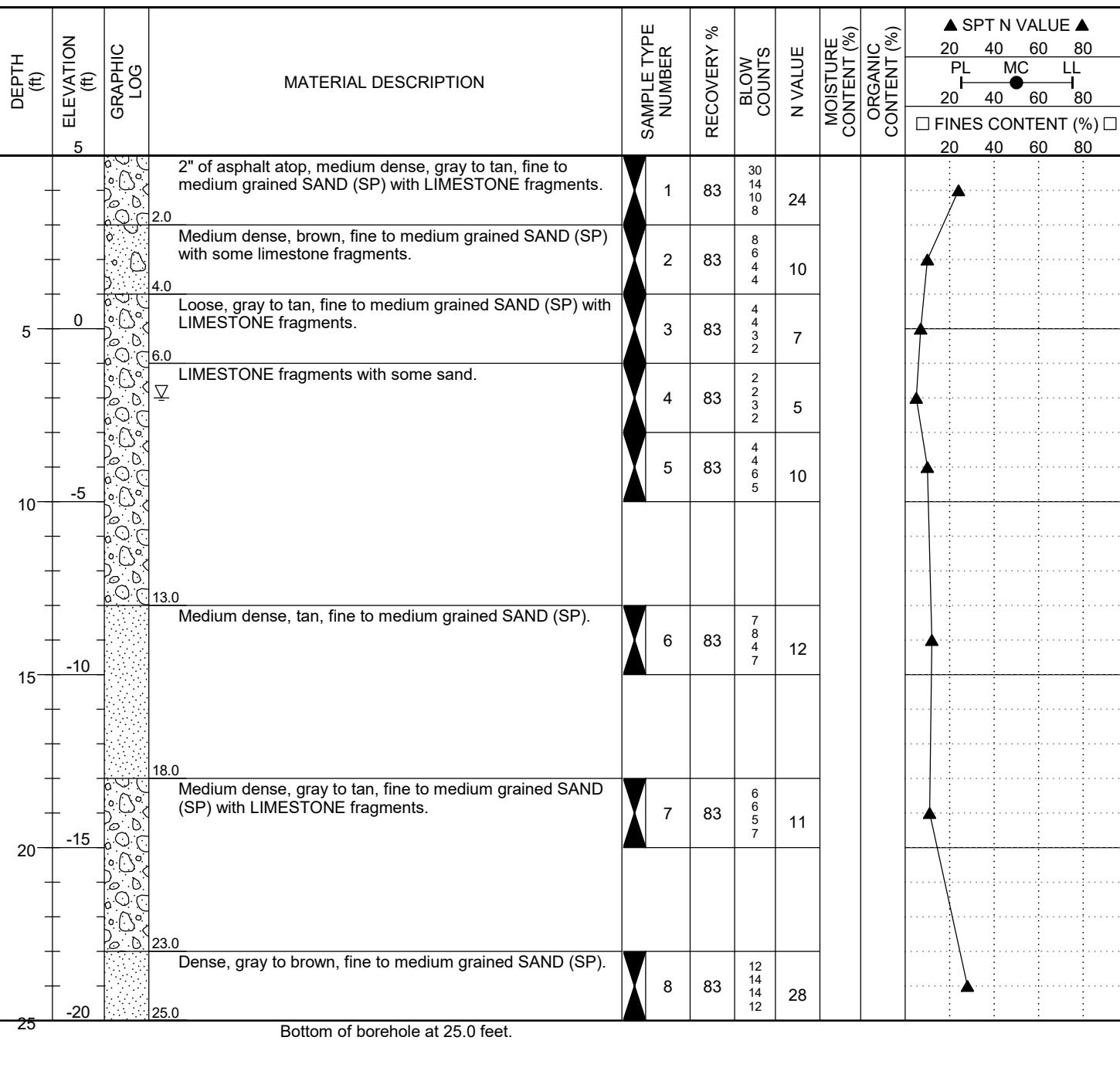
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GROUND WATER LEVEL: AT TIME OF DRILLING 7.00 ft / Elev -2.00 ft

LATITUDE 26.030349

LONGITUDE -80.169853

HAMMER TYPE 140# with 30 in Drop - Automatic Hammer





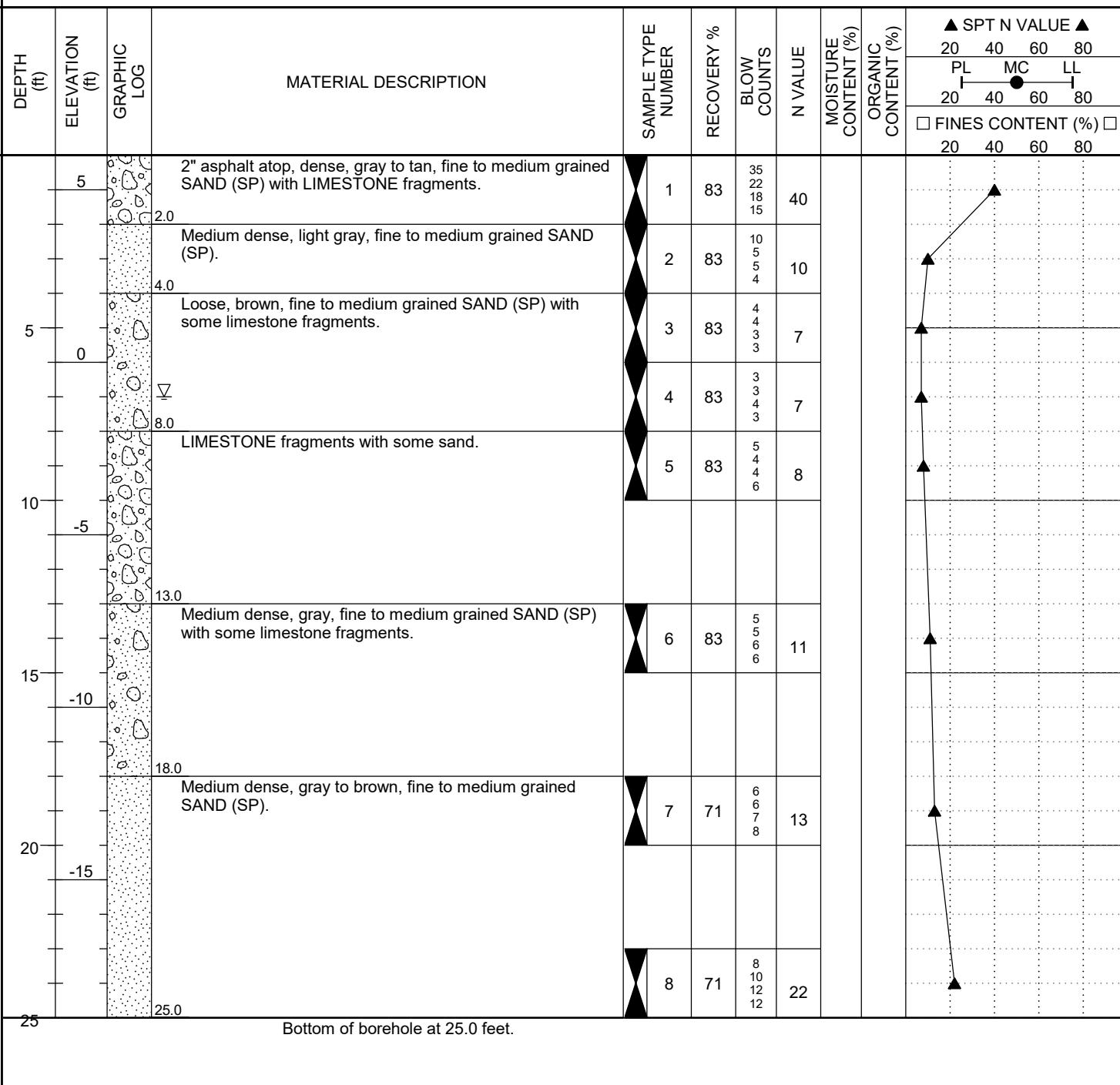
GFA International, Inc.
1215 Wallace Drive
Delray Beach, FL 33444
561-347-0070

LOG OF BORING B05

PAGE 1 OF 1

CLIENT Mike Carter Construction, Inc.
PROJECT NUMBER 0630.2200058.0000
DRILLING CONTRACTOR Universal Engineering Sciences
DRILLER Jose Barrios
DRILL RIG CME-75
METHOD Standard Penetration Test
NOTE:

PROJECT NAME 3090 Sheridan Street - Hollywood
PROJECT LOCATION 3090 Sheridan Street, Hollywood FL 33021
HOLE DEPTH 25 ft
HOLE DIAMETER
DATE STARTED 4/26/22
COMPLETED 4/26/22
GROUND WATER LEVEL: AT TIME OF DRILLING 7.00 ft / Elev -1.00 ft
LATITUDE 26.030685
LONGITUDE -80.169628
HAMMER TYPE 140# with 30 in Drop - Automatic Hammer





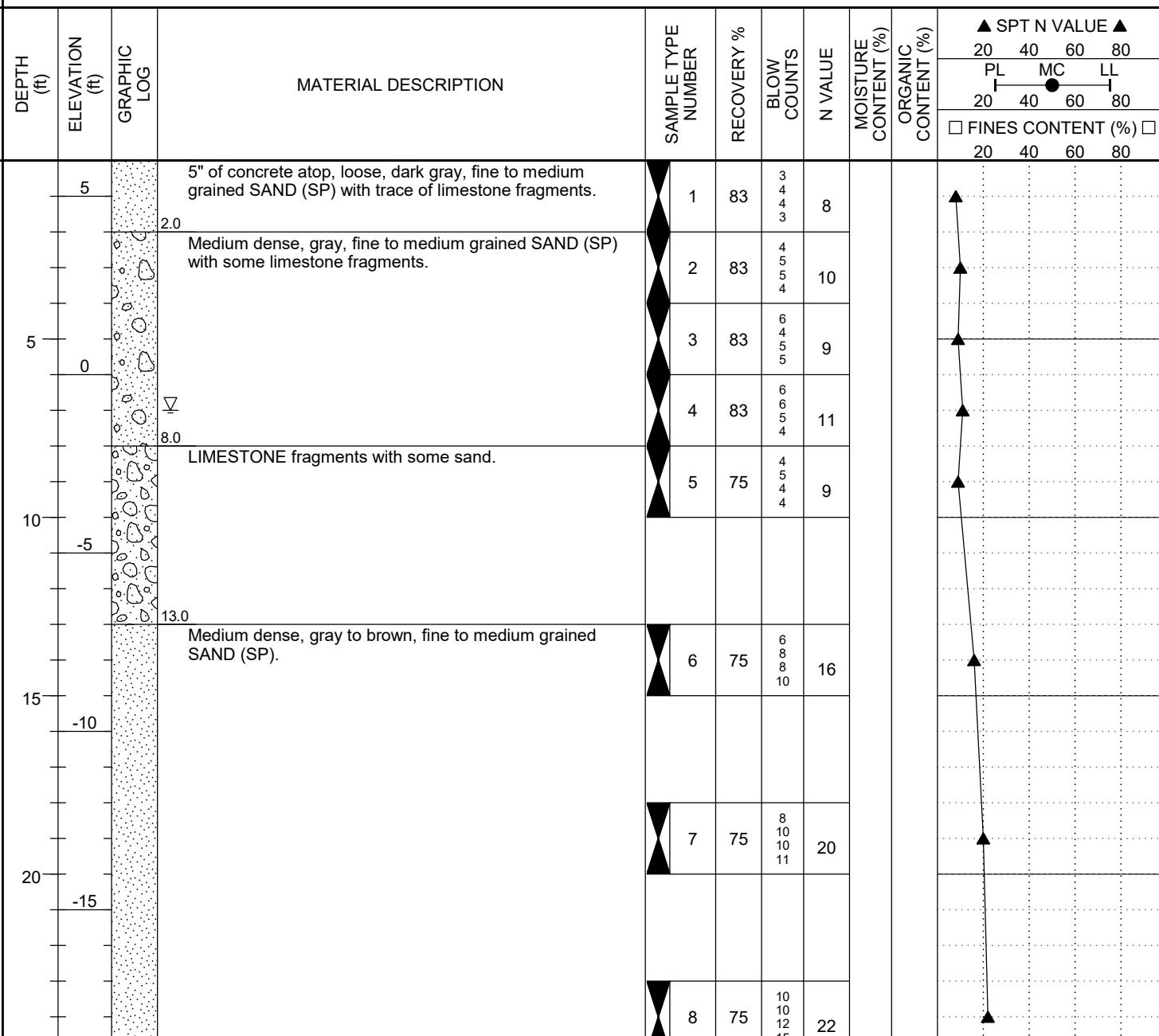
GFA International, Inc.
1215 Wallace Drive
Delray Beach, FL 33444
561-347-0070

LOG OF BORING B06

PAGE 1 OF 1

CLIENT Mike Carter Construction, Inc.
PROJECT NUMBER 0630.2200058.0000
DRILLING CONTRACTOR Universal Engineering Sciences
DRILLER Jose Barrios
DRILL RIG CME-75
METHOD Standard Penetration Test
NOTE:

PROJECT NAME 3090 Sheridan Street - Hollywood
PROJECT LOCATION 3090 Sheridan Street, Hollywood FL 33021
HOLE DEPTH 25 ft
HOLE DIAMETER
DATE STARTED 4/25/22
COMPLETED 4/25/22
GROUND WATER LEVEL: AT TIME OF DRILLING 7.00 ft / Elev -1.00 ft
LATITUDE 26.030679
LONGITUDE -80.169868
HAMMER TYPE 140# with 30 in Drop - Automatic Hammer





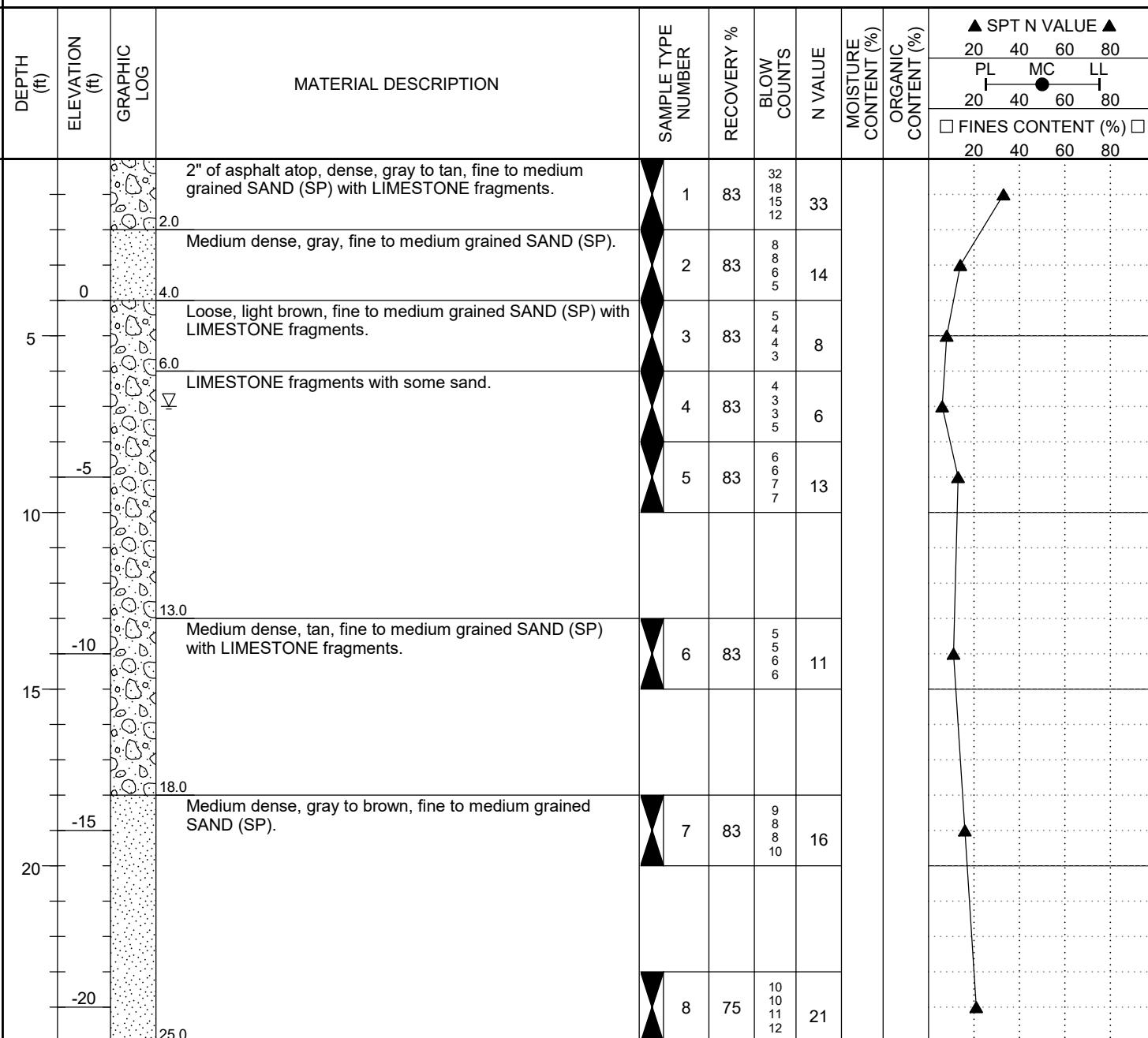
GFA International, Inc.
1215 Wallace Drive
Delray Beach, FL 33444
561-347-0070

LOG OF BORING B07

PAGE 1 OF 1

CLIENT Mike Carter Construction, Inc.
PROJECT NUMBER 0630.2200058.0000
DRILLING CONTRACTOR Universal Engineering Sciences
DRILLER Jose Barrios
DRILL RIG CME-75
METHOD Standard Penetration Test
NOTE:

PROJECT NAME 3090 Sheridan Street - Hollywood
PROJECT LOCATION 3090 Sheridan Street, Hollywood FL 33021
HOLE DEPTH 25 ft
HOLE DIAMETER
DATE STARTED 4/26/22
COMPLETED 4/26/22
GROUND WATER LEVEL: AT TIME OF DRILLING 7.00 ft / Elev -3.00 ft
LATITUDE 26.030841
LONGITUDE -80.169729
HAMMER TYPE 140# with 30 in Drop - Automatic Hammer





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1215 Wallace Drive
Delray Beach, FL 33444
561-347-0070

LOG OF BORING B08

PAGE 1 OF 1

CLIENT Mike Carter Construction, Inc.

PROJECT NUMBER 0630.2200058.0000

DRILLING CONTRACTOR Universal Engineering Sciences

DRILLER Jose Barrios

DRILL RIG CME-75

METHOD Standard Penetration Test

NOTE:

PROJECT NAME 3090 Sheridan Street - Hollywood

PROJECT LOCATION 3090 Sheridan Street, Hollywood FL 33021

HOLE DEPTH 25 ft

HOLE DIAMETER

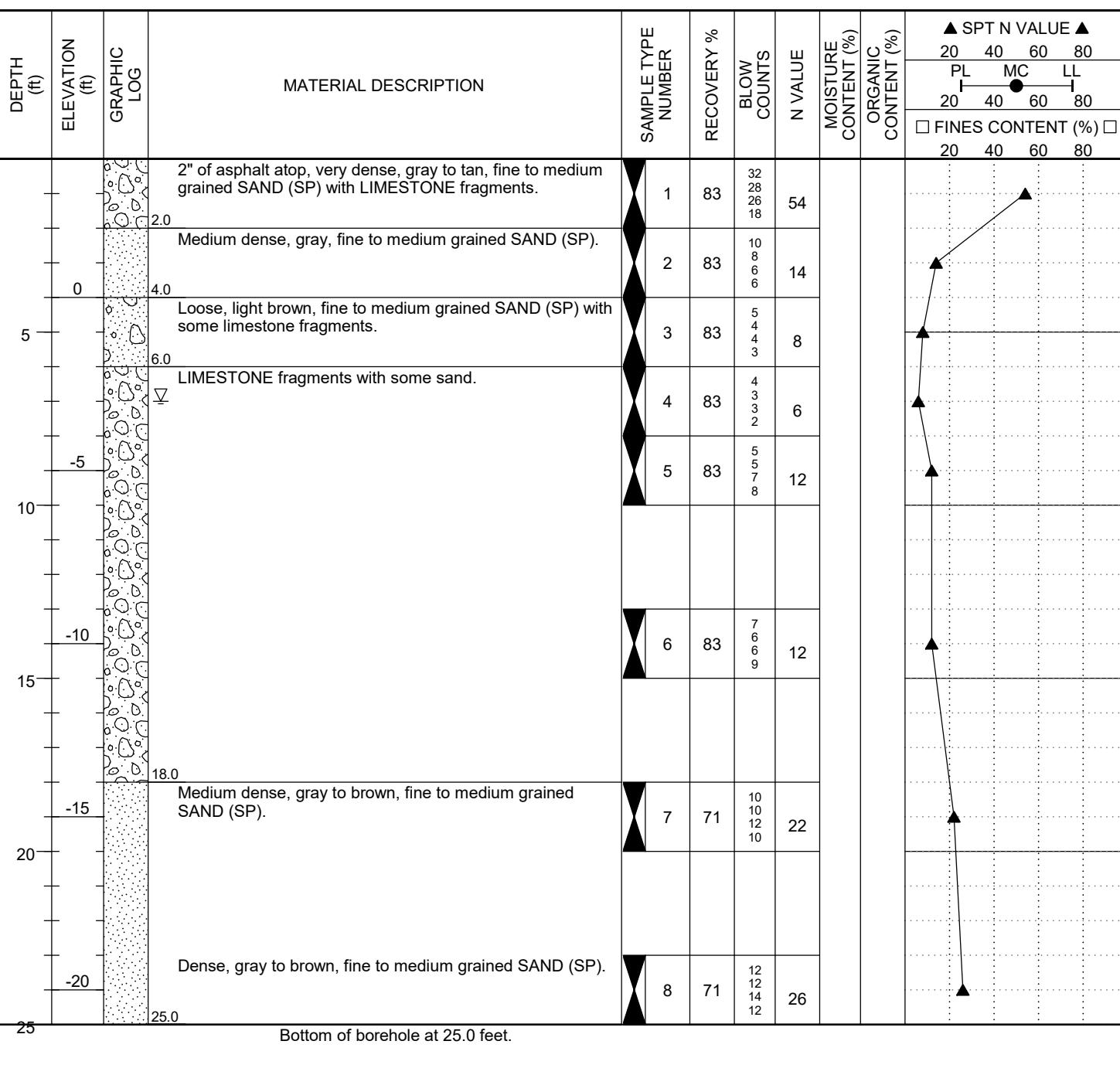
DATE STARTED 4/26/22

COMPLETED 4/26/22

GROUND WATER LEVEL: AT TIME OF DRILLING 7.00 ft / Elev -3.00 ft

LATITUDE 26.030506 LONGITUDE -80.169723

HAMMER TYPE 140# with 30 in Drop - Automatic Hammer





GFA International, Inc.
1215 Wallace Drive
Delray Beach, FL 33444
561-347-0070

BORING NUMBER EX01

PAGE 1 OF 1

CLIENT Mike Carter Construction, Inc.

PROJECT NAME 3090 Sheridan Street - Hollywood

PROJECT NUMBER 0630.2200058.0000

PROJECT LOCATION 3090 Sheridan Street, Hollywood FL 33021

DATE STARTED 4/25/22 COMPLETED 4/25/22

LATITUDE 26.031202 LONGITUDE -80.169956

DRILLING CONTRACTOR Universal Engineering Sciences

GROUND WATER LEVELS:

DRILLING METHOD

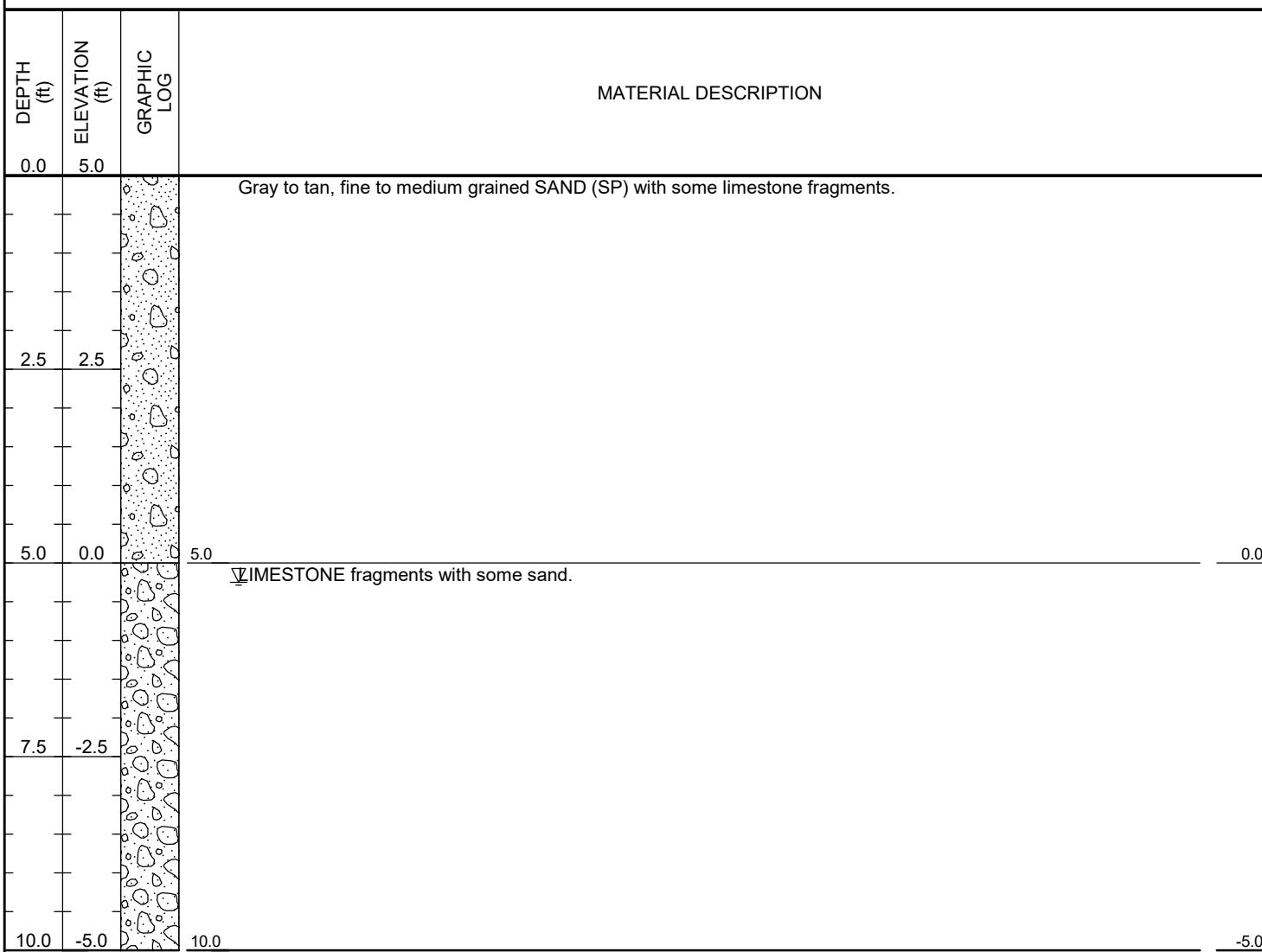
AT TIME OF DRILLING 5.25 ft / Elev -0.25 ft

LOGGED BY Jose Barrios CHECKED BY David Lopez

AT END OF ---

NOTES

AFTER DRILLING ---





GFA International, Inc.
1215 Wallace Drive
Delray Beach, FL 33444
561-347-0070

BORING NUMBER EX02

PAGE 1 OF 1

CLIENT Mike Carter Construction, Inc.

PROJECT NAME 3090 Sheridan Street - Hollywood

PROJECT NUMBER 0630.2200058.0000

PROJECT LOCATION 3090 Sheridan Street, Hollywood FL 33021

DATE STARTED 4/25/22 COMPLETED 4/25/22

LATITUDE 26.030429 LONGITUDE -80.16995

DRILLING CONTRACTOR Universal Engineering Sciences

GROUND WATER LEVELS:

DRILLING METHOD

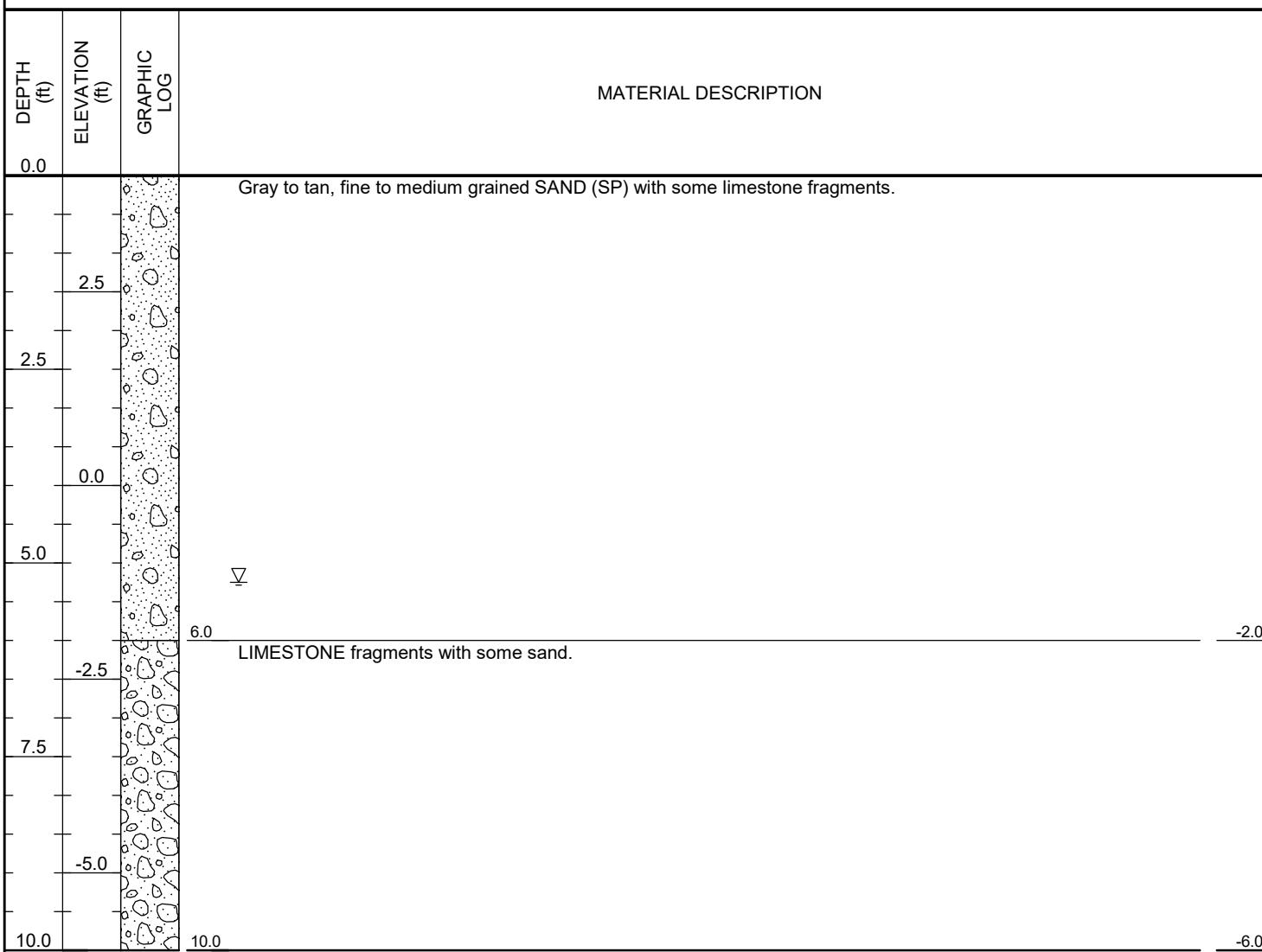
▽ AT TIME OF DRILLING 5.25 ft / Elev -1.25 ft

LOGGED BY Jose Barrios CHECKED BY David Lopez

AT END OF ---

NOTES

AFTER DRILLING ---



APPENDIX B
Notes Related to Test Borings



**NOTES RELATED TO
RECORDS OF TEST BORING AND
GENERALIZED SUBSURFACE PROFILE**

1. Groundwater level was encountered and recorded (if shown) following the completion of the soil test boring on the date indicated. Fluctuations in groundwater levels are common; consult report text for a discussion.
2. The boring location was identified and located in the field based on measured and estimated distances from existing site features.
3. The borehole was backfilled to site grade following boring completion, patched with asphalt cold patch mix when pavement was encountered.
4. The Record of Test Boring represents our interpretation of field conditions based on engineering examination of the soil samples.
5. The Record of Test Boring is subject to the limitations, conclusions, and recommendations presented in the report text.
6. The Standard Penetration Test (SPT) was performed in accordance ASTM Procedure D-1586. SPT testing procedure consists of driving a 1.4-inch I.D. split-tube sampler into the soil profile using a 140-pound hammer falling 30 inches.
7. On the Record of Test Boring listed as "Blow Counts", the N-value is the sum of the SPT hammer blows required to drive the split-tube sampler through the second and third 6-inch increment of the sampling layer, and is an indication of soil strength.
8. Shown on the Record of Test Boring an SPT N-value expressed as 50/2" is descriptive of the fact that 50 hammer blows were required to drive the split-spoon sampler a distance of approximately 2 inches.
9. The soil/rock strata interfaces shown on the Records of Test Boring are approximate and may vary from those in the field. The soil/rock conditions shown on the Records of Test Boring refer to conditions at the specific location tested; soil/rock conditions may vary between test locations.
10. Relative density and consistency for sands/gravels, silts/clays, and limestone are described as follows:

Cohesionless Soils	
SPT (N-Value)	Relative Density
0 – 3	Very Loose
4 – 8	Loose
9 – 24	Medium Dense
25 – 40	Dense
Over 40	Very Dense

Silts and Clays	
SPT (N-Value)	Consistency
0 – 1	Very Soft
2 – 4	Soft
4 – 6	Firm
7 – 12	Stiff
13 – 24	Very Stiff
Over 24	Hard

Limestone	
SPT (N-Value)	Relative Density
0 – 19	Very Soft
20 – 49	Soft
50 – 100	Medium Hard
50 for 3 to 5"	Moderately Hard
50 for 0 to 2"	Hard

11. Definition of descriptive terms of modifiers for silts/clays/shells/gravels are described as follows:

Percentage of Modifier Material	First Qualifier	Second Qualifier
0 – 5	With a Trace of + Silt, Clay, Shell	With a Trace
5 – 12	Slightly + Silty, Clayey, Shelly	With Some
12 – 30	Silty, Clayey, Shelly	With
30 – 50	Very + Silty, Clayey, Shelly	And

12. Descriptive characteristics for organic content percentages are described as follows:

Percentage of Organic Material	Descriptor
0 – 5	With a Trace
5 – 20	With Organics
20 – 75	Highly Organic
75 – 100	Peat

APPENDIX C

Discussion of Soil Groups



DISCUSSION OF SOIL GROUPS

COARSE GRAINED SOILS

GW and SW GROUPS. These groups comprise well-graded gravelly and sandy soils having little or no plastic fines (less than 5 percent passing the No. 200 sieve). The presence of the fines must not noticeably change the strength characteristics of the coarse-grained fraction and must not interface with its free-draining characteristics.

GP and SP GROUPS. Poorly graded gravels and sands containing little or no plastic fines (less than 5 percent passing the No. 200 sieve) are classed in GP and SP groups. The materials may be called uniform gravels, uniform sands or non-uniform mixtures of very coarse material and very fine sands, with intermediate sizes lacking (sometimes called skip-graded, gap-graded or step-graded). This last group often results from borrow pit excavation in which gravel and sand layers are mixed.

GM and SM GROUPS. In general, the GM and SM groups comprise gravels or sands with fines (more than 12 percent passing the No. 200 sieve) having low or no plasticity. The plasticity index and liquid limit of soils in the group should plot below the "A" line on the plasticity chart. The gradation of the material is not considered significant and both well and poorly graded materials are included.

GC and SC GROUPS. In general, the GC and SC groups comprise gravelly or sandy soils with fines (more than 12 percent passing the No. 200 sieve), which have a fairly high plasticity. The liquid limit and plasticity index should plot above the "A" line on the plasticity chart.

FINE GRAINED SOILS

ML and MH GROUPS. In these groups, the symbol M has been used to designate predominantly silty material. The symbols L and H represent low and high liquid limits, respectively, and an arbitrary dividing line between the two is set at a liquid limit of 50. The soils in the ML and MH groups are sandy silts, clayey silts or inorganic silts with relatively low plasticity. Also included are loess type soils and rock flours.

CL and CH GROUPS. In these groups the symbol C stands for clay, with L and H denoting low or high liquid limits, with the dividing line again set at a liquid limit of 50. The soils are primarily inorganic clays. Low plasticity clays are classified as CL and are usually lean clays, sandy clays or silty clays. The medium and high plasticity clays are classified as CH. These include the fat clays, gumbo clays and some volcanic clays.



APPENDIX D
Hydraulic Conductivity Results





HYDRAULIC CONDUCTIVITY TEST RESULTS

Project name: _____
3090 Sheridan Street - Hollywood

Project number: _____
0630.2200058.0000

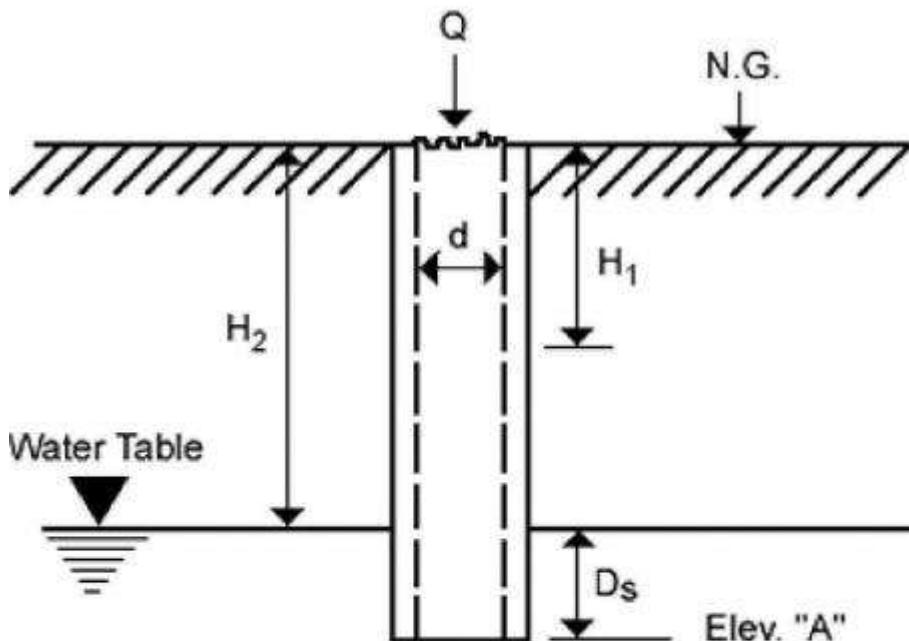
Date: _____
4/25/2022

SFWMD USUAL Open Hole Formula:

$$K = \frac{4Q}{\pi d(2H_2^2 + 4H_2 D_s + H_2 d)}$$

Exfiltration Number	First volume of water reading (after stabilization)	Final volume of water reading	Elapsed time	Average flow rate at constant head Q	Average flow rate at constant head Q*0.00223	Perforated casing diameter or hole diameter (d)	Water table H ₂	Total length of bore hole	Length of bore hole below stabilized ground water (D _s)	Hydraulic Conductivity (K)
	gallon	gallon	min.	gallon/minute	ft ³ /sec	ft	ft	ft	ft	ft ³ /sec/ft ² - ft of head
E-1	0.0	43.00	10	4.30	0.0096	0.25	5.25	10.0	4.8	3.13E-04
E-2	0.0	49.00	10	4.90	0.0109	0.25	5.25	10.0	4.8	3.56E-04

USUAL OPEN-HOLE TEST



$$K = \frac{4Q}{\pi d (2H_2^2 + 4H_2 D_s + H_2 d)}$$

K = Hydraulic Conductivity (cfs/ft.² – ft. head)

Q = "Stabilized" Flow Rate (cfs)

d = Diameter of Test Hole (ft)

H₂ = Depth to Water Table (ft)

D_S = Saturated Hole Depth (ft)

Elev. "A" = Proposed Trench Bottom Elev. (ft – NGVD)

H₁ = Average Head on Unsaturated Hole Surface (ft. head)





3325 S. University Drive, Suite 111, Davie, FL 33328

Main Office: 954.318.0624

Fax: 954.358.0190

Web: www.rossengineers.com

APPENDIX E

Drainage As-Builts

Safeguard Self Storage - Hollywood

3090 SHERMAN ST. HOLLYWOOD, FL 33021

CITY OF HOLLYWOOD FINAL TAC MEETING - MAY 8, 2023
CITY OF HOLLYWOOD FINAL TAC SUBMITTAL - APRIL 17, 2023

LAND DESCRIPTION:

THE NORTH 985.83 FEET OF PARCEL "A", TAFT STREET INDUSTRIAL PARK, ACCORDING TO THE PLAT THEREOF, AS RECORDED IN PLAT BOOK 122, PAGE 25, OF THE PUBLIC RECORDS OF BROWARD COUNTY, FLORIDA

SAID LANDS LYING IN THE CITY OF HOLLYWOOD, BROWARD COUNTY, FLORIDA AND CONTAINING NET TOTAL OF 228,712.56 SQUARE FEET (5.25 ACRES) MORE OR LESS.



LOCATION MAP

NOT TO SCALE

CITY OF HOLLYWOOD PERMIT SET

Sheet Number	Sheet Title
COVER	COVER SHEET
C-1	GENERAL NOTES AND SPECIFICATIONS
C-1.1	Civil Site Plan
C-2	DEMOLITION PLAN
C-3	GRADING AND DRAINAGE PLAN
C-3.1	PAVING SIDEWALK AND CURBING PLAN
C-3.2	PAVING GRADING AND DRAINAGE DETAILS (SHEET 1 OF 3)
C-3.3	PAVING GRADING AND DRAINAGE DETAILS (SHEET 2 OF 3)
C-3.4	PAVING GRADING AND DRAINAGE DETAILS (SHEET 3 OF 3)
C-3.5	RAINTANK DETAILS
C-4	WATER AND SEPTIC PLAN
C-4.1	WATER AND SEPTIC PLAN DETAILS (SHEET 1 OF 2)
C-4.2	WATER AND SEPTIC PLAN DETAILS (SHEET 2 OF 2)
C-5	PAVEMENT MARKING AND SIGNAGE PLAN
C-5.1	PAVEMENT MARKING AND SIGNAGE DETAILS
C-6	POLLUTION PREVENTION CONTROL PLAN
C-6.1	POLLUTION PREVENTION PLAN DETAILS
EXHIBIT	FIRE TRUCK TRACKING

REVISION #4 Date: 4/14/2023

PREVIOUS CITY OF HOLLYWOOD SUBMISSIONS

CITY OF HOLLYWOOD PRELIMINARY TAC MEETING - FEBRUARY 21, 2023
CITY OF HOLLYWOOD PRELIMINARY TAC SUBMITTAL - FEBRUARY 6, 2023
CITY OF HOLLYWOOD PACO MEETING - NOVEMBER 17, 2022

PREPARED FOR:

MIKE CARTER CONSTRUCTION, INC.



3325 S. UNIVERSITY DRIVE, SUITE 111
DAVIE, FLORIDA 33328
(954)318-0624 (954)358-0190 FAX
CERTIFICATE OF AUTHORIZATION No. 9808

1. APPLICABLE CODES

- ALL CONSTRUCTION AND MATERIALS SHALL CONFORM TO THE STANDARD AND SPECIFICATIONS OF CITY OF HOLLYWOOD AND ALL OTHER LOCAL STATE AND NATIONAL CODES WHERE APPLICABLE EXCEPT WITHIN DEPARTMENT OF TRANSPORTATION (D.O.T.) R/W WHEREIN FLORIDA DEPARTMENT OF TRANSPORTATION (F.D.O.T.) GOVERNS.
- ALL CONSTRUCTION SHALL BE DONE IN A SAFE MANNER AND IN STRICT COMPLIANCE WITH ALL THE REQUIREMENTS OF FEDERAL OCCUPATIONAL SAFETY AND HEALTH ACT OF 1970, AND ALL STATE AND LOCAL SAFETY AND HEALTH REGULATIONS.
- ALL ELEVATIONS SHOWN ON THE CONSTRUCTION DRAWINGS ARE BASED ON THE NORTH AMERICAN VERTICAL DATUM 1988, (NAVD) UNLESS OTHERWISE NOTED.
- CITY OF HOLLYWOOD WATER AND SEWER DEPARTMENT WATER AND SEWER SPECIFICATIONS SHALL DICTATE WHEN IN CONFLICT WITH ANY OF THE FOLLOWING SPECIFICATIONS.
- ALL MATERIALS AND CONSTRUCTION WITHIN THE D.O.T. R/W SHALL CONFORM TO THE D.O.T. "DESIGN STANDARDS" (2015) AND "STANDARD SPECIFICATIONS" (2015).
- THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING THE MAINTENANCE OF TRAFFIC (M.O.T.) PLAN PRIOR TO CONSTRUCTION. THE CONSULTANT'S ENGINEER SHALL ENSURE THAT THE M.O.T. PLAN FOR THE PROJECT CONFORMS WITH STANDARD INDUSTRIES 600, APPLICABLE CODE FOR WORK BEING PERFORMED. THE CONTRACTOR SHALL ENSURE THE M.O.T. PLAN IS IMPLEMENTED EXACTLY AS APPROVED.

2. PRECONSTRUCTION RESPONSIBILITIES

- UPON THE RECEIPT OF THE "NOTICE TO PROCEED", THE CONTRACTOR SHALL CONTACT THE ENGINEER OF RECORD AND ARRANGE A PRECONSTRUCTION CONFERENCE TO INCLUDE ALL INVOLVED GOVERNMENTAL AGENCIES, UTILITY OWNERS, THE OWNER AND THE ENGINEER OF RECORD.
- THE CONTRACTOR SHALL OBTAIN A SUNSHINE STATE ONE CALL OF FLORIDA, INC. CERTIFICATION NUMBER AT LEAST 48 HOURS PRIOR TO BEGINNING ANY EXCAVATION CALL 1-800-432-4770.
- ALL UTILITY EASEMENTS TO BE SECURED PRIOR TO CONSTRUCTION (IF REQUIRED).
- LOCATION OF EXISTING FACILITIES AS SHOWN ON CONSTRUCTION DRAWINGS ARE DRAWN FROM AVAILABLE RECORDS. THE ENGINEER ASSUMES NO RESPONSIBILITY FOR THE ACCURACY OF THE FACILITIES SHOWN, OR FOR ANY FACILITY NOT SHOWN. THE CONTRACTOR SHALL NOTIFY, AS POSSIBLE, THE ELEVATIONS AND LOCATIONS OF EXISTING FACILITIES PRIOR TO CONSTRUCTION. AN EXISTING FACILITY IS FOUND TO CONFLICT WITH THE PROPOSED CONSTRUCTION UPON EXCAVATION, THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ENGINEER OF RECORD SO THAT APPROPRIATE MEASURES CAN BE TAKEN TO RESOLVE THE PROBLEM.
- THE CONTRACTOR MUST CALL CITY OF HOLLYWOOD AT LEAST 48 HOURS BEFORE ANY EXCAVATION WITHIN THE R/W TO DETERMINE THE LOCATION TO OF THE EXISTING TRAFFIC SIGNAL INTERCONNECT CABLE.

3. INSPECTIONS

- THE CONTRACTOR SHALL NOTIFY CITY OF HOLLYWOOD AND ANY OTHER GOVERNMENTAL AGENCIES HAVING JURISDICTION AT LEAST 24 HOURS PRIOR TO BEGINNING CONSTRUCTION AND PRIOR TO THE INSPECTION OF THE FOLLOWING ITEMS, WHERE APPLICABLE:
 - CLEARING AND FILING
 - STORM DRAINAGE SYSTEM
 - SANITARY SEWER SYSTEM
 - WATER DISTRIBUTION SYSTEM
 - SUBGRADE
 - LIMEROCK BASE
 - WALKWAY CONCRETE
 - SIDEWALK
 - FINAL

4. SHOP DRAWINGS

- PRIOR TO THEIR CONSTRUCTION OR INSTALLATION, SHOP DRAWINGS SHALL BE SUBMITTED TO AND APPROVED BY THE ENGINEER OF RECORD AND CITY OF HOLLYWOOD FOR THE FOLLOWING: SANITARY MANHOLES, STORM DRAIN MANHOLES, CATCH BASINS, FIRE HYDRANTS, PIPING, VALVES AND ALL REQUIRED ACCESSORIES. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO OBTAIN ALL OTHER AGENCY APPROVALS IF REQUIRED.

5. TEMPORARY FACILITIES

- TEMPORARY FACILITIES:
 - IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO ARRANGE FOR OR SUPPLY TEMPORARY WATER SERVICE, SANITARY FACILITIES AND ELECTRICITY.
- TRAFFIC REGULATION:
 - MAINTENANCE OF TRAFFIC IN THE PUBLIC RIGHTS-OF-WAY SHALL BE IN ACCORDANCE WITH MANUAL TRAFFIC CONTROL DEVICES (M.T.C.D.).
 - ALL OPEN TRENCHES AND HOLES ADJACENT TO ROADWAYS OR WALKWAYS SHALL BE PROPERLY MARKED AND BARRICADED TO ASSURE THE SAFETY OF BOTH VEHICULAR AND PEDESTRIAN TRAFFIC.
 - NO TRENCHES OR HOLES NEAR WALKWAYS, IN ROADWAYS OR THEIR SHOULDERS ARE TO BE LEFT OPEN DURING NIGHTTIME HOURS WITHOUT EXPRESS PERMISSION OF CITY OF HOLLYWOOD.
 - IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR FOR ANY NECESSARY CONSTRUCTION, PAVEMENT MARKING AND SIGNAGE OR ANY PEDESTRIAN SIGNALIZATION AND/OR SIGNAL MODIFICATION TO ACCOMMODATE AN ALTERNATE SAFE WALK ROUTE.

6. WATER DISTRIBUTION SYSTEM

- SEPARATION OF WATER AND SEWER MAINS:
 - SANITARY SEWERS, STORM SEWERS, AND FORCE MAINS SHOULD CROSS UNDER WATER MAINS WHENEVER POSSIBLE. SANITARY SEWERS, STORM SEWERS, AND FORCE MAINS CROSSING WATER MAINS SHALL BE LAID TO PROVIDE A MINIMUM VERTICAL DISTANCE OF 12" INCHES BETWEEN THE INVERT OF THE UPPER PIPE AND THE CROWN OF THE LOWER PIPE WHENEVER POSSIBLE.
 - WHERE SANITARY SEWERS, STORM SEWERS, OR FORCE MAINS MUST CROSS A WATER MAIN WITH LESS THAN 12" INCHES VERTICAL DISTANCE, BOTH THE SEWER AND THE WATER MAIN SHALL BE CONSTRUCTED OF DUCTILE IRON PIPE. AT THE CROSSING, SEPARATE LENGTHS OF DIP MUST BE USED TO PROVIDE A MINIMUM SEPARATION OF 10 FEET BETWEEN ANY TWO JOINTS. ALL JOINTS ON THE WATER MAIN WITHIN 20 FEET OF THE CROSSING MUST BE MECHANICALLY RESTRAINED. A MINIMUM VERTICAL CLEARANCE OF 6 INCHES MUST BE MAINTAINED AT ALL CROSSINGS.
 - ALL CROSSINGS SHALL BE ARRANGED SO THAT THE SEWER PIPE JOINTS AND THE WATER MAIN PIPE JOINTS ARE EQUIDISTANT FROM THE POINT OF CROSSING (PIPES CENTERED ON THE CROSSING).
 - WHERE A NEW PIPE CONFLICTS WITH AN EXISTING PIPE WITH LESS THAN

12" INCHES VERTICAL CLEARANCE, THE NEW PIPE SHALL BE CONSTRUCTED OF DIP, AND THE CROSSING SHALL BE ARRANGED TO MEET THE REQUIREMENTS ABOVE.

- MINIMUM 10-FOOT HORIZONTAL SEPARATION SHALL BE MAINTAINED BETWEEN ANY TYPE OF SEWER AND WATER MAIN IN PARALLEL INSTALLATIONS WHENEVER POSSIBLE.
- IN CASES WHERE IT IS NOT POSSIBLE TO MAINTAIN A 10 FOOT HORIZONTAL SEPARATION, THE WATER MAIN MUST BE AID IN A SEPARATE TRAP, OR AN UNARMORED CAST IRON SHELL LOCATED ON ONE SIDE OF THE SEWER OR FORCE MAIN AT SUCH AN ELEVATION THAT THE BOTTOM OF THE WATER IS AT LEAST 18 INCHES ABOVE THE TOP OF THE SEWER.

- WHERE IT IS NOT POSSIBLE TO MAINTAIN A VERTICAL DISTANCE OF 12" INCHES IN PARALLEL INSTALLATIONS, THE WATER MAIN SHALL BE CONSTRUCTED OF DIP AND THE SANITARY SEWER OR THE FORCE MAIN SHALL BE CONSTRUCTED OF DIP WITH A MINIMUM VERTICAL DISTANCE OF 6 INCHES. THE WATER MAIN SHOULD ALWAYS BE ABOVE THE SEWER. JOINTS ON THE WATER MAIN SHALL BE LOCATED AS FAR APART AS POSSIBLE FROM JOINTS ON THE SEWER OR FORCE MAIN (STAGGERED JOINTS).
- CONTRACTOR SHALL MAINTAIN WATER SERVICE TO ALL EXISTING FACILITIES DURING CONSTRUCTION.

6.2. MATERIALS:

- POLYVINYL CHLORIDE (PVC) PIPE SHALL BE DR 18 ANSI / AWWA C900-97 OR LATEST REVISION.

- ALL PIPE LARGER THAN 12" DIAMETER MUST BE DUCTILE IRON (MIN. CLASS 50) 9" AND 10" DIP (MAX. CLASS 50) 4" AND 6" DIP (MIN. CLASS 52). ALL DUCTILE IRON PIPE SHALL CONFORM TO THE REQUIREMENTS OF ANSI/AWWA C151/A21.51-96 AND CEMENT MORTAR LINED AND SEAL COATED PER ANSI/AWWA C104/A21.4-95.

- FITTINGS SHALL BE DUCTILE IRON, MEETING ANSI/AWWA C-153-00 SPECIFICATIONS. FITTINGS MUST BE CEMENT LINED AND SEAL COATED PER ANSI/AWWA C104/A21.4-95.

- VALVES SHALL BE GATE VALVES, IRON BODY, FULLY RESILIENT SEAT BRONZED MOUNTED NON-RISING STEM, RATED AT 200 PSI AND CONFORMING TO ANSI/AWWA C509-94 OR LATEST REVISION, AND SHALL HAVE MECHANICAL JOINTS.

- ALL PIPE LARGER THAN 12" DIAMETER MUST BE DUCTILE IRON (MIN. CLASS 50) 9" AND 10" DIP (MAX. CLASS 50) 4" AND 6" DIP (MIN. CLASS 52). ALL DUCTILE IRON PIPE SHALL CONFORM TO THE REQUIREMENTS OF ANSI/AWWA C151/A21.51-96 AND CEMENT MORTAR LINED AND SEAL COATED PER ANSI/AWWA C104/A21.4-95.

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- VALVES SHALL BE GATE VALVES, IRON BODY, FULLY RESILIENT SEAT BRONZED MOUNTED NON-RISING STEM, RATED AT 200 PSI AND CONFORMING TO ANSI/AWWA C509-94 OR LATEST REVISION, AND SHALL HAVE MECHANICAL JOINTS.

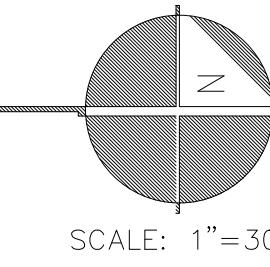
- ALL PIPE LARGER THAN 12" DIAMETER MUST BE DUCTILE IRON (MIN. CLASS 50) 9" AND 10" DIP (MAX. CLASS 50) 4" AND 6" DIP (MIN. CLASS 52). ALL DUCTILE IRON PIPE SHALL CONFORM TO THE REQUIREMENTS OF ANSI/AWWA C151/A21.51-96 AND CEMENT MORTAR LINED AND SEAL COATED PER ANSI/AWWA C104/A21.4-95.

- FITTINGS SHALL BE DUCTILE IRON, MEETING ANSI/AWWA C-153-00 SPECIFICATIONS. FITTINGS MUST BE CEMENT LINED AND SEAL COATED PER ANSI/AWWA C104/A21.4-95.

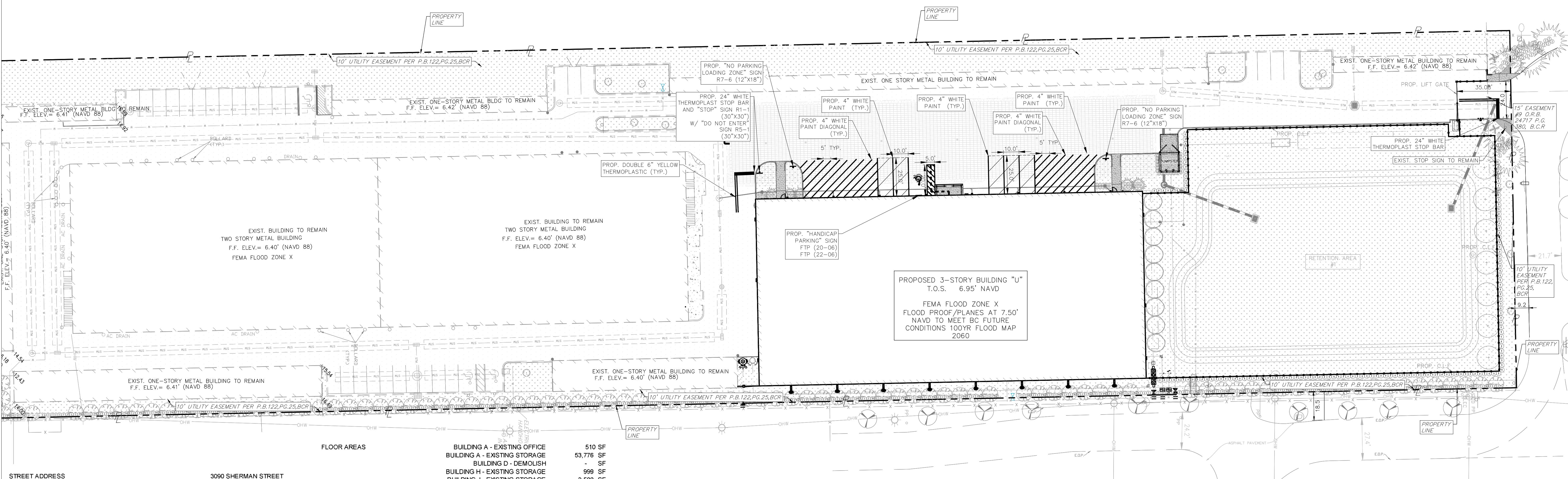
- VALVES SHALL BE GATE VALVES, IRON BODY, FULLY RESILIENT SEAT BRONZED MOUNTED NON-RISING STEM, RATED AT 200 PSI AND CONFORMING TO ANSI/AWWA C509-94 OR LATEST REVISION, AND SHALL HAVE MECHANICAL JOINTS.

- ALL PIPE LARGER THAN 12" DIAMETER MUST BE DUCTILE IRON (MIN. CLASS 50) 9" AND 10" DIP (MAX. CLASS 50) 4" AND 6" DIP (MIN. CLASS 52). ALL DUCTILE IRON PIPE SHALL CONFORM TO THE REQUIREMENTS OF ANSI/AWWA C151/A21.51-96 AND CEMENT MORTAR LINED AND SEAL COATED PER ANSI/AWWA C104/A21.4-95.

- FITTINGS SHALL BE DUCTILE IRON, MEETING ANSI/AWWA C-153-00 SPECIFICATIONS. FITTINGS MUST BE CEMENT LINED AND SEAL COATED PER ANSI/AWWA C104/A21.4-95.



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Check positive response codes before you dig!



STREET ADDRESS
CITY
JURISDICTION
PARCEL ID#
EXISTING ZONING
PROPOSED ZONING
EXISTING LAND USE
PROPOSED LAND USE
FUTURE LAND USE

3090 SHERMAN STREET
HOLLYWOOD, FLORIDA
CITY OF HOLLYWOOD
514208160010
IM-1
IM-1
SELF-STORAGE FACILITY
SELF-STORAGE FACILITY
INDUSTRIAL (IND)

FLOOD ZONE
WETLANDS
EASEMENTS

FEMA FLOOD ZONE X
SEE SURVEY
SEE SURVEY

SETBACKS

FRONT OR STREET REQUIRED (NORTH)
PROPOSED (NORTH) 15 FT

SIDE REQUIRED (EAST) 0 FT
PROPOSED (EAST) 11 FT

SIDE REQUIRED (WEST) 20 FT
PROPOSED (WEST) 99 FT

REAR REQUIRED (SOUTH)
PROPOSED (SOUTH) 0 FT

PROPOSED (SOUTH) 494 FT

EXISTING LOT AREA

NORTH 92,490 SF
SOUTH 228,712 SF
TOTAL 321,202 SF

MAXIMUM FLOOR AREA RATIO
7.37 ACRES

FLOOR AREAS

BUILDING A - EXISTING OFFICE	510 SF
BUILDING A - EXISTING STORAGE	53,776 SF
BUILDING D - DEMOLISH	- SF
BUILDING H - EXISTING STORAGE	999 SF
BUILDING J - EXISTING STORAGE	3,502 SF
BUILDING K - EXISTING STORAGE	1,402 SF
BUILDING L 1st FLOOR - EXISTING STORAGE	22,027 SF
BUILDING L 2nd FLOOR - EXISTING STORAGE	22,027 SF
BUILDING M 1st FLOOR - EXISTING STORAGE	22,059 SF
BUILDING M 2nd FLOOR - EXISTING STORAGE	22,059 SF
BUILDING N - EXISTING STORAGE	803 SF
BUILDING O - EXISTING STORAGE	1,347 SF
BUILDING P - EXISTING STORAGE	3,996 SF
BUILDING Q - EXISTING STORAGE	2,405 SF
BUILDING R - DEMOLISH	- SF
BUILDING T - DEMOLISH	- SF
BUILDING U 1st FLOOR - PROPOSED STORAGE	30,495 SF
BUILDING U 2nd FLOOR - PROPOSED STORAGE	30,495 SF
BUILDING U 3rd FLOOR - PROPOSED STORAGE	30,495 SF

MAXIMUM BUILDING HEIGHT

	ALLOWED	35 FT
PROPOSED	34 FT	
ALLOWED	3 STORIES	
PROPOSED	3 STORIES	

PARKING REQUIRED

OFFICE 1 PER 250 SF	3.00
SELF-STORAGE FACILITY 1 PER 10,000 SF	25.00
TOTAL REQUIRED	28.00

PARKING PROVIDED

EXISTING ACCESSIBLE PROVIDED	5.00
NEW ACCESSIBLE PROVIDED	1.00
EXISTING STANDARD PROVIDED	44.00
NEW STANDARD PROVIDED	0.00
TOTAL PROVIDED	50.00

LOADING REQUIRED

ALLOWED	289,082 SF
PROPOSED	142,811 SF

LOADING PROVIDED

ALLOWED	3.00
PROPOSED	963,606 SF

MAXIMUM LOT COVERAGE	0.90
ALLOWED	289,082 SF

PROPOSED	142,811 SF
----------	------------

LOADING REQUIRED	5
LOADING PROVIDED	5

100% CONSTRUCTION DOCUMENTS

ZT	4/14/23	▲	3/27/23	CITY OF HOLLYWOOD - TAC SUBMITTAL COMMENTS
DESIGNED BY	DATE			
ZT	4/14/23			
DRAWN BY	DATE			
RR	4/14/23			
CHECKED BY	DATE			
RR	4/14/23			
APPROVED BY	DATE	No.	DATE	REVISIONS

SAFEGUARD STORAGE
3090 SHERMAN STREET
HOLLYWOOD, FL 33021

RE
ROSS ENGINEERING, INC.

3325 S. UNIVERSITY DRIVE, SUITE 111
DAVIE, FLORIDA 33328
(954)318-0624 (954)358-0190 FAX
CERTIFICATE OF AUTHORIZATION No. 9808

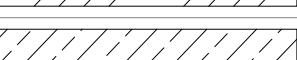
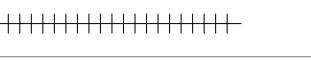


CIVIL SITE PLAN

SCALE: 1"=30'

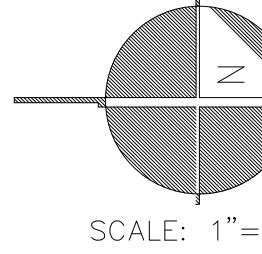
SHEET No. C-1.1

LEGEND

LEGEND	
— - - - -	PROPERTY LINE
	
— STM —	EXISTING DRAINAGE
	
— W —	EXISTING WATER
	
	EXISTING UTILITIES TO BE REMOVED
	
	EXISTING CONCRETE TO BE REMOVED
	EXISTING SODD TO BE REMOVED
	
	EXISTING SODD TO REMAIN

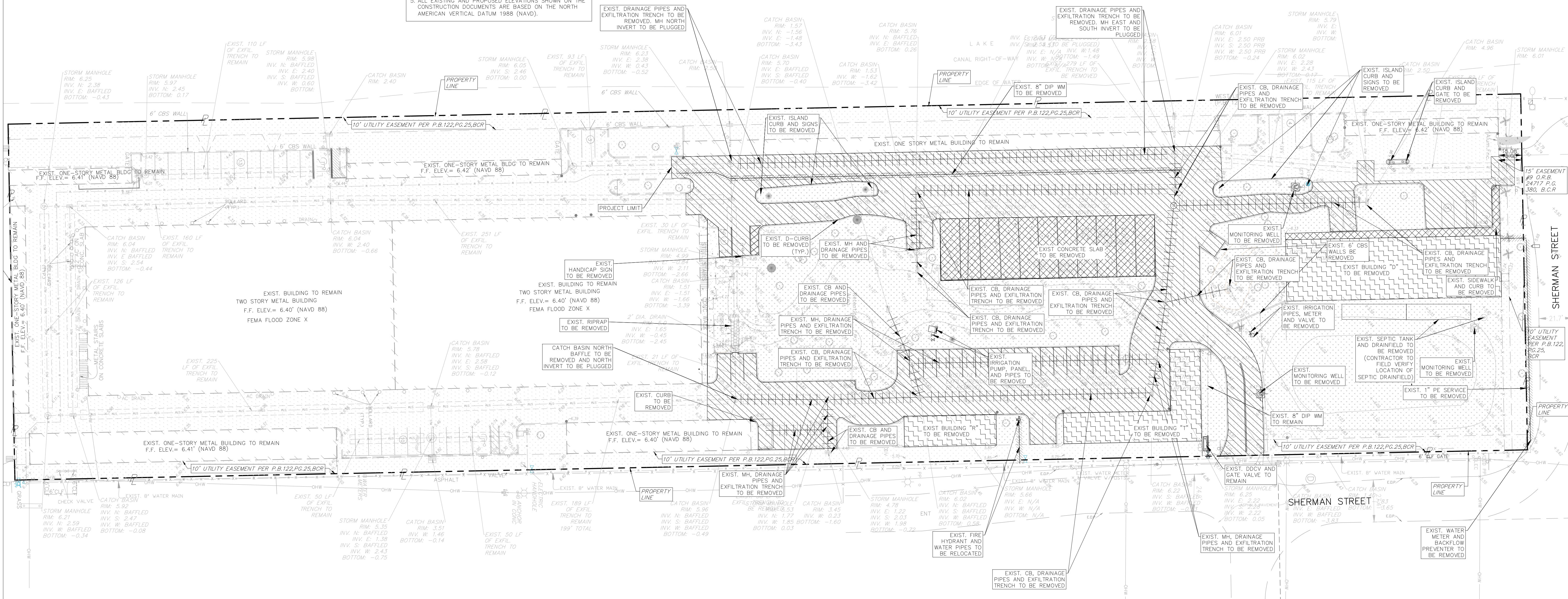
NO

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2. CONTRACTOR TO FIELD VERIFY ANY CONFLICTS WITH TREES AND/OR UTILITIES AND DRAINAGE. CONTRACTOR TO NOTIFY ENGINEER OF ANY CONFLICTS BEFORE PROCEEDING WITH ANY SOLUTION TO THE CONFLICT.
3. CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO SAFEGUARD ALL EXISTING STRUCTURES AND UTILITIES, ANY DAMAGE DONE TO EXISTING UTILITIES SHALL BE REPAIRED BY THE CONTRACTOR AT NOT EXPENSE TO THE SUBJECT UTILITY. CALL "SUNSHINE" 48 HOURS BEFORE DIGGING.
4. CONTRACTOR IS TO RESTORE ANY CURB, LANDSCAPE, ASPHALT, ETC. (NOT SCHEDULED FOR DEMOLITION) DAMAGED DURING CONSTRUCTION TO A CONDITION EQUAL TO WHAT IS EXISTING.
5. ALL EXISTING AND PROPOSED ELEVATIONS SHOWN ON THE CONSTRUCTION DOCUMENTS ARE BASED ON THE NORTH AMERICAN VERTICAL DATUM 1988 (NAVD).



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SCALE: 1" =



100% CONSTRUCTION DOCUMENTS

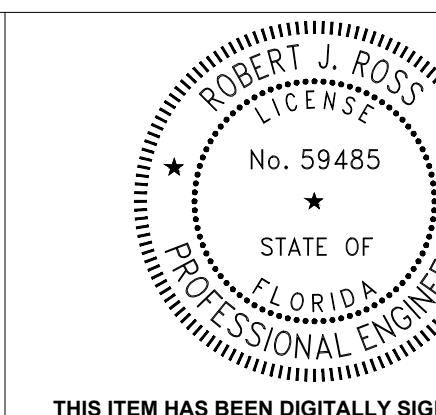
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DESIGNED BY	DATE			
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RR	4/14/23			
APPROVED BY	DATE	No.	DATE	REVISIONS

SAFEGUARD STORAGE

3090 SHERMAN STREET
HOLLYWOOD, FL 33021



3325 S. UNIVERSITY DRIVE, SUITE 111
DAVIE, FLORIDA 33328
(954)318-0624 (954)358-0190 FAX
CERTIFICATE OF AUTHORIZATION No. 9808



Date: 1/1/2020

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DEMOLITION PLAN

SCALE: 1" = 1

SHEET No. C-2

LEGEND

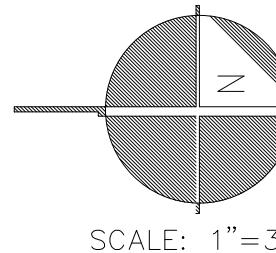
	PROPERTY LINE
	EXISTING BUILDING
	EXISTING BUILDING
	EXISTING OVERHEAD LINE
	EXISTING WATER LINE
	EXISTING STORM SEWER LINE
	EXISTING STORM SEWER LINE W/ EXFILTRATION TRENCH TO REMAIN
	EXISTING CHAIN LINK FENCE

5.50
5.00
MEG
(5.00)
~

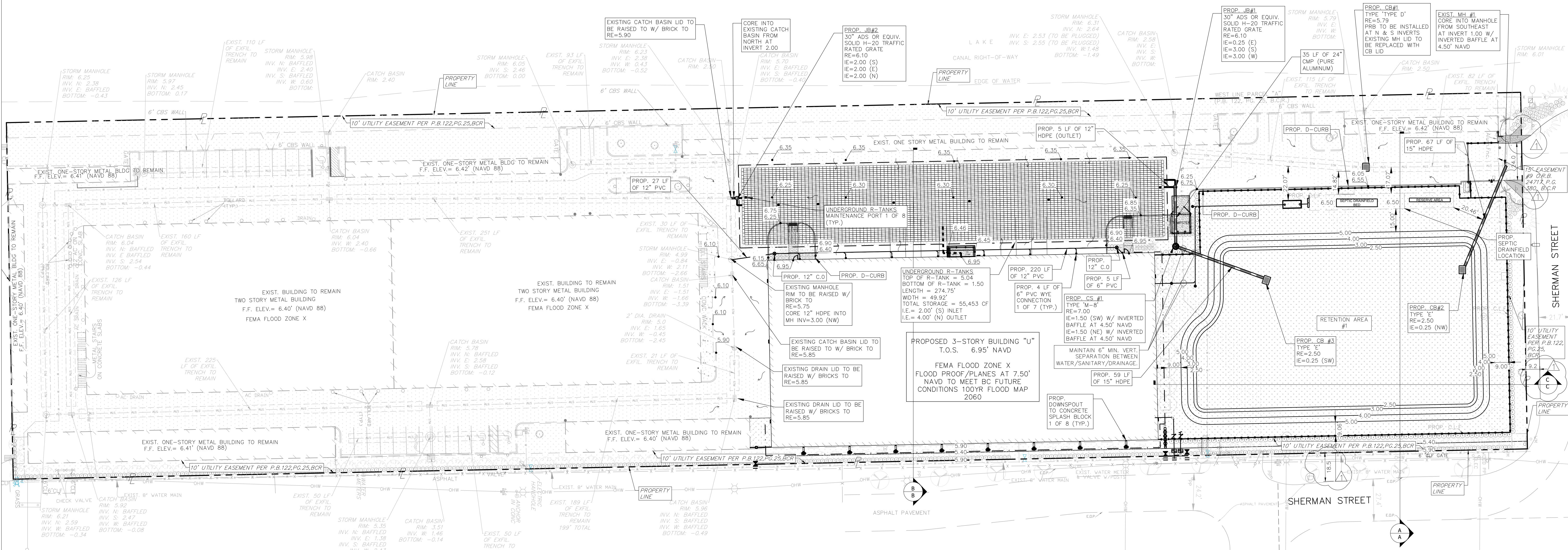


PROPOSED CONTOUR LINE
PROPOSED GRADE
MATCH EXISTING GRADE (MEG)
FLOW ARROW
PROPOSED CATCH BASIN (CB)
PROPOSED DRAINAGE MANHOLE (MH)

PROPOSED DRAINAGE PIPE
PROPOSED DRAINAGE PIPE W/ EXFILTRATION TRENCH
EXISTING SOD TO REMAIN
PROPOSED SOD



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6. ANY LIP FROM $\frac{1}{8}$ " BUT NOT GREATER THAN $\frac{1}{2}$ " WILL BE BEVELED TO MEET ADA REQUIREMENTS

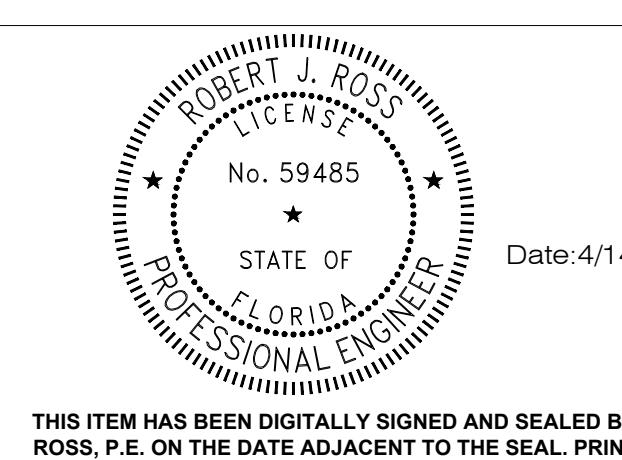
00% CONSTRUCTION DOCUMENTS

ZT	4/14/23	 1	3/27/23	CITY OF HOLLYWOOD – TAC SUBMITTAL COMMENTS
DESIGNED BY DATE				
ZT	4/14/23			
DRAWN BY	DATE			
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CHECKED BY	DATE			
RR	4/14/23			
APPROVED BY	DATE	No.	DATE	REVISIONS

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HOLLYWOOD, FL 33021



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CERTIFICATE OF AUTHORIZATION No. 9808



4/2023

1,233

1

1

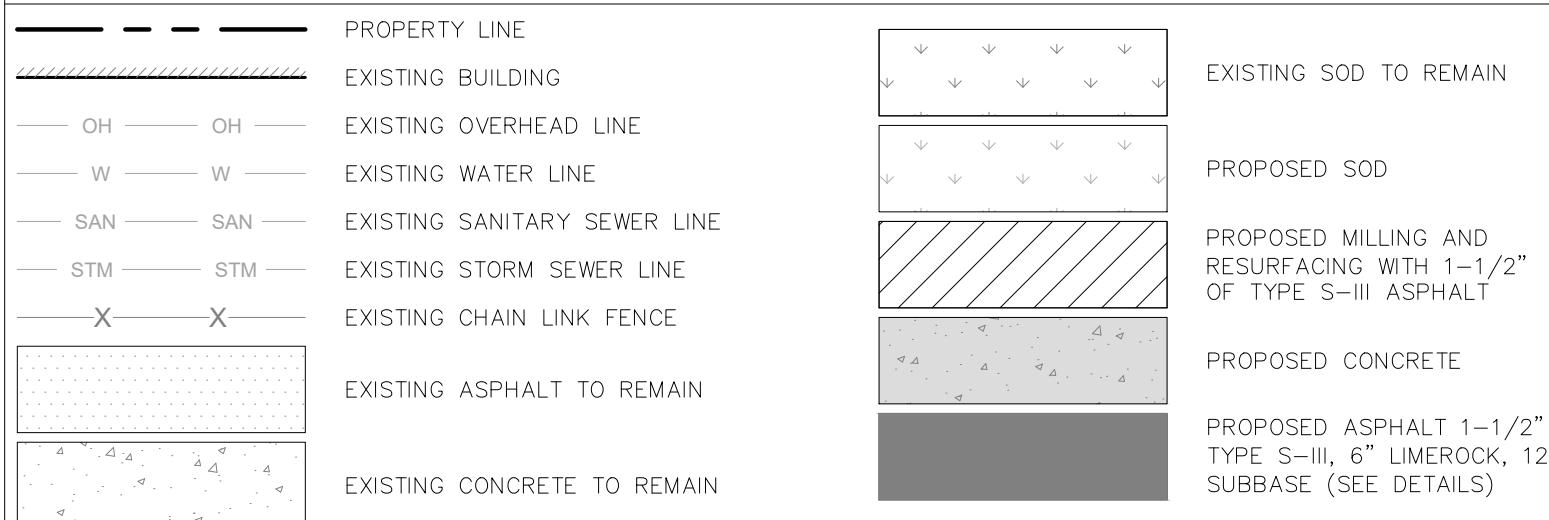
Y ROBERT J.
ITED COPIES

GRADING AND DRAINAGE PLAN

SCALE: 1"=30'

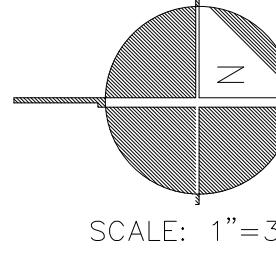
SHEET No. C-3

LEGEND

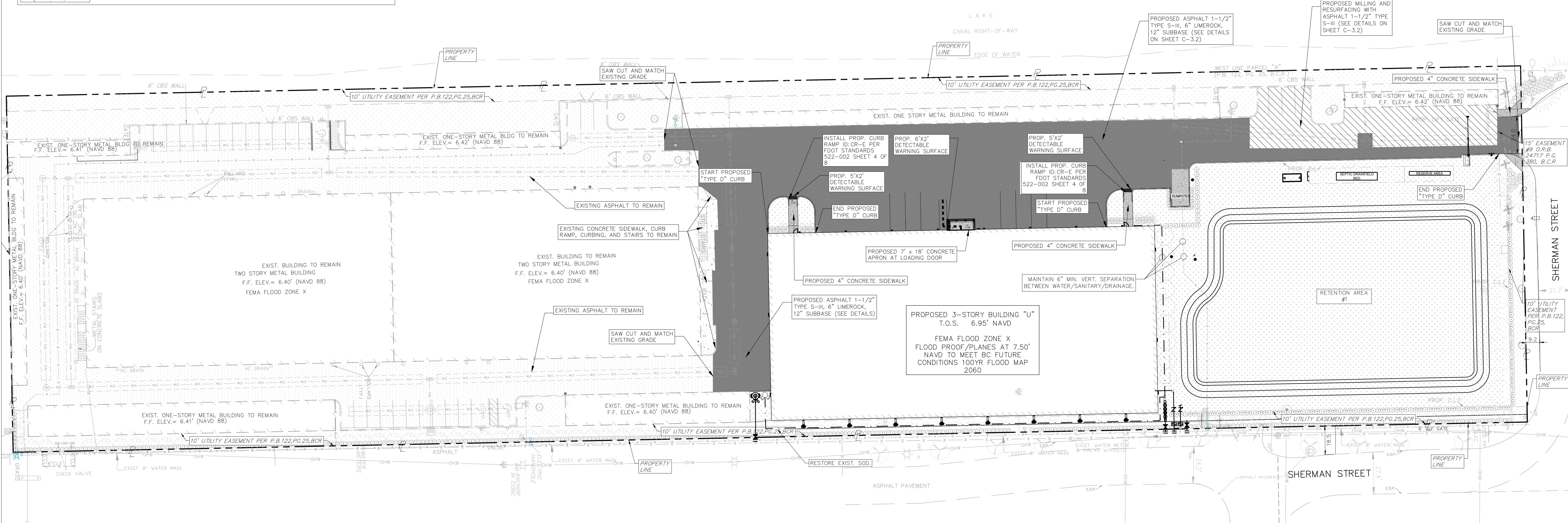


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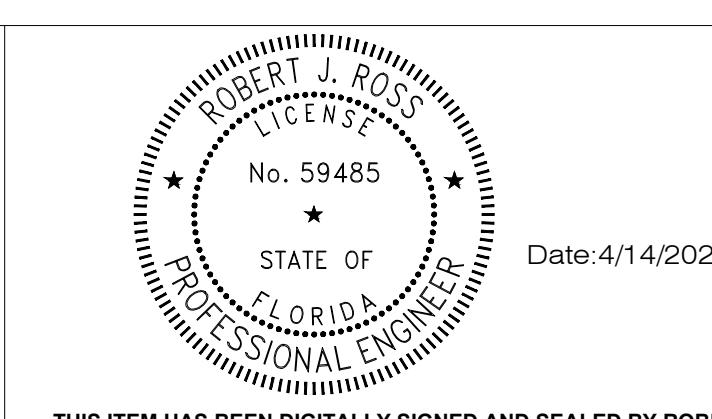
100% CONSTRUCTION DOCUMENTS

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DESIGNED BY	DATE			
ZT	4/14/23			
DRAWN BY	DATE			
RR	4/14/23			
CHECKED BY	DATE			
RR	4/14/23			
APPROVED BY	DATE	No.	DATE	REVISIONS

SAFEGUARD STORAGE
3090 SHERMAN STREET
HOLLYWOOD, FL 33021



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CERTIFICATE OF AUTHORIZATION No. 9808



23

ERT J.

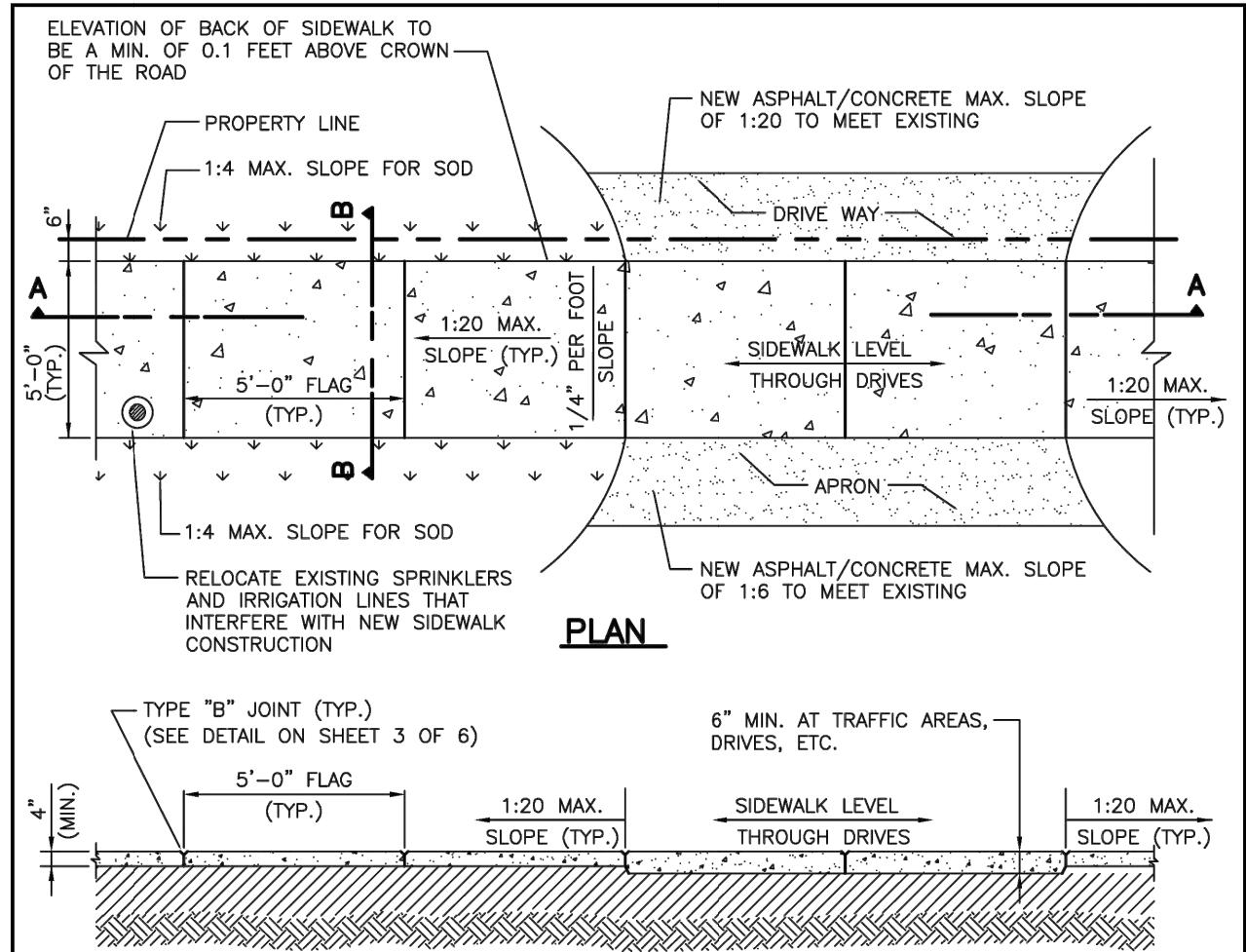
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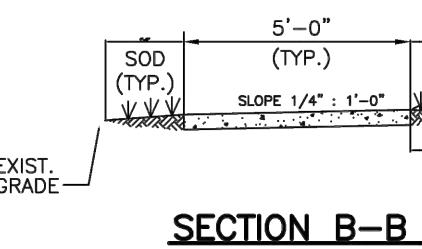
PAVING SIDEWALK AND CURBING PLAN

SCALE: 1" = 30'

SHEET No. C-3.1

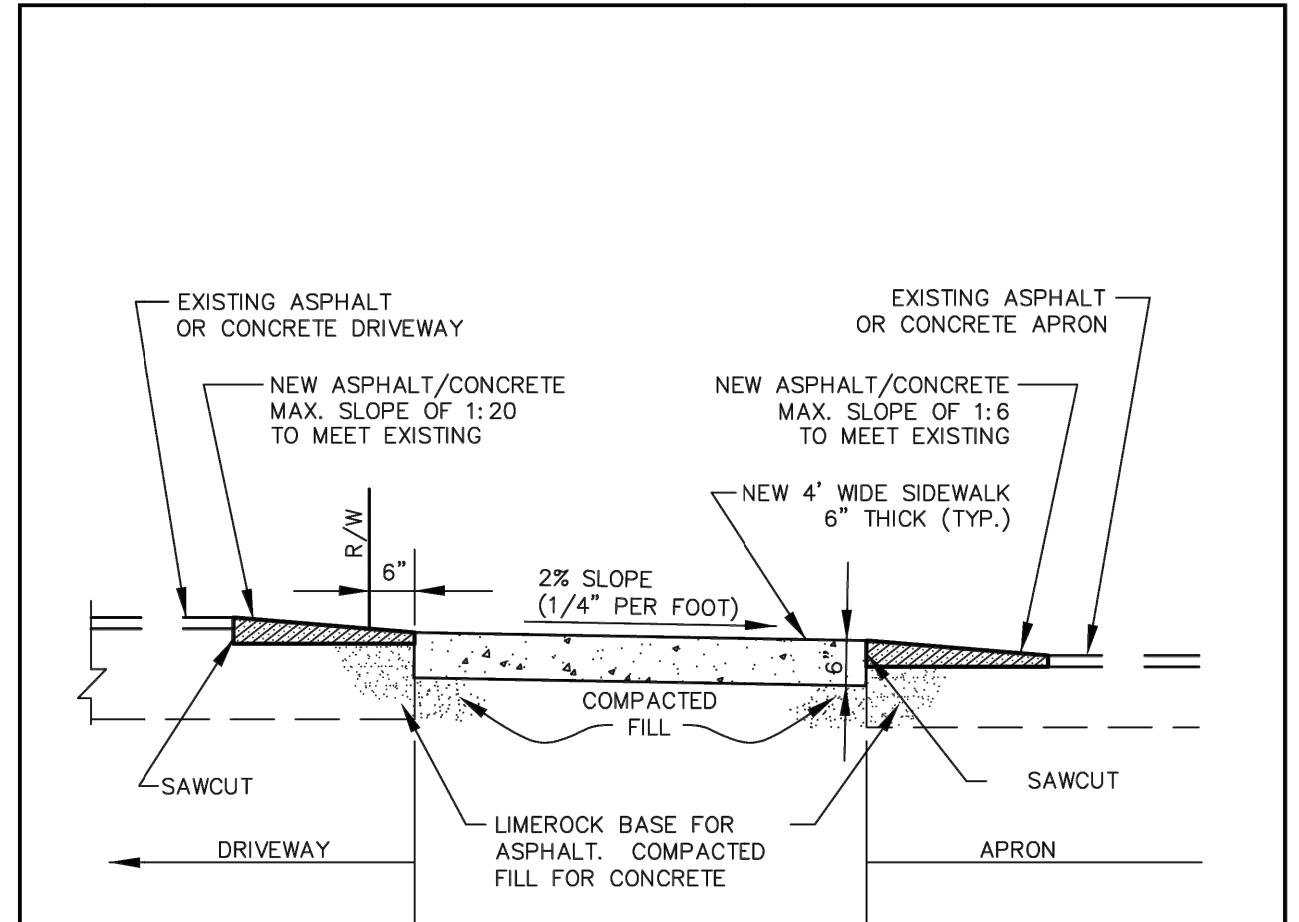


SECTION A-A

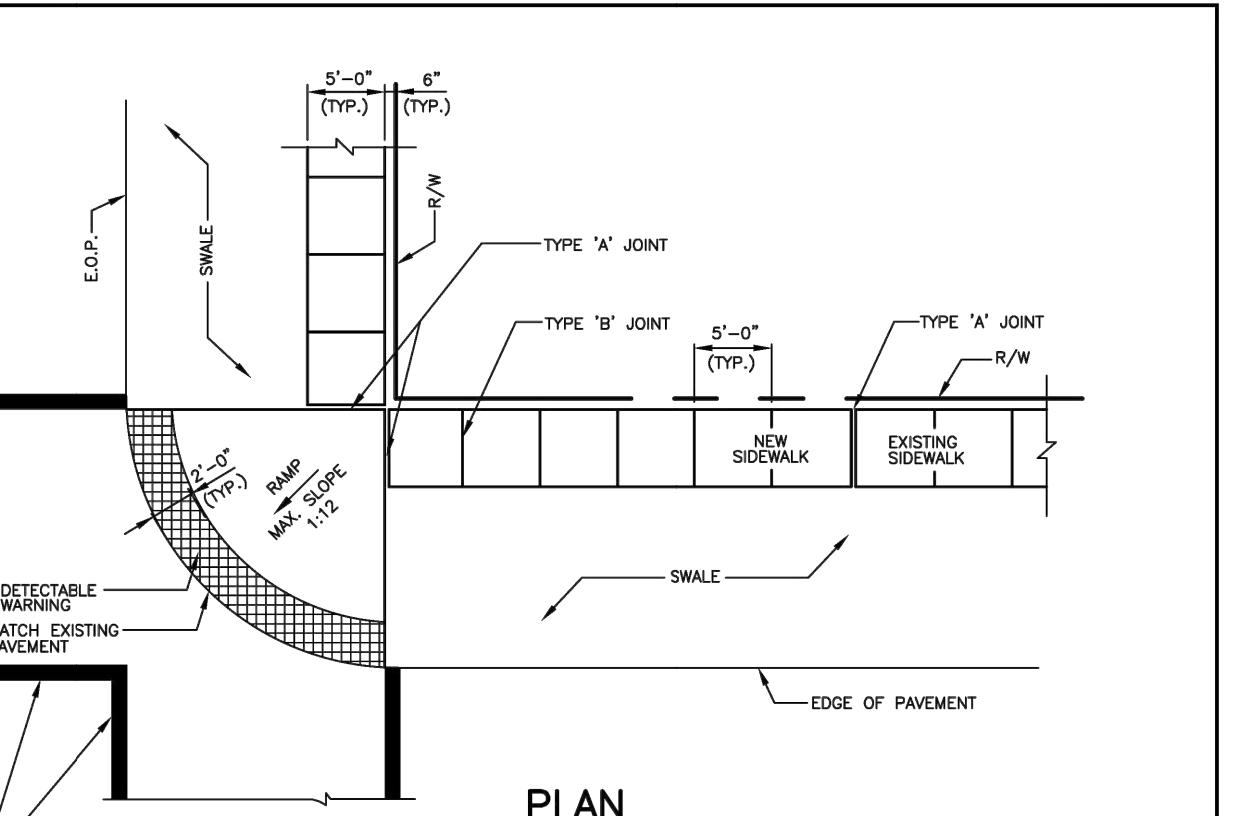


NOTE:
ALL SIDEWALK CONSTRUCTION SHALL BE IN STRICT ACCORDANCE WITH THE CONTRACT SPECIFICATIONS.
LIGHT BROWN FINISH PERPENDICULAR TO THE DIRECTION OF THE SIDEWALK.

CITY OF HOLLYWOOD, FLORIDA DEPARTMENT OF DEVELOPMENT SERVICES ENGINEERING, TRANSPORTATION AND MOBILITY DIVISION				DESIGNED BY J.V.	SCALE	N.T.S.	APPROVED BY
STANDARD DETAIL SIDEWALK CONSTRUCTION DETAIL				DRAWN BY R.A.Q.	PROJECT No.	CITY ENGINEER	
				CHECKED BY	ACCT. No.	SHEET No.	
				DATE	05/20/2010	CAD/DWG No.	OF 4 SHEETS

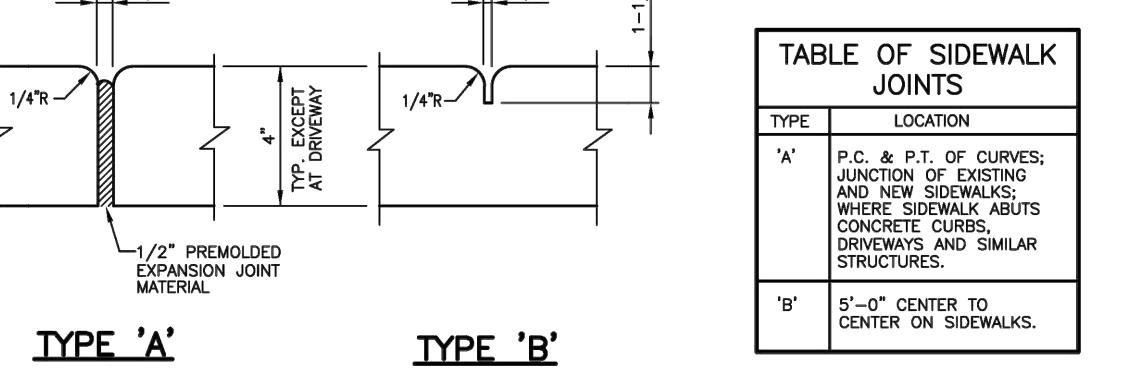


SECTIONAL VIEW AT DRIVES



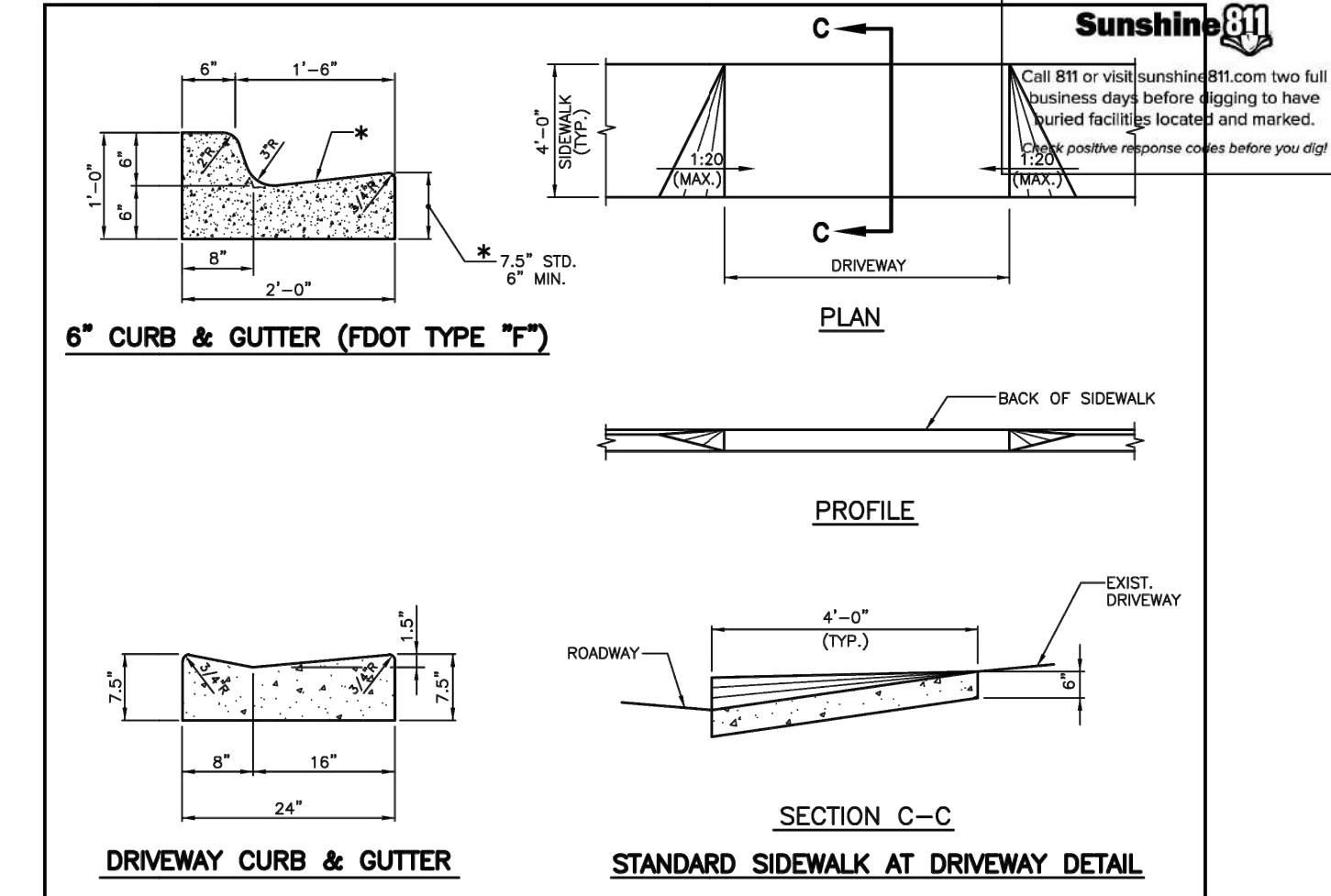
PLAN

NOTE 1:
IF THERE IS AN EXISTING CROSSWALK AT THE INTERSECTION, THE ADA RAMP MUST BE LOCATED WITHIN THE CROSSWALK.

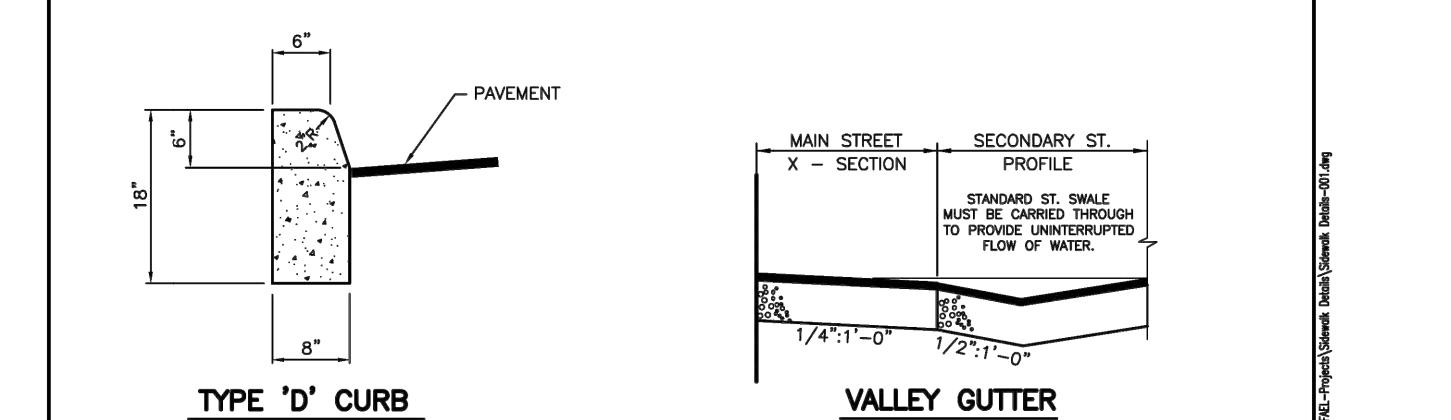


SIDEWALK JOINTS

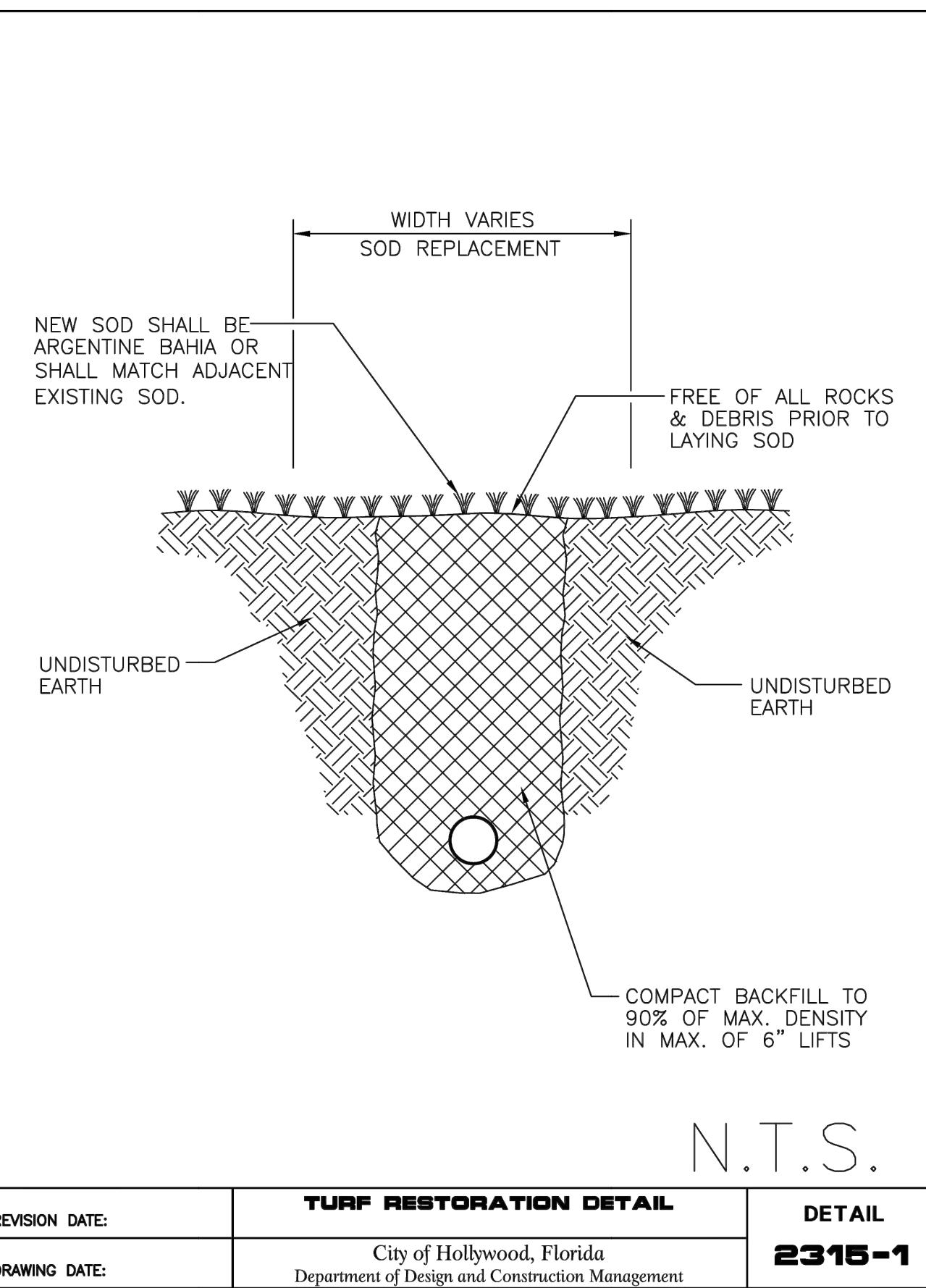
CITY OF HOLLYWOOD, FLORIDA DEPARTMENT OF DEVELOPMENT SERVICES ENGINEERING, TRANSPORTATION AND MOBILITY DIVISION				DESIGNED BY J.V.	SCALE	N.T.S.	APPROVED BY
STANDARD DETAIL SIDEWALK CONSTRUCTION DETAIL				DRAWN BY R.A.Q.	PROJECT No.	CITY ENGINEER	
				CHECKED BY	ACCT. No.	SHEET No.	
				DATE	05/20/2010	CAD/DWG No.	OF 4 SHEETS



STANDARD SIDEWALK AT DRIVEWAY DETAIL

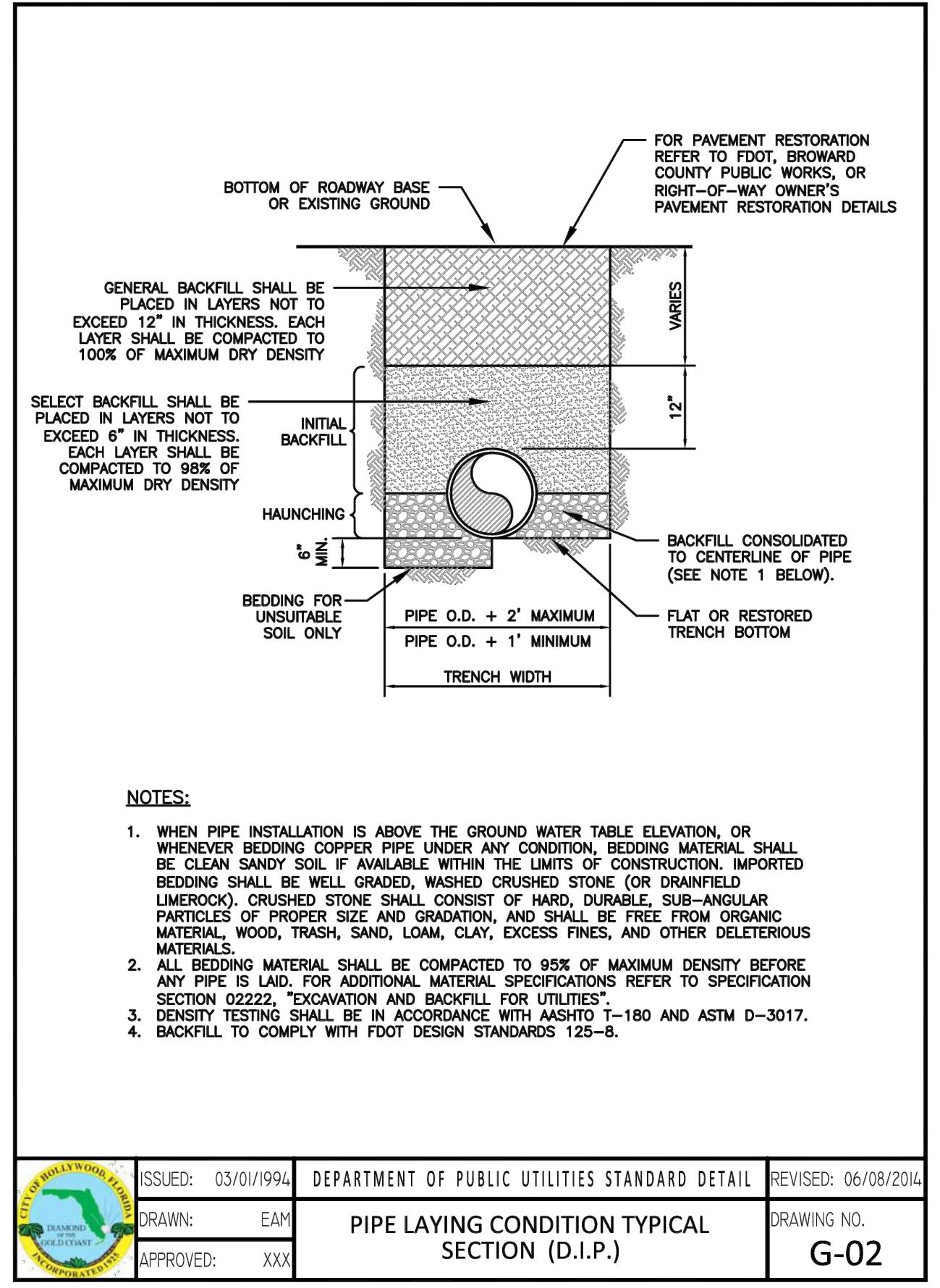


VALLEY GUTTER

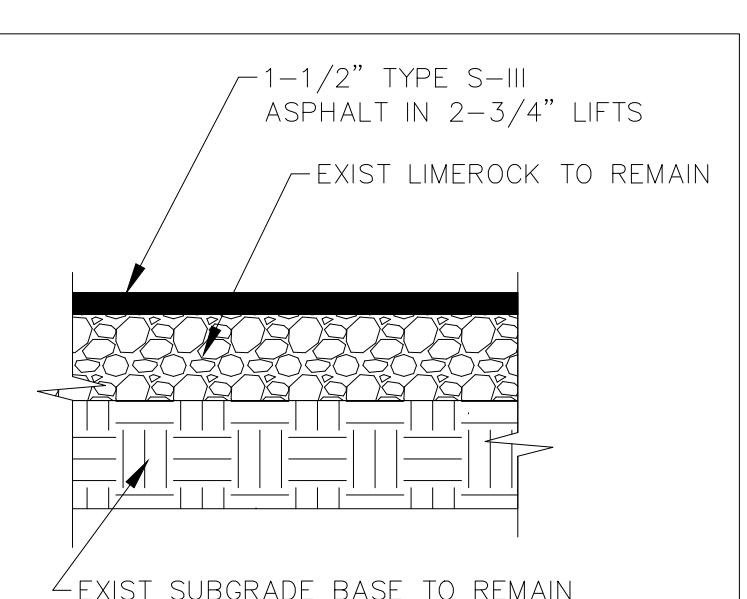
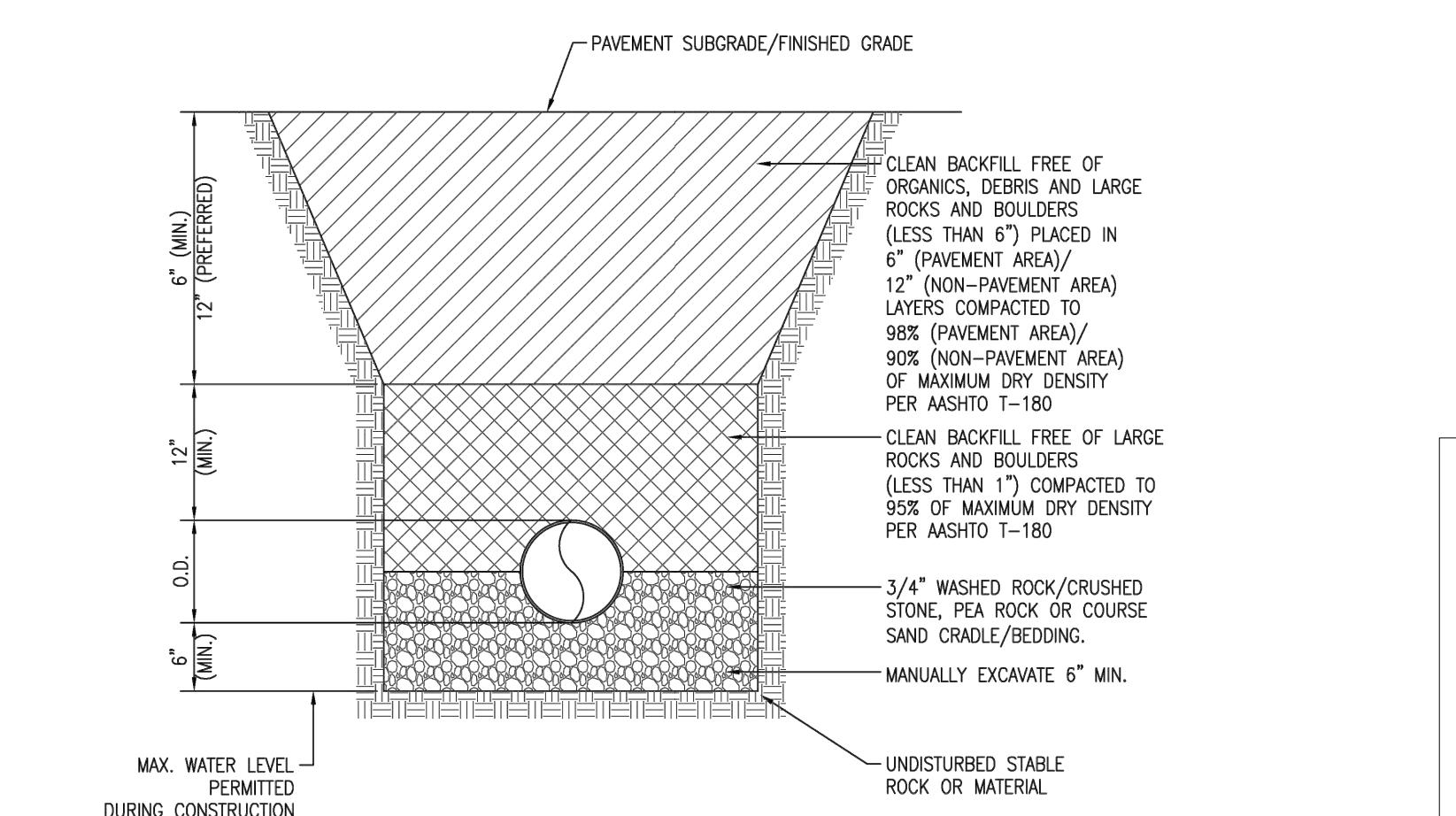


N.T.S.

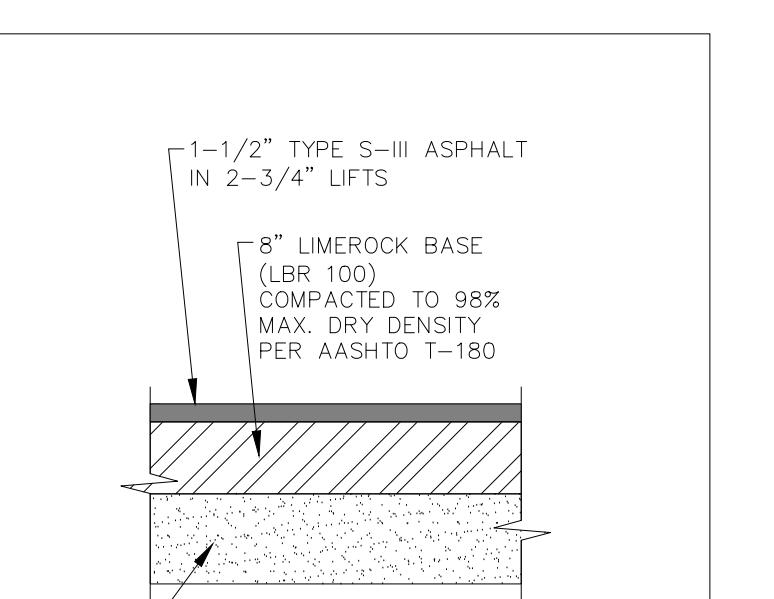
REVISION DATE:	TURF RESTORATION DETAIL		DETAIL
DRAWING DATE:	City of Hollywood, Florida Department of Design and Construction Management		2315-1



	ISSUED: 03/01/1994	DEPARTMENT OF PUBLIC UTILITIES STANDARD DETAIL	REVISED: 06/08/2014
DRAWN: EAM	PIPE LAYING CONDITION TYPICAL SECTION (D.I.P.)		DRAWING NO. G-02
APPROVED: XXX			



MILL & RESURFACE
PAVEMENT SECTION
N.T.S.



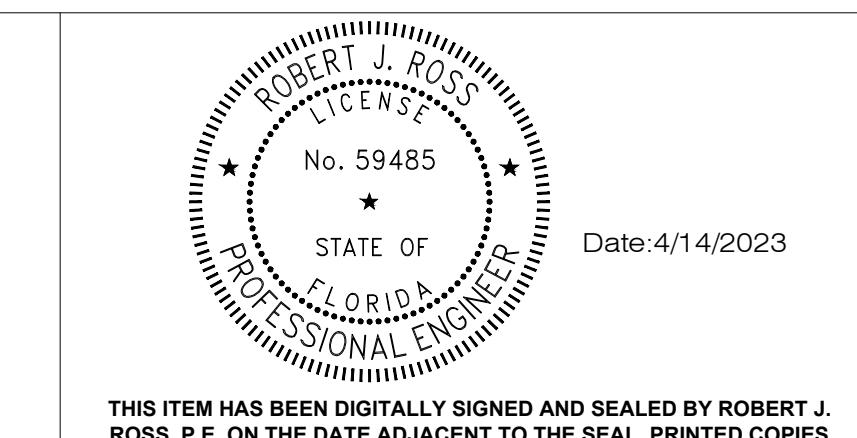
TYPICAL ASPHALT
PAVEMENT SECTION
N.T.S.

ZT	4/14/23		3/27/23	CITY OF HOLLYWOOD - TAC SUBMITTAL COMMENTS		
DESIGNED BY	DATE					
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APPROVED BY	DATE	REVISIONS	DATE	REVISIONS	DATE	REVISIONS

SAFEGUARD STORAGE
3090 SHERMAN STREET
HOLLYWOOD, FL 33021



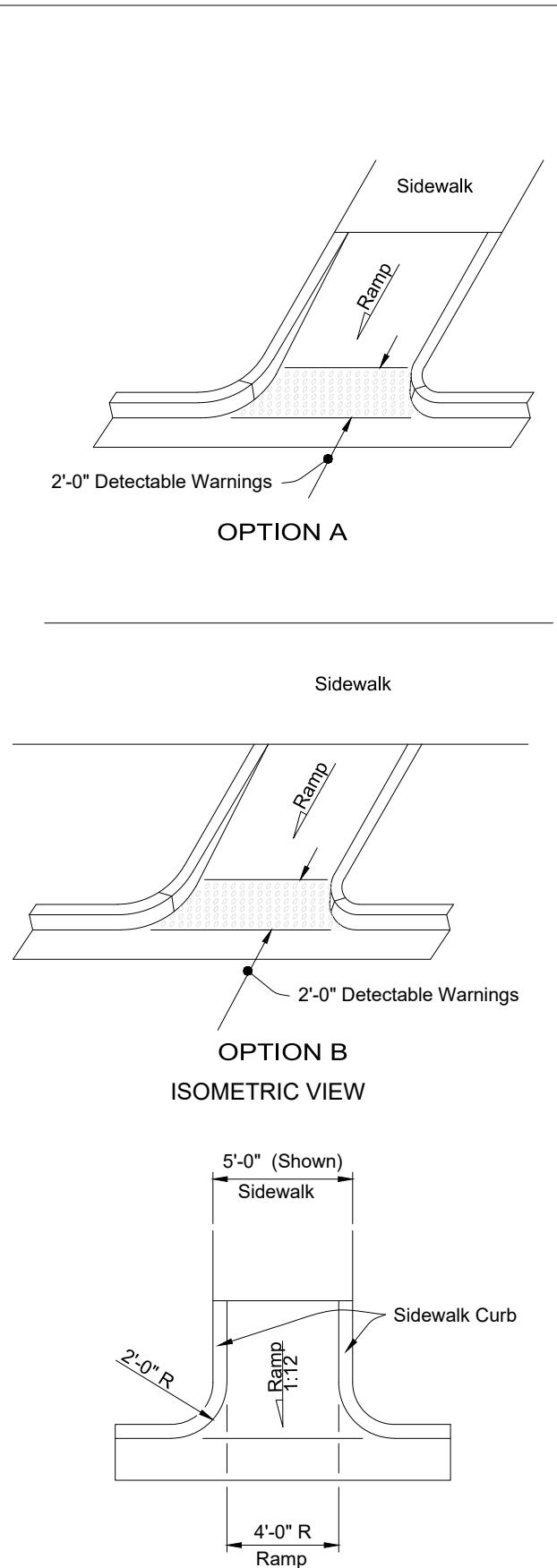
3325 S. UNIVERSITY DRIVE, SUITE 111
DAVIE, FLORIDA 33328
(954)318-0624 (954)358-0190 FAX
CERTIFICATE OF AUTHORIZATION No. 9808



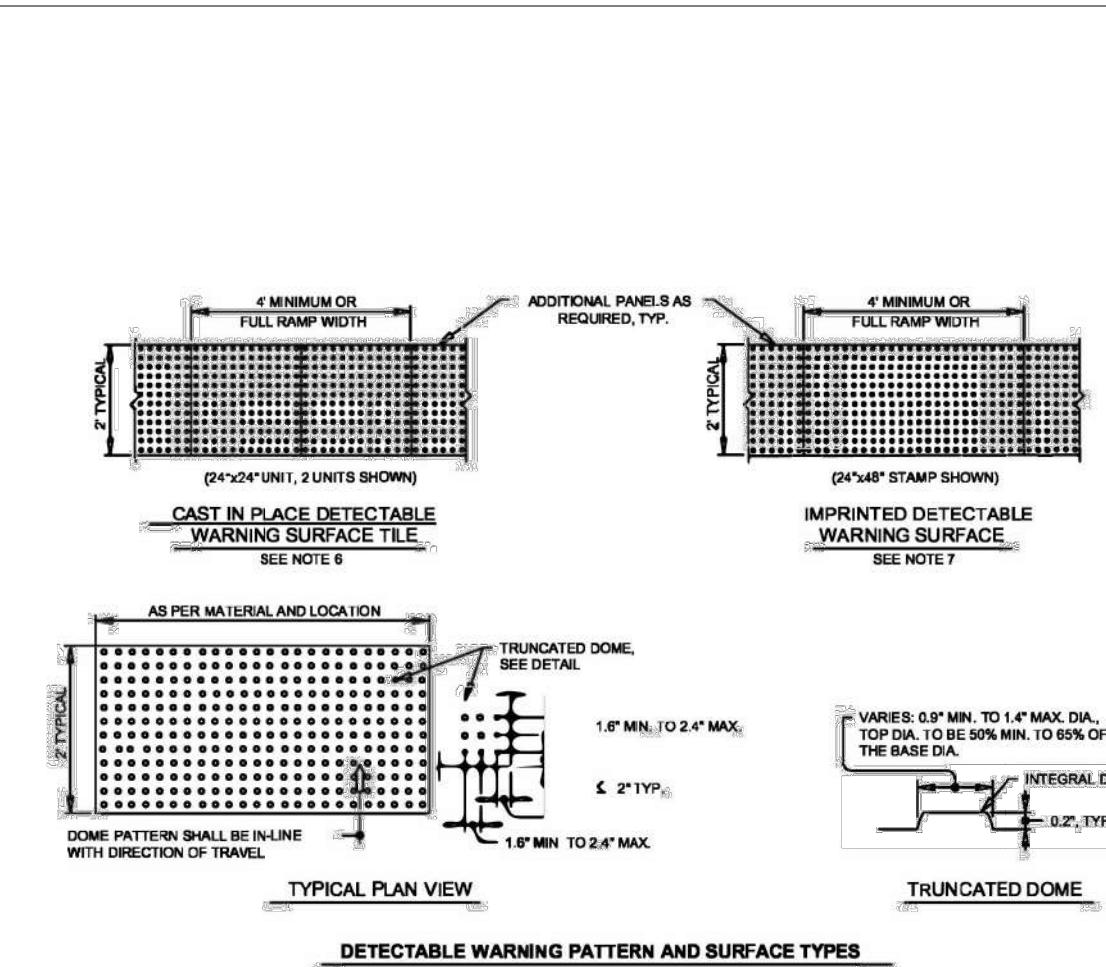
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SCALE: N.T.S.	SHEET No. C-3.2
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PAVING GRADING AND DRAINAGE DETAILS
(SHEET 1 OF 3)

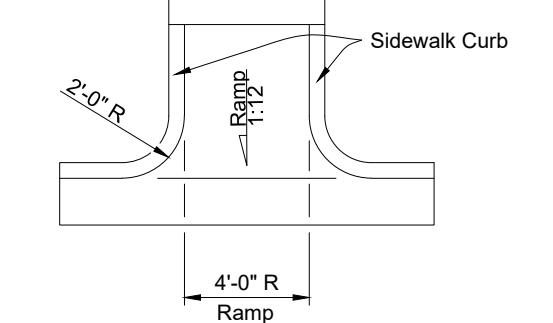


OPTION A

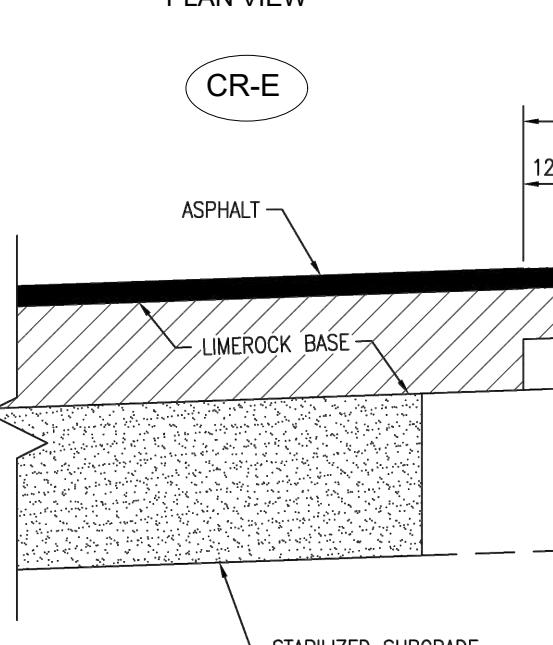


NOTES:
 1. CURB RAMP DETECTABLE WARNING SURFACES SHALL EXTEND THE FULL WIDTH OF THE RAMP. DETECTABLE WARNING SURFACES SHALL CONFORM TO THE TRUNCATED DOME PATTERN IN CONFORMANCE WITH THE U.S. DEPARTMENT OF ADA STANDARDS FOR ACCESSIBLE DESIGN, A.D.A. ACCESSIBILITY GUIDELINES, SECTION 4.2.9. SEE DETAIL ABOVE. THE SLOPES ARE NOT TO HAVE DETECTABLE WARNINGS.
 2. DETECTABLE WARNING SURFACES SHALL BE ADJACENT TO ADJACENT SURFACES, EITHER LIGHT OR DARK.
 3. DETECTABLE WARNING PAVERS SHALL CONFORM TO ASTM C 902 'PEDESTRIAN AND LIGHT TRAFFIC PAVING BRICK'.
 4. DETECTABLE WARNING PAVERS SHALL BE 1" SAND BED COMPACTED, OVER NATIVE SOIL COMPACTED TO 98% OF MAXIMUM DENSITY.
 5. DETECTABLE WARNING PAVERS SHALL BE AS MANUFACTURED BY PINE HALL BRICK OR EQUAL.
 6. CEMENT PAVERS SHALL BE AS MANUFACTURED BY PINE HALL BRICK OR EQUAL.
 7. IMPRINTED DETECTABLE WARNING SURFACE TILE IN LINE).

8. CONCRETE USED WITH THE STAMP METHOD SHALL HAVE AGGREGATE NO LARGER THAN 1/2".
 9. ALL MATERIALS SHALL BE INSTALLED AS PER THESE CONDITIONS AND THOSE OF THE MANUFACTURER'S REQUIREMENTS.



OPTION B

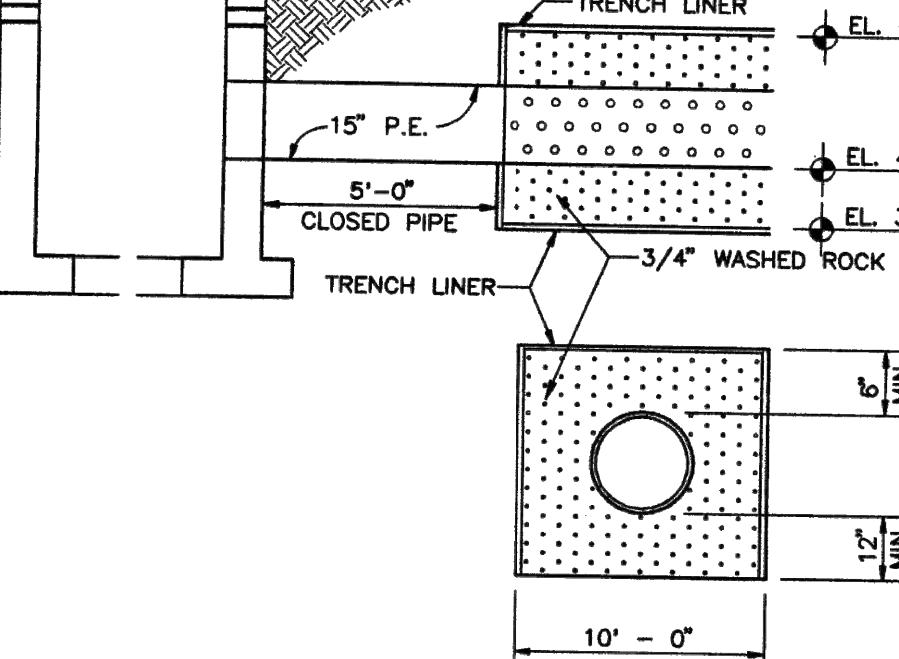


PAVEMENT CONNECTION DETAIL

N.T.S.

PAVEMENT RESTORATION DETAIL

N.T.S.



NOTES:
 1. PIPES SHALL TERMINATE 2 FEET FROM END OF TRENCH OR CONNECT TO ADDITIONAL CATCH BASINS AS REQUIRED (CAP ENDS OF PIPE).
 2. SIDES, TOP AND BOTTOM OF TRENCH TO BE LINED WITH TRENCH LINER MATERIAL OVERLAP LINER A MINIMUM OF TWO FEET AT TOP OF TRENCH.

EXISTING EXFILTRATION TRENCH DETAIL

N.T.S.

ZT	4/14/23	3/27/23	CITY OF HOLLYWOOD - TAC SUBMITTAL COMMENTS
DESIGNED BY			DATE
ZT	4/14/23		
DRAWN BY			DATE
RR	4/14/23		
CHECKED BY			DATE
RR	4/14/23		
APPROVED BY		No.	DATE
			REVISIONS

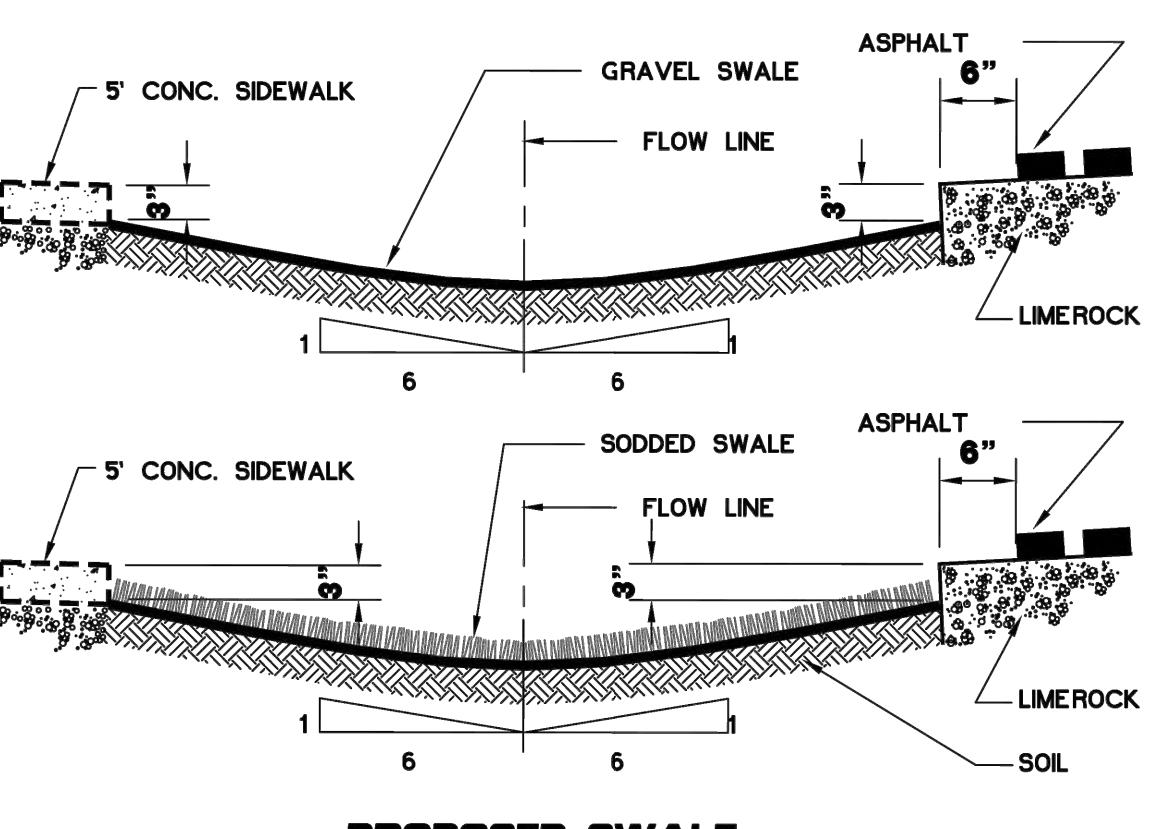
SAFEGUARD STORAGE
3090 SHERMAN STREET
HOLLYWOOD, FL 33021

RE
ROSS ENGINEERING, INC.

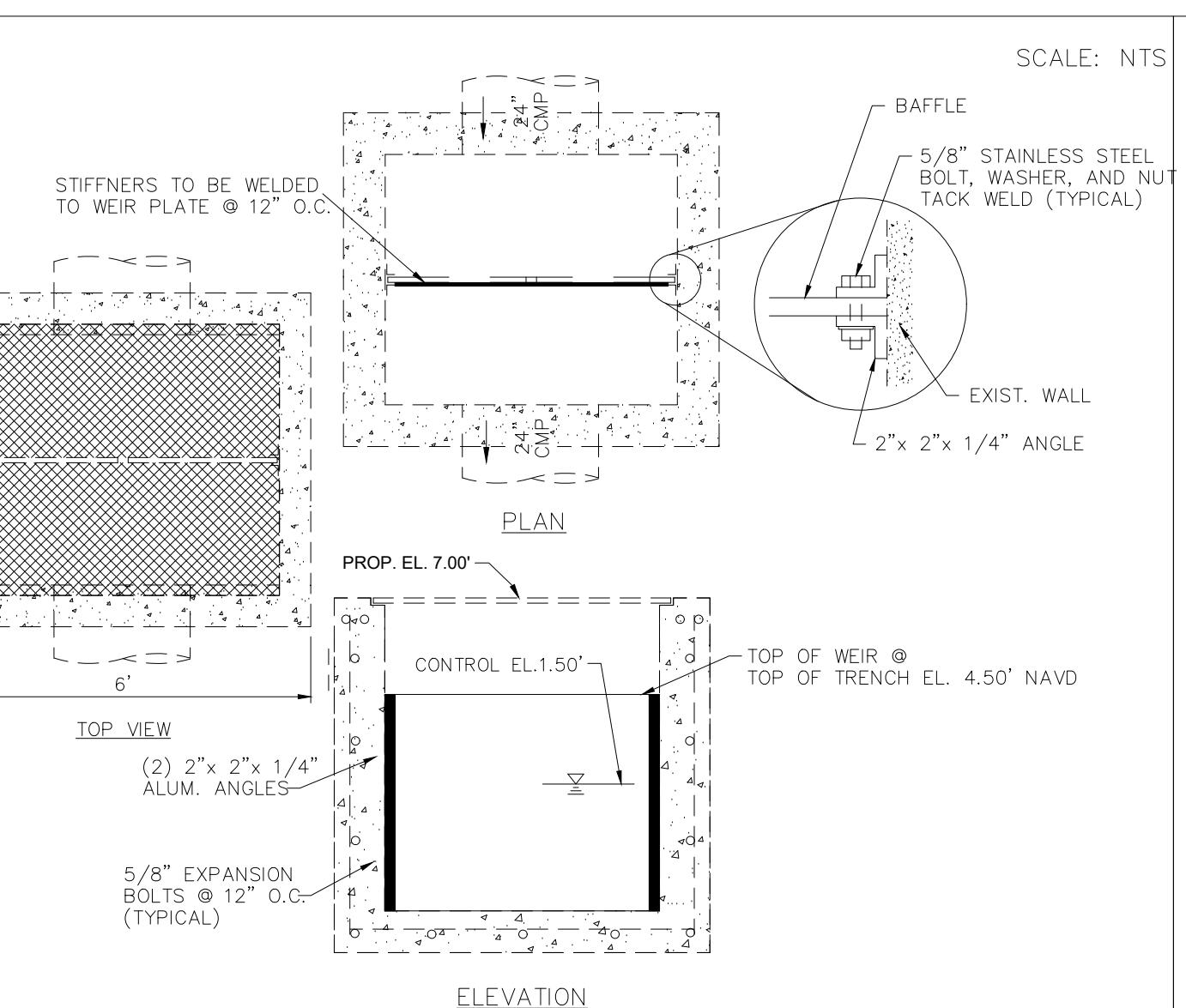
3325 S. UNIVERSITY DRIVE, SUITE 111
DAVIE, FLORIDA 33328
(954)318-0624 (954)358-0190 FAX
CERTIFICATE OF AUTHORIZATION No. 9808

SWALE GRADING SPECIFICATIONS:

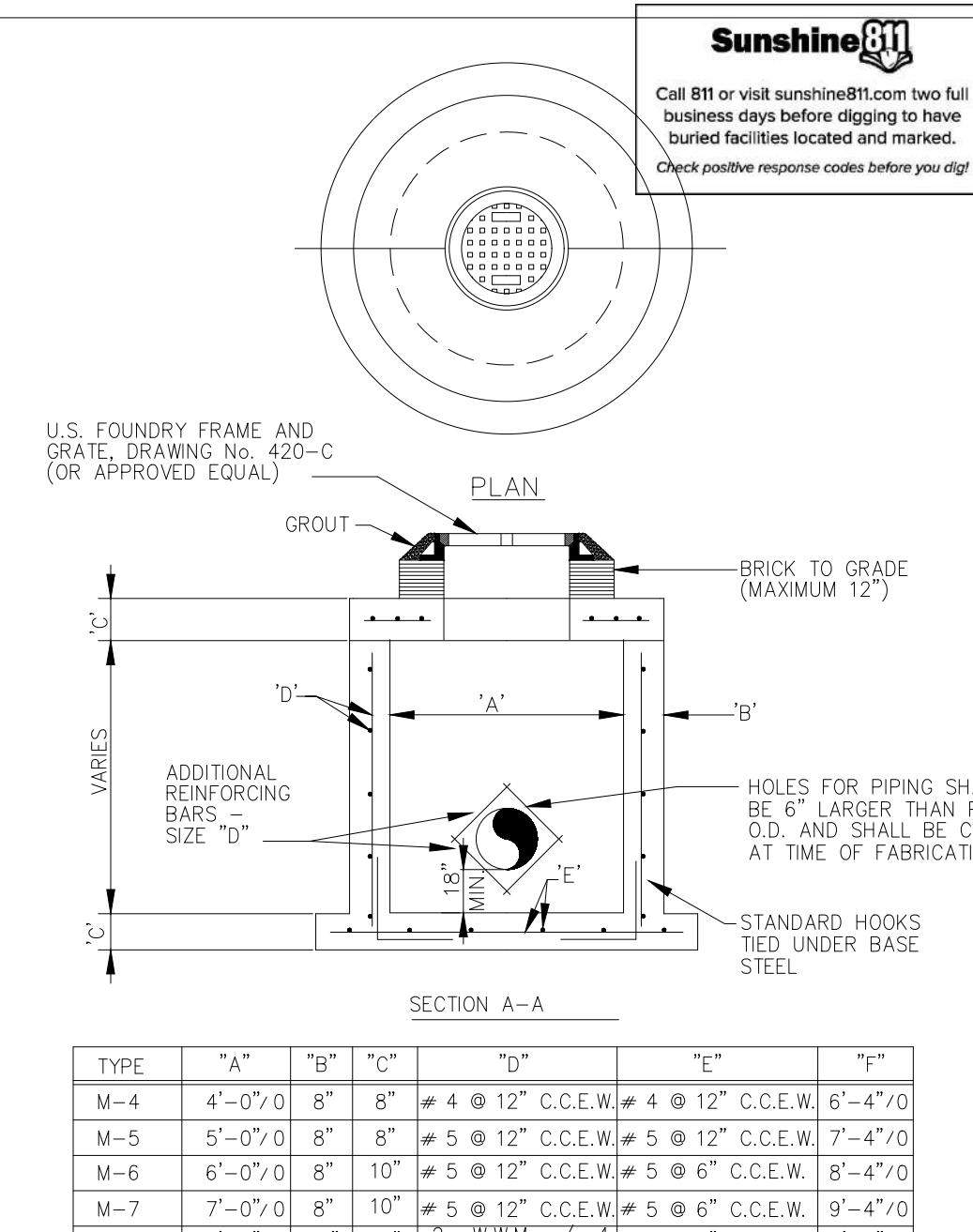
GRASS REPLACEMENT TO MATCH LOT WHICH SWALE FRONTS.
 SOIL AND GRASS TO BE HAULED OFF TO DUMP SITE.
 SPRINKLER SYSTEM TO BE REMOVED, LOWERED, AND RECONNECTED WHERE APPLICABLE.
 UNDERGROUND LOCATIONS AND ALL OTHER SAFETY FACTORS, ARE THE CONTRACTORS RESPONSIBILITIES.



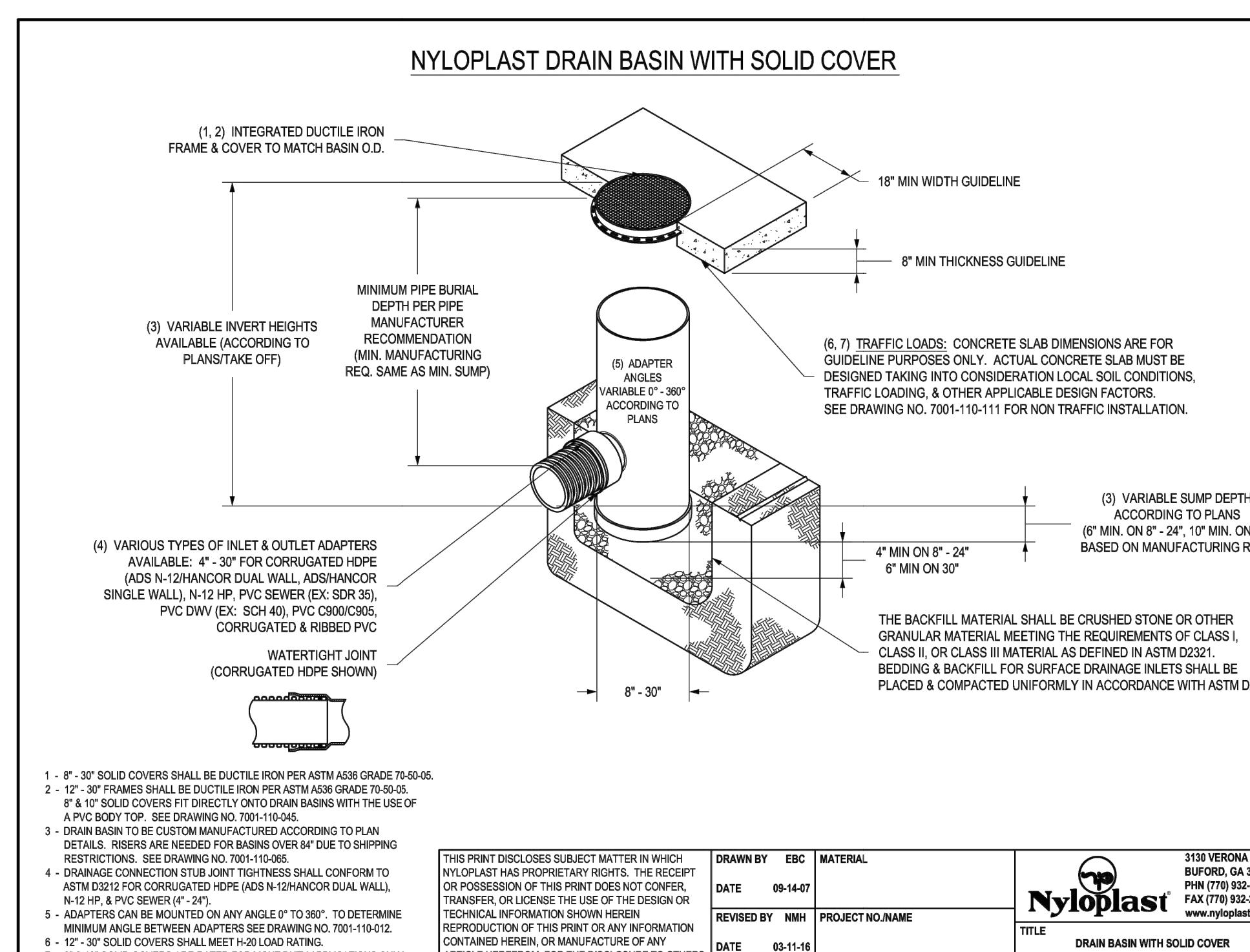
FILE NO. 7001-110-025 DRAWN BY: S. SAWYER DATE: 03/27/23 SHEET 1 OF 1
ENGINEERING SUPPORT SERVICES
PUBLIC UTILITIES DEPARTMENT
 CITY OF HOLLYWOOD FLORIDA
SWALE GRADING SPECIFICATIONS



CONTROL STRUCTURE DETAIL



DRAINAGE MANHOLE (TYPICAL)



Nyloplast

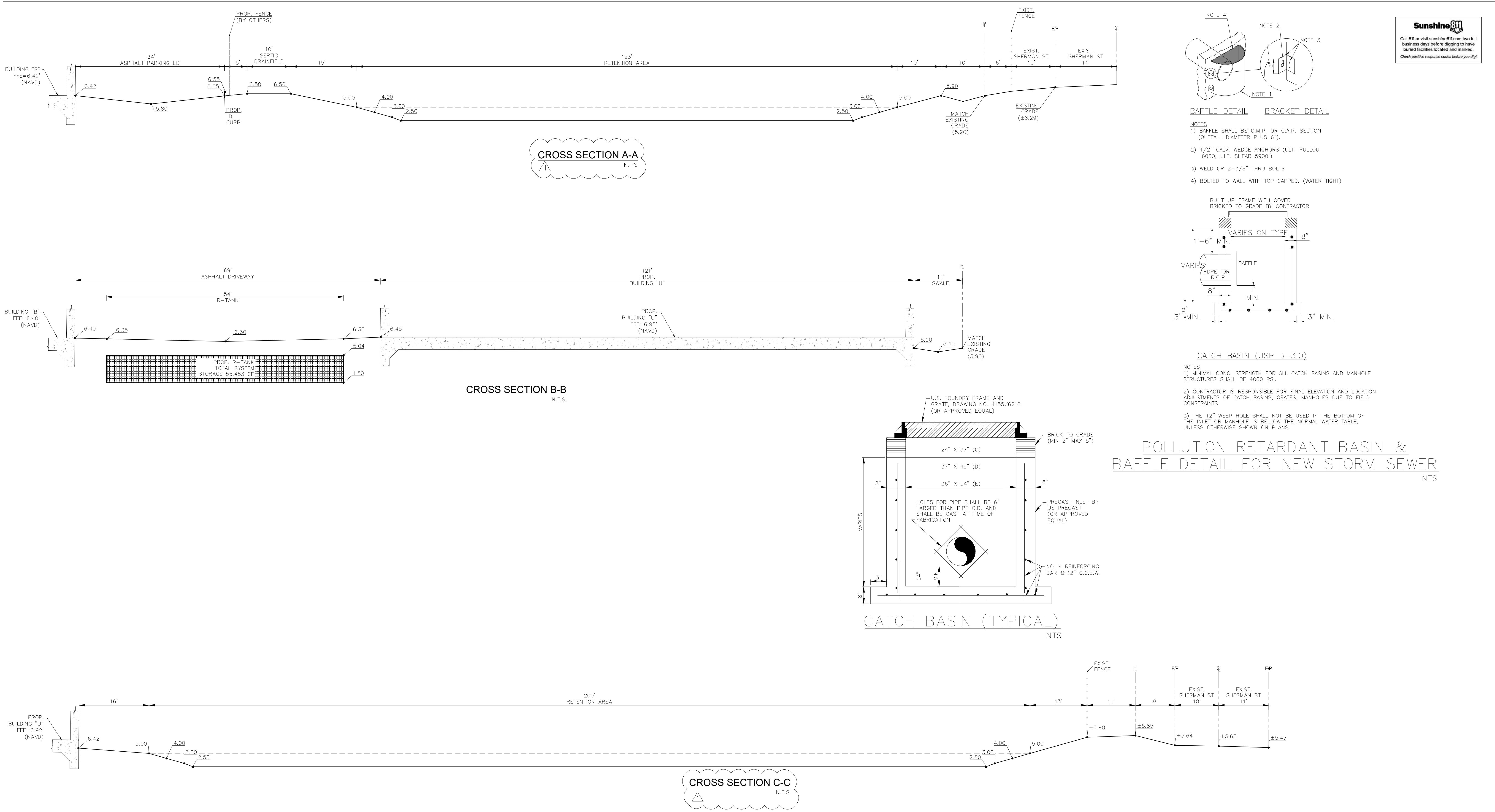
3019 LEMAR AVE
BUFORD, GA 30518
PH (770) 932-2443
FAX (770) 932-2490
www.nyloplast-us.com

03/13/2013

REV D

7001-110-256

100% CONSTRUCTION DOCUMENTS

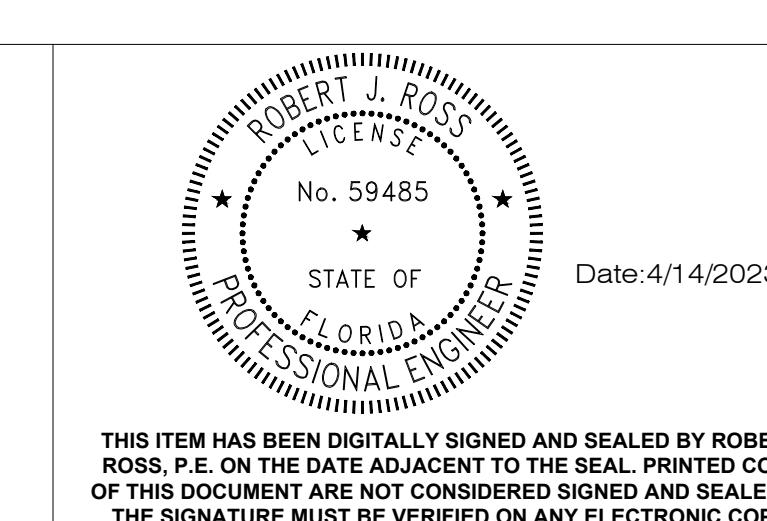


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DESIGNED BY				
ZT	4/14/23	DATE		
DRAWN BY				
RR	4/14/23	DATE		
CHECKED BY				
RR	4/14/23	DATE		
APPROVED BY			No. DATE	REVISIONS

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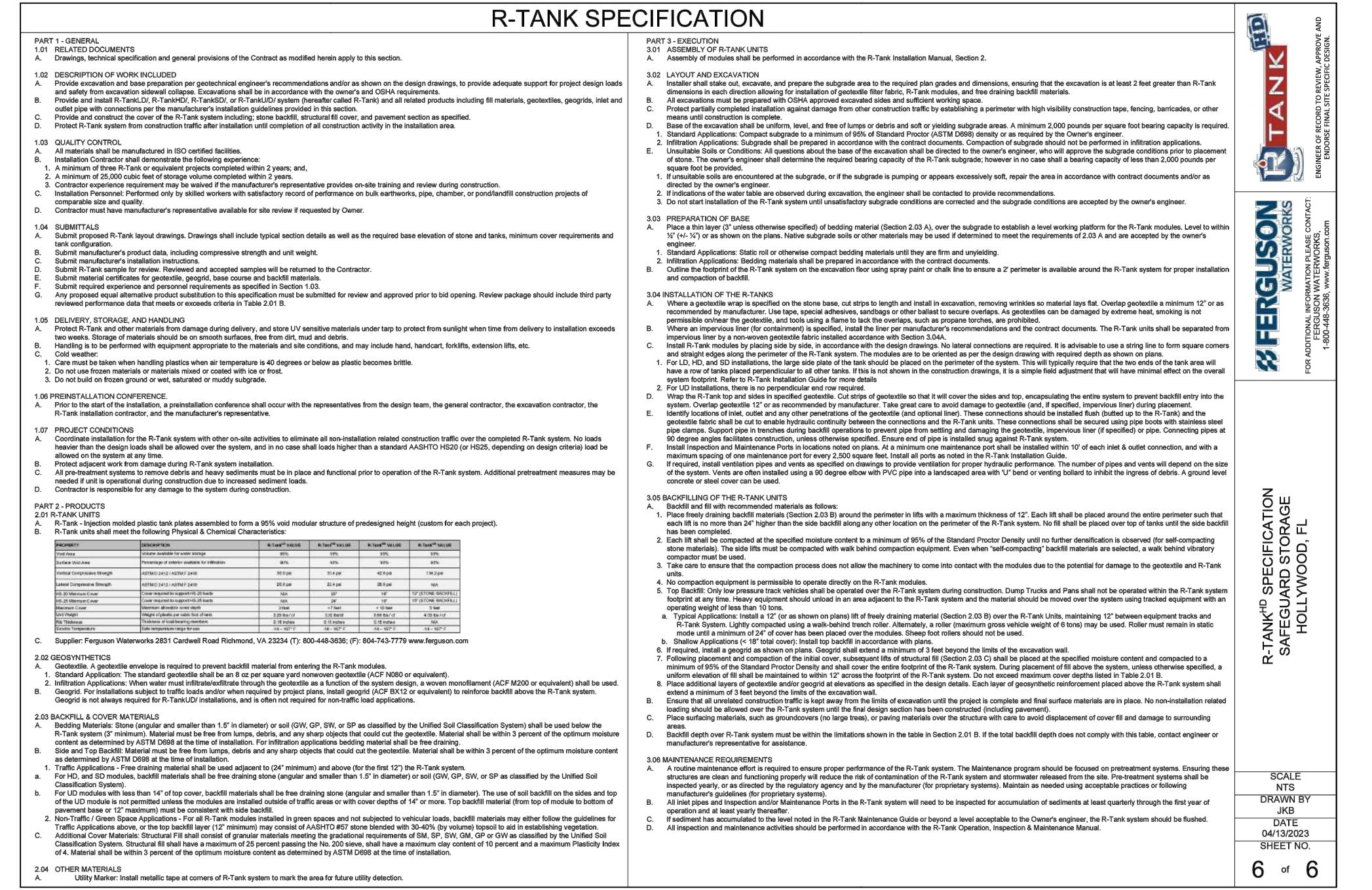
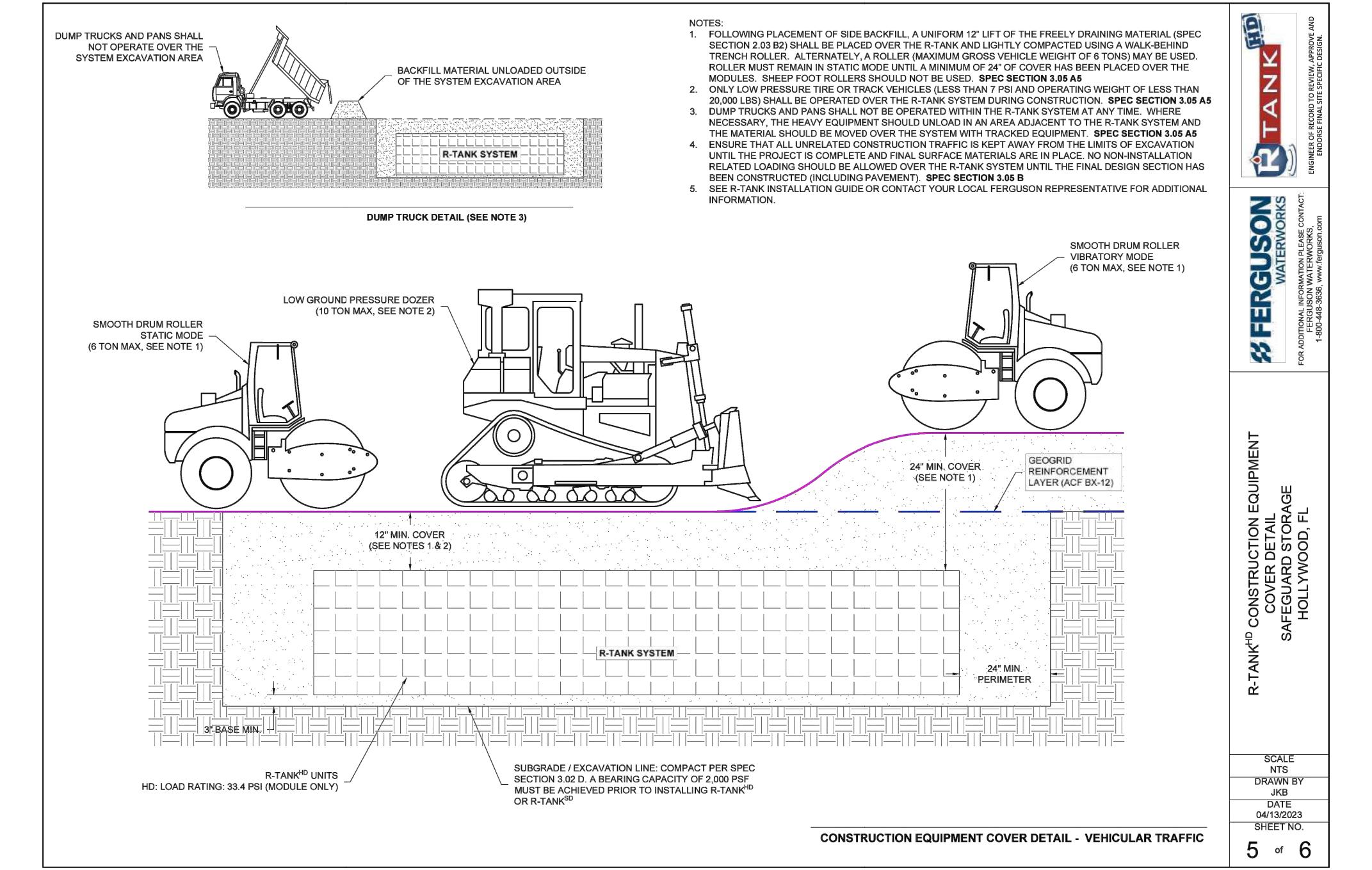
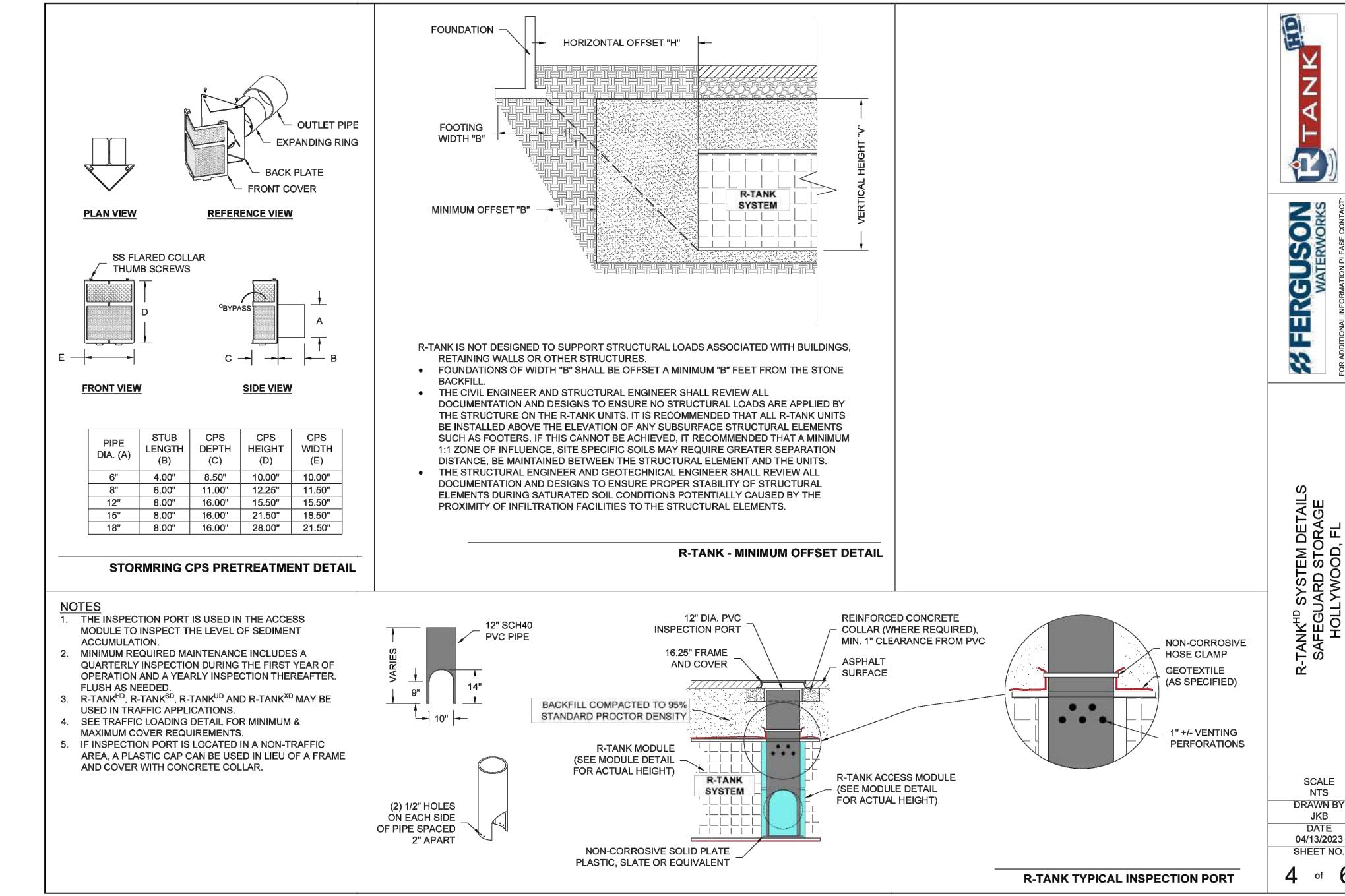
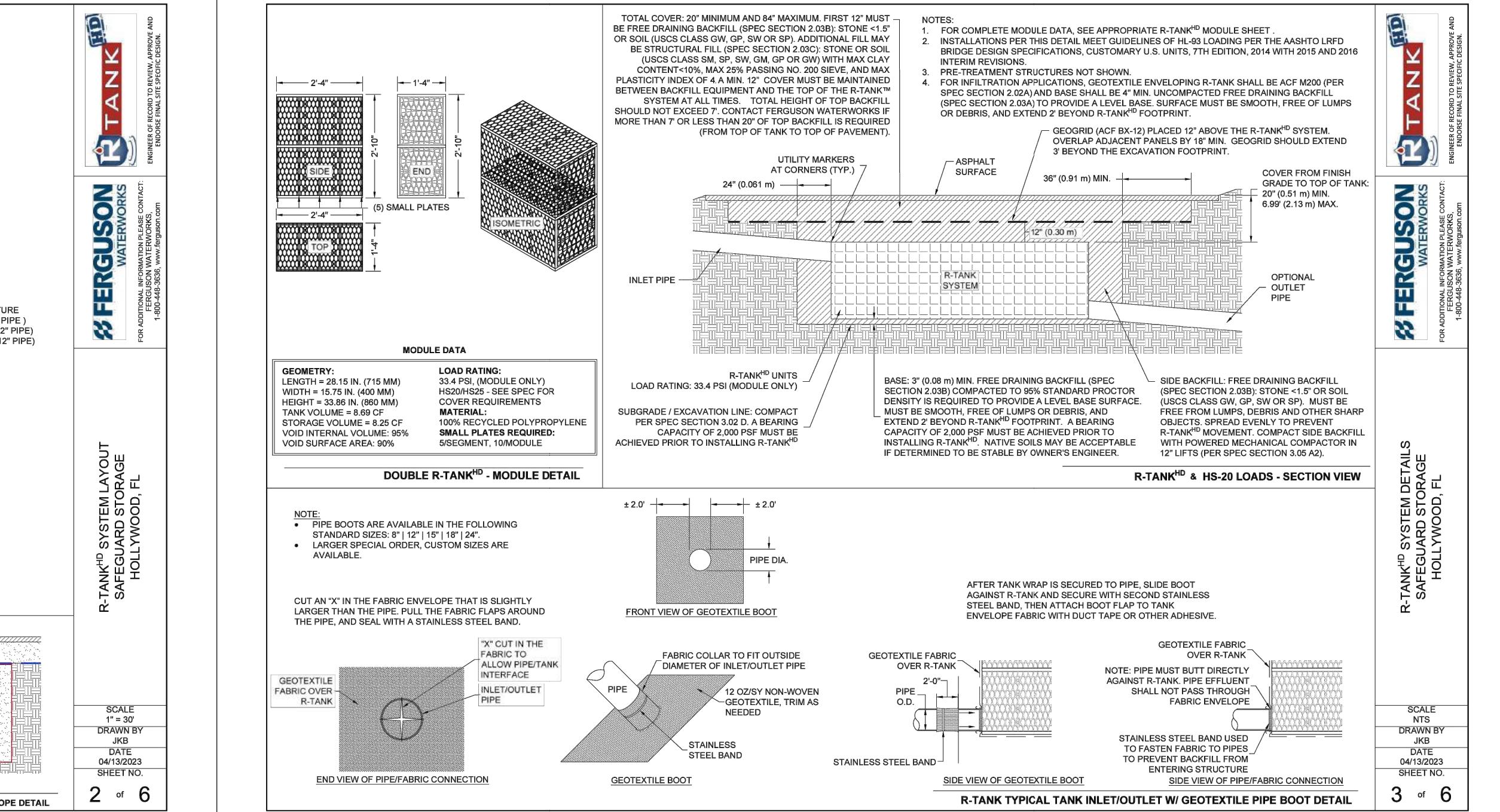
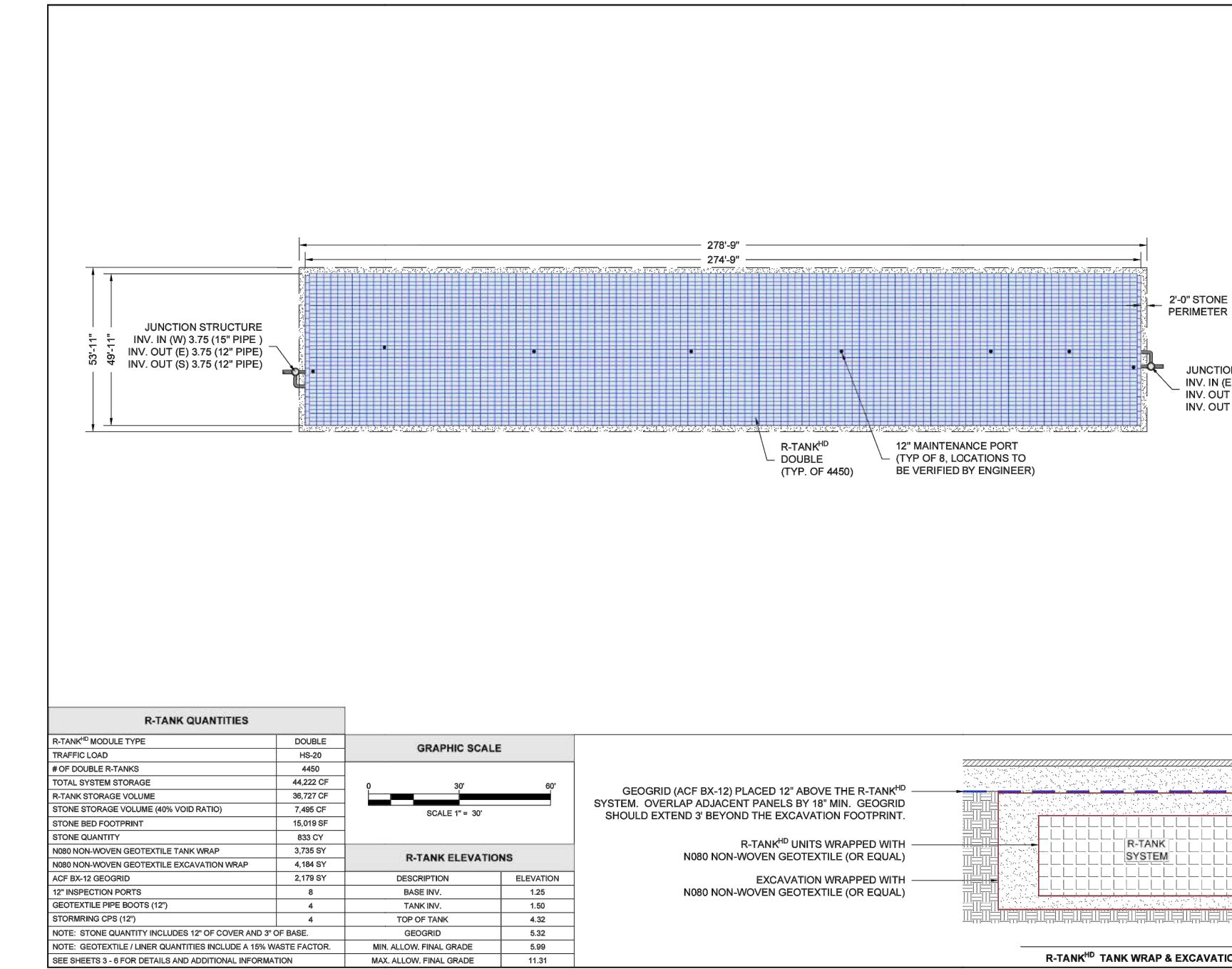
**PAVING GRADING AND DRAINAGE DETAILS
(SHEET 3 OF 3)**

SCALE: NTS

SHEET No. C-3.4

Sunshine 811
Call 811 or visit sunshine811.com two full business days before digging to have buried facilities located and marked.
Check positive response codes before you dig!

100% CONSTRUCTION DOCUMENTS

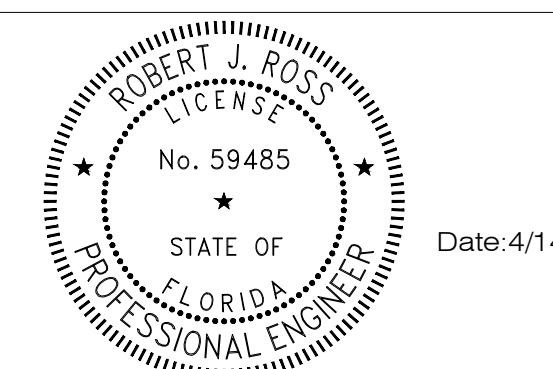


ZT	4/14/23	3/27/23	CITY OF HOLLYWOOD - TAC SUBMITTAL COMMENTS
DESIGNED BY			DATE
ZT	4/14/23		
DRAWN BY			DATE
RR	4/14/23		
CHECKED BY			DATE
RR	4/14/23		
APPROVED BY		No. DATE	REVISIONS

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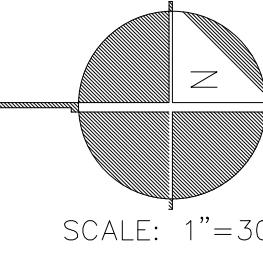


Date: 4/14/2023

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RAINTANK DETAILS

SCALE: NTS
SHEET NO. C-35



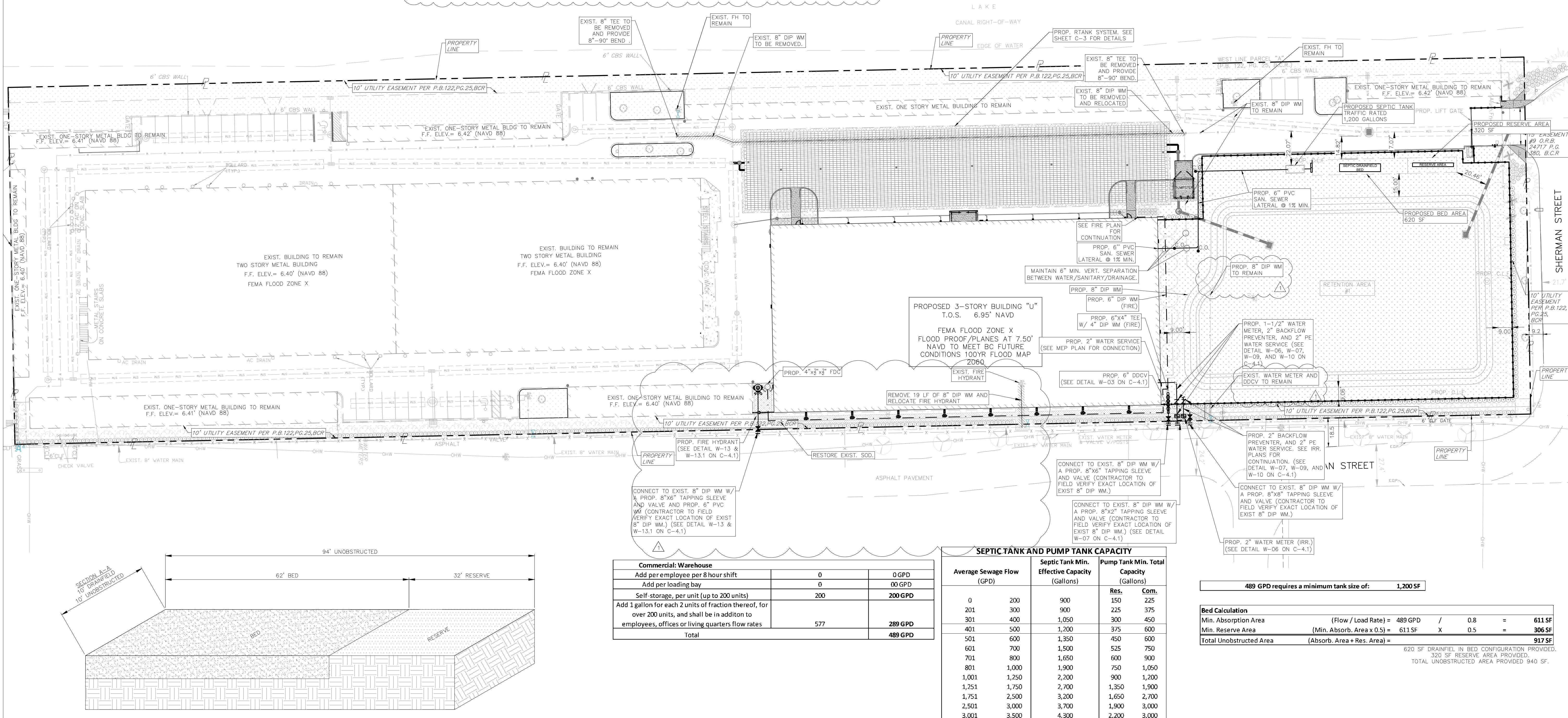
Sunshine 811
Call 811 or visit sunshine811.com two full business days before digging to have buried facilities located and marked.
Check positive response codes before you dig

LEGEND

	PROPERTY LINE		PROPOSED BACKFLOW PREVENTER
	EXISTING BUILDING		PROPOSED WATER METER
	EXISTING OVERHEAD LINE		PROPOSED SANITARY LINE
	EXISTING WATER LINE		PROPOSED CLEAN OUT
	EXISTING SANITARY SEWER LINE		PROPOSED DDCV
	PROPOSED WATER LINE		PROPOSED TAPPING SLEEVE
	PROPOSED SOD & RESTORATION		PROPOSED FDC

NOTES

1. EXISTING UNDERGROUND UTILITIES SHOWN HEREON ARE APPROXIMATE LOCATIONS AND HAVE BEEN PREPARED FROM THE MOST RELIABLE INFORMATION AVAILABLE TO THE ENGINEER. CONTRACTOR SHALL BE RESPONSIBLE TO VERIFY LOCATION AND DEPTH OF ALL UNDERGROUND UTILITIES PRIOR TO COMMENCEMENT OF CONSTRUCTION.
2. CONTRACTOR TO FIELD VERIFY ANY CONFLICTS WITH TREES AND/OR UTILITIES AND DRAINAGE. CONTRACTOR TO NOTIFY ENGINEER OF ANY CONFLICTS BEFORE PROCEEDING WITH ANY SOLUTION TO THE CONFLICT.
3. CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO SAFEGUARD ALL EXISTING STRUCTURES AND UTILITIES, ANY DAMAGE DONE TO EXISTING UTILITIES SHALL BE REPAIRED BY THE CONTRACTOR AT NOT EXPENSE TO THE SUBJECT UTILITY. CALL "SUNSHINE" 48 HOURS BEFORE DIGGING.
4. CONTRACTOR IS TO RESTORE ANY CURB, LANDSCAPE, ASPHALT, ETC. (NOT SCHEDULED FOR DEMOLITION) DAMAGED DURING CONSTRUCTION TO A CONDITION EQUAL TO WHAT IS EXISTING.
5. ALL PROPOSED FIRE LINES, MAINS, SERVICES, "SIAMESE" CONNECTION LINES ETC. TO BE DESIGNED AND INSTALLED BY STATE LICENSED FIRE LINE CONTRACTOR PER F.S. 633.
6. ALL FIRE LINES TO BE INSPECTED BY CERTIFIED FIRE LINE INSPECTORS.
7. UPON COMPLETION OF REQUIRED TESTING, THE STATE LICENSED FIRE LINE CONTRACTOR SHALL ISSUE A "LICENSED UNDERGROUND TEST CERTIFICATE" PRIOR TO ACCEPTANCE FOR PLACING FIRE LINE INTO SERVICE.
8. ALL UNDERGROUND FIRE MAIN WORK MUST BE COMPLETED BY FIRE PROTECTION CONTRACTOR HOLDING A CLASS I, II, OR V LICENSE PER FS 633.102.



100% CONSTRUCTION DOCUMENTS

WATER AND SEPTIC PLAN

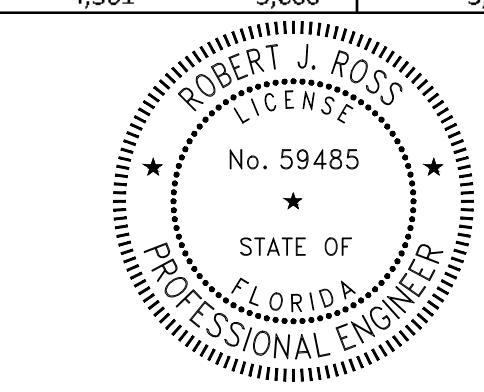
ZT	4/14/23		3/27/23	CITY OF HOLLYWOOD – TAC SUBMITTAL COMMENTS
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RR	4/14/23			
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RR	4/14/23			
APPROVED BY	DATE	No.	DATE	REVISIONS

SAFEGUARD STORAGE

3090 SHERMAN STREET
HOLLYWOOD, FL 33021

The logo for Ross Engineering, Inc. features the letters "RE" in a large, bold, blue font with a metallic, reflective texture. Below the letters, the company name "ROSS ENGINEERING, INC." is written in a smaller, blue, sans-serif font.

3325 S. UNIVERSITY DRIVE, SUITE 111
DAVIE, FLORIDA 33328
(954)318-0624 (954)358-0190 FAX
CERTIFICATE OF AUTHORIZATION No. 9808



Date:4/14/2023

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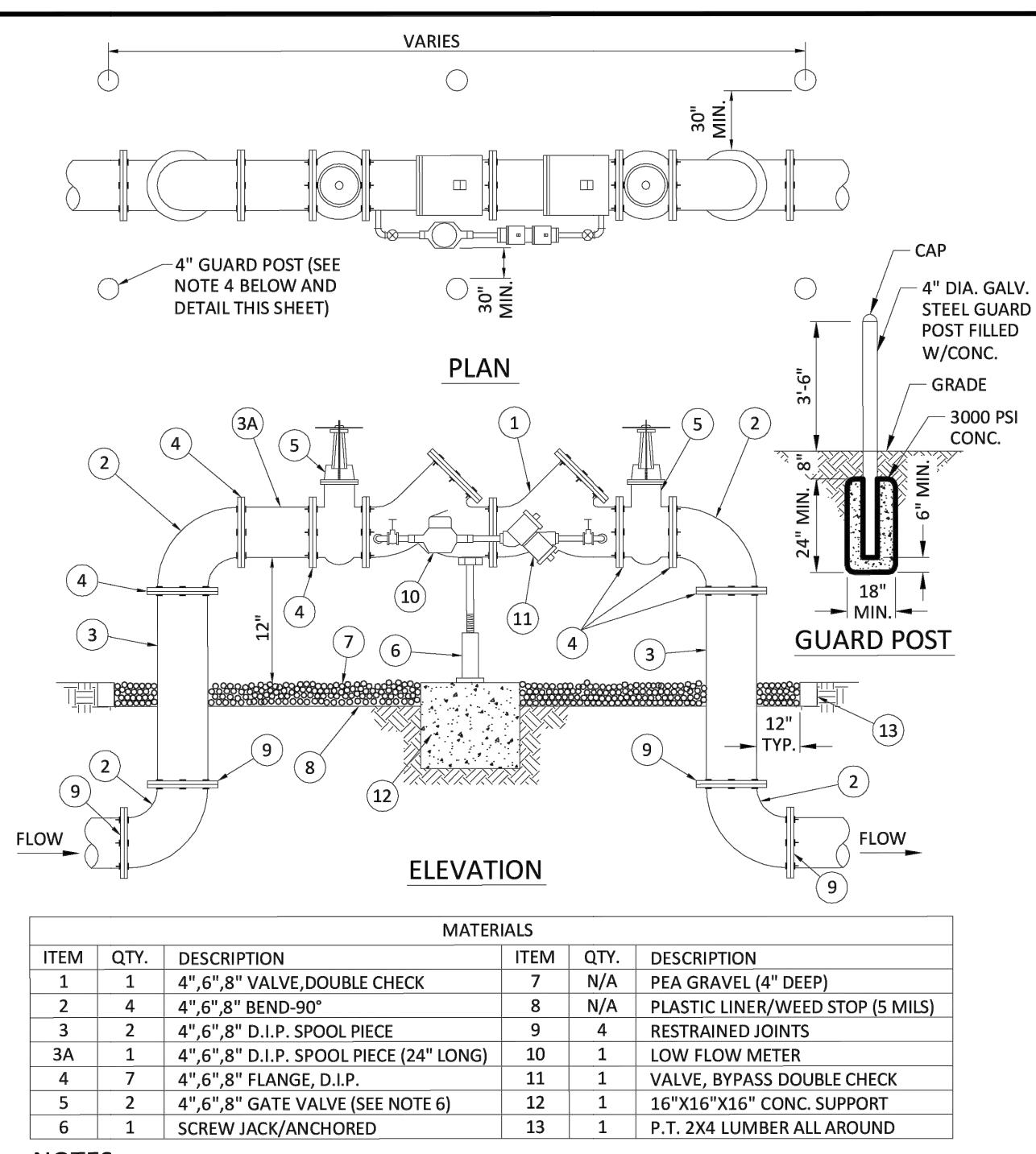
SCALE 1" = 30'

SHEET No. C-4

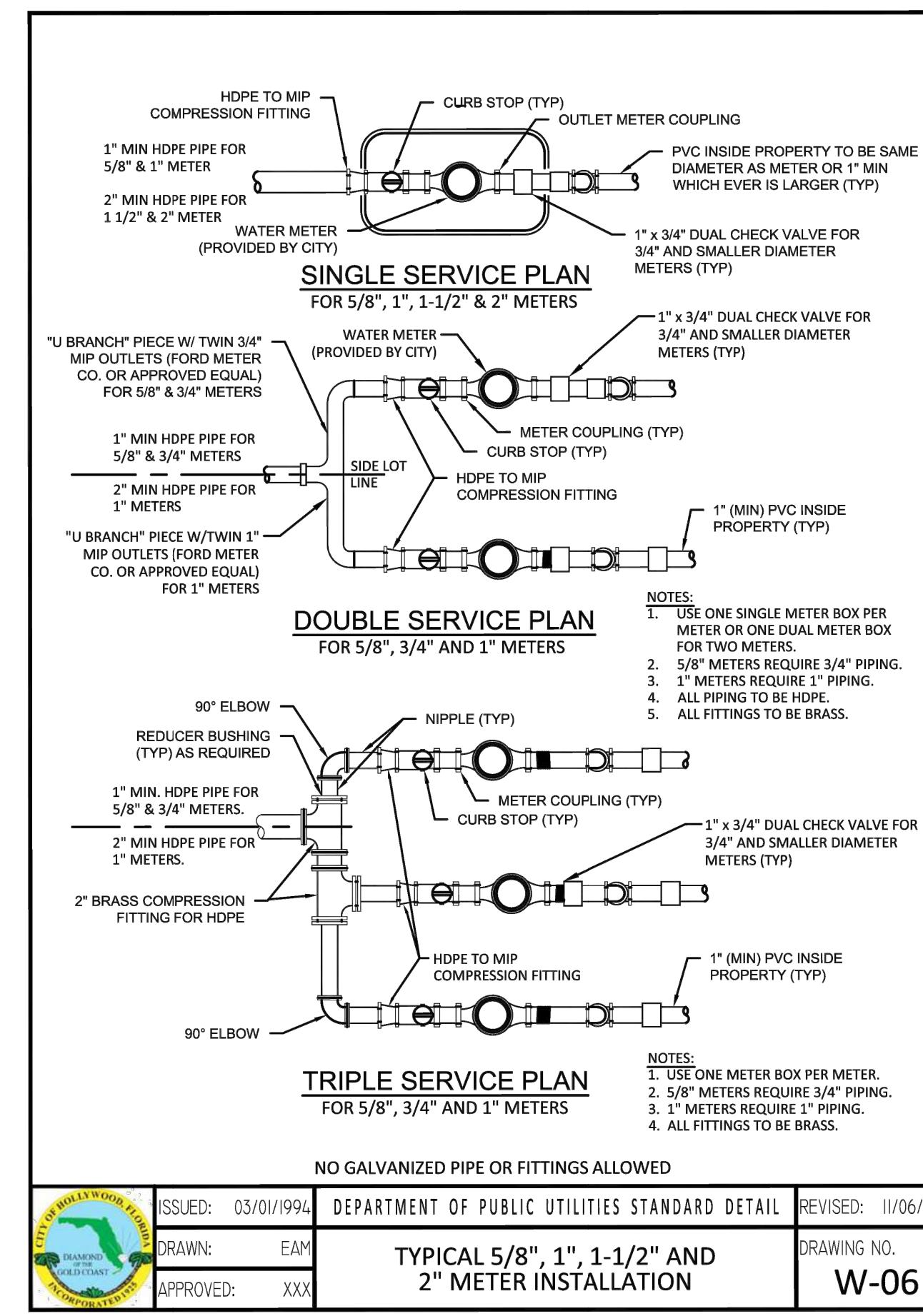
WATER SYSTEM NOTES:

1. NEW OR RELOCATED UNDERGROUND WATER MAINS INCLUDED IN THIS PROJECT THAT WILL CROSS ANY EXISTING OR PROPOSED GRAVITY OR VACUUM-TYPE SANITARY SEWER OR STORM SEWER WILL BE LAID SO THE OUTSIDE OF THE WATER MAIN IS AT LEAST SIX INCHES ABOVE THE OTHER PIPELINE OR AT LEAST 12 INCHES BELOW THE OTHER PIPELINE.
2. NEW OR RELOCATED UNDERGROUND WATER MAINS INCLUDED IN THIS PROJECT THAT WILL CROSS ANY EXISTING OR PROPOSED PRESSURE-TYPE SANITARY SEWER, WASTEWATER OR STORM WATER FORCE MAIN, OR PIPELINE CONVEYING RECLAIMED WATER WILL BE LAID SO THE OUTSIDE OF THE WATER MAIN IS AT LEAST 12 INCHES ABOVE OR BELOW THE OTHER PIPELINE. [FAC 62-555.314(2); EXCEPTIONS ALLOWED UNDER FAC 62-555.314(5)].
3. AT ALL UTILITY CROSSINGS DESCRIBED ABOVE, ONE FULL LENGTH OF WATER MAIN PIPE WILL BE CENTERED ABOVE OR BELOW THE OTHER PIPELINE SO THE WATER MAIN JOINTS WILL BE AS FAR AS POSSIBLE FROM THE OTHER PIPELINE. THE PIPES WILL BE LAID SO THE ALL WATER MAINS ARE AT LEAST THREE FEET FROM ALL JOINTS IN VACUUM-TYPE SANITARY SEWERS, STORM SEWERS, STORM WATER FORCE MAINS, OR PIPELINES CONVEYING RECLAIMED WATER REGULATED UNDER PART III OF CHAPTER 62-610, F.A.C., AND AT LEAST SIX FEET IN GRAVITY OR PRESSURE-TYPE SANITARY SEWERS, WASTEWATER FORCE MAINS, OR PIPELINES CONVEYING RECLAIMED WATER NOT REGULATED UNDER PART III OF CHAPTER 62-610, F.A.C. [FAC 62-555.314(2); EXCEPTIONS ALLOWED UNDER FAC 62-555.314(5)].
4. NEW UNDERGROUND WATER MAINS INCLUDED IN THIS PROJECT TO BE DUCTILE IRON PIPE (D.I.P.) WHEN CROSSING BELOW SANITARY SEWER MAINS.
5. POLYETHYLENE ENCASEMENT MATERIAL SHALL BE USED TO ENCASE ALL BURIED DUCTILE IRON PIPE, FITTINGS, VALVES, RODS, AND APPLIANCES IN ACCORDANCE WITH AWWA C105, METHOD A. THE POLYETHYLENE TUBING SHALL BE CUT TWO FEET LONGER THAN THE PIPE SECTION AND SHALL OVERLAP THE ENDS OF THE PIPE BY ONE FOOT. THE POLYETHYLENE TUBING SHALL BE GATHERED AND LAPPED TO PROVIDE A SNUG FIT AND SHALL BE SECURED AT QUARTER POINTS WITH POLYETHYLENE TAPE. EACH END OF THE POLYETHYLENE TUBING SHALL BE SECURED WITH A WRAP OF POLYETHYLENE TAPE.
6. THE POLYETHYLENE TUBING SHALL PREVENT CONTACT BETWEEN THE PIPE AND BEDDING MATERIAL, BUT IS NOT INTENDED TO BE A COMPLETELY AIRTIGHT AND WATERPROOF ENCLOSURE. DAMAGED POLYETHYLENE TUBING SHALL BE REPAIRED IN A WORKMANLIKE MANNER USING POLYETHYLENE TAPE, OR THE DAMAGED SECTION SHALL BE REPLACED. POLY WRAP WILL NOT BE PAID FOR AS A SEPARATE BID ITEM. IT SHALL BE CONSIDERED TO BE A PART OF THE PRICE BID FOR WATER MAINS.
7. FIRE HYDRANT BARRELS SHALL BE ENCASED IN POLY WRAP UP TO THE GROUND SURFACE AND THE WEEP HOLES SHALL NOT BE COVERED BY THE POLY WRAP.
8. GATE VALVES FOR USE WITH PIPE LESS THAN THREE INCHES (3") IN DIAMETER SHALL BE RATED FOR TWO HUNDRED (200) PSI WORKING PRESSURE, NON-SHOCK, BLOCK PATTERN, SCREWED BONNET, NON-RISING STEM, BRASS BODY, AND SOLID WEDGE. THEY SHALL BE STANDARD THREADED FOR PVC PIPE AND HAVE A MALEABLE IRON HANDWHEEL. GATE VALVES 3" THROUGH 16" IN DIAMETER SHALL BE RESILIENT SEAT AND BIDIRECTIONAL FLOW ONLY. VALVES FOR SPECIAL APPLICATIONS WILL REQUIRE CITY UTILITY APPROVAL.
9. VALVE BOXES AND COVERS FOR ALL VALVES SHALL BE OF CAST IRON CONSTRUCTION AND ADJUSTABLE SCREW-ON TYPE. THE LID SHALL HAVE CAST IN THE METAL THE WORD "WATER" FOR THE WATER LINES. ALL VALVE BOXES SHALL BE SIX INCH (6") NOMINAL DIAMETER AND SHALL BE SUITABLE FOR DEPTHS OF THE PARTICULAR VALVE. THE STEM OF THE BURIED VALVE SHALL BE WITHIN TWENTY-FOUR INCHES (24") OF THE FINISHED GRADE UNLESS OTHERWISE APPROVED BY THE CITY.
10. ALL WATER MAIN INSTALLATIONS SHALL COMPLY WITH THE COLOR CODING REQUIREMENTS OF CHAPTER 62-555.320 F.A.C.

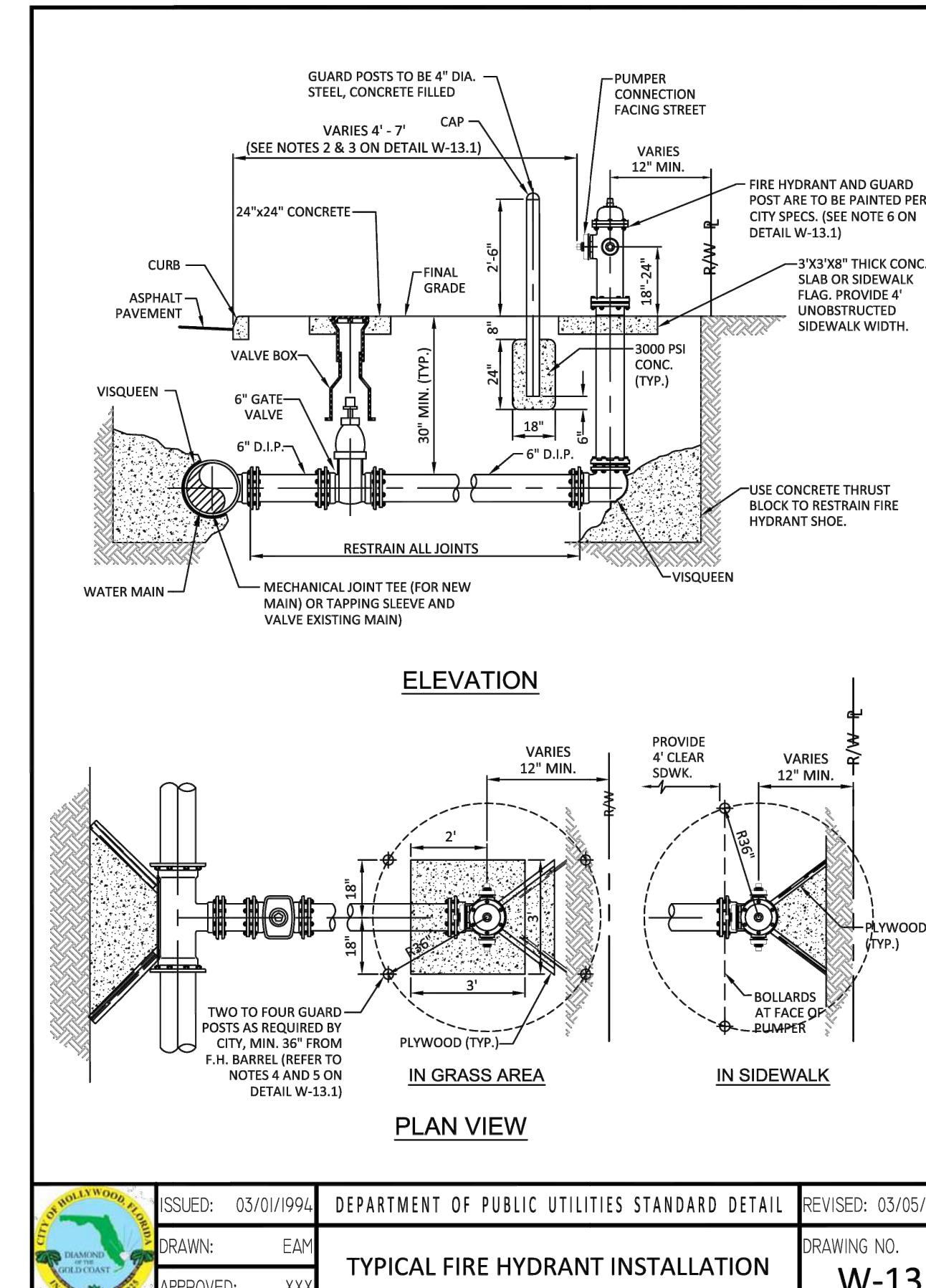
	ISSUED: 03/01/1994	DEPARTMENT OF PUBLIC UTILITIES STANDARD DETAIL	REVISED: 06/08/2014
DRAWN: EAM	APPROVED: XXX	WATER SYSTEM NOTES	DRAWING NO. W-01



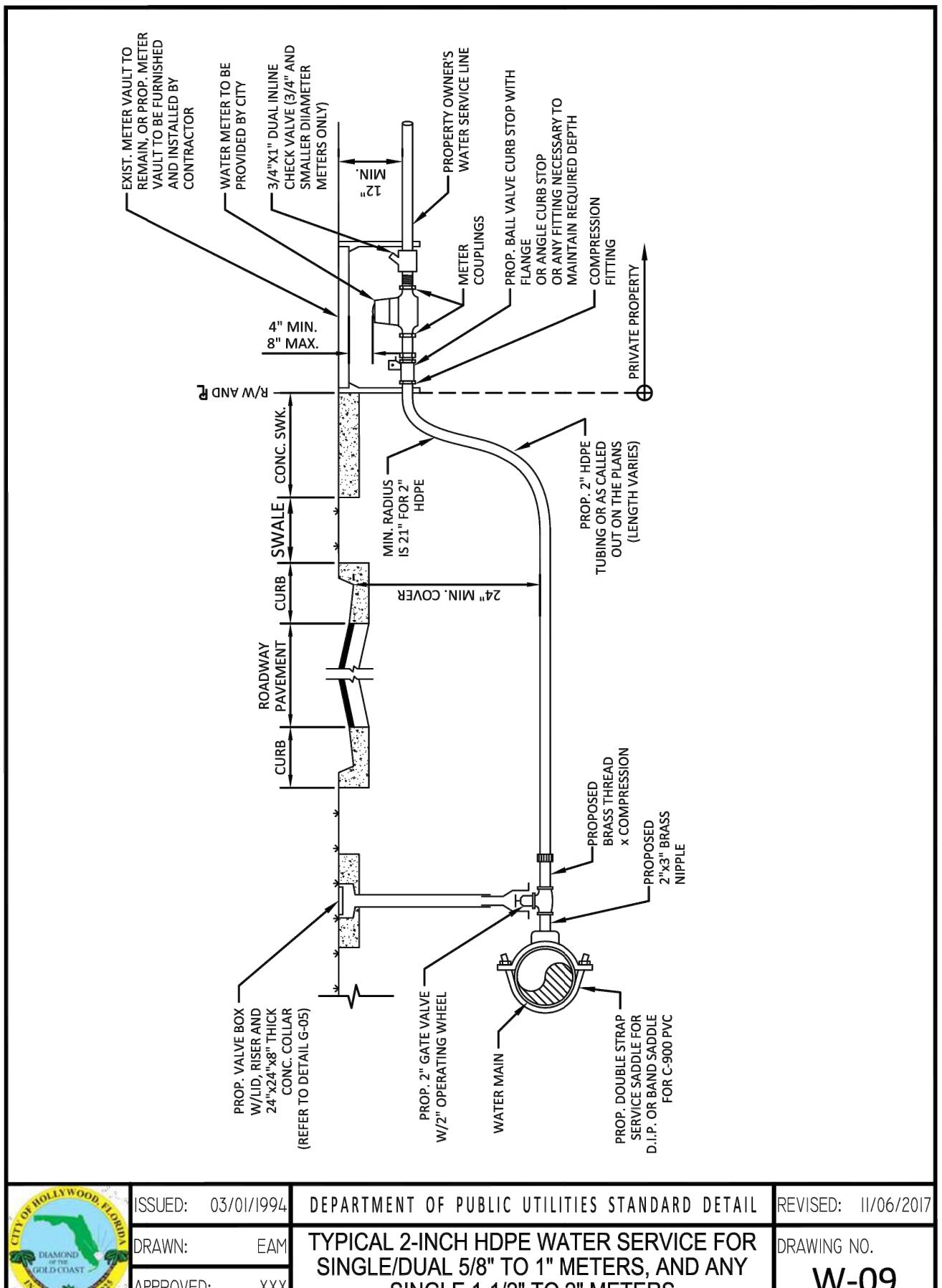
	ISSUED: 03/01/1994	DEPARTMENT OF PUBLIC UTILITIES STANDARD DETAIL	REVISED: 06/08/2014
DRAWN: EAM	APPROVED: XXX	TYPICAL 4", 6" AND 8" DOUBLE CHECK DETECTOR ASSEMBLY FOR FIRE SPRINKLER SERVICE (90° BENDS)	DRAWING NO. W-03



	ISSUED: 03/01/1994	DEPARTMENT OF PUBLIC UTILITIES STANDARD DETAIL	REVISED: 11/06/2017
DRAWN: EAM	APPROVED: XXX	TYPICAL 5/8", 1", 1-1/2" AND 2" METER INSTALLATION	DRAWING NO. W-06



	ISSUED: 03/01/1994	DEPARTMENT OF PUBLIC UTILITIES STANDARD DETAIL	REVISED: 03/05/2019
DRAWN: EAM	APPROVED: XXX	TYPICAL FIRE HYDRANT INSTALLATION	DRAWING NO. W-13



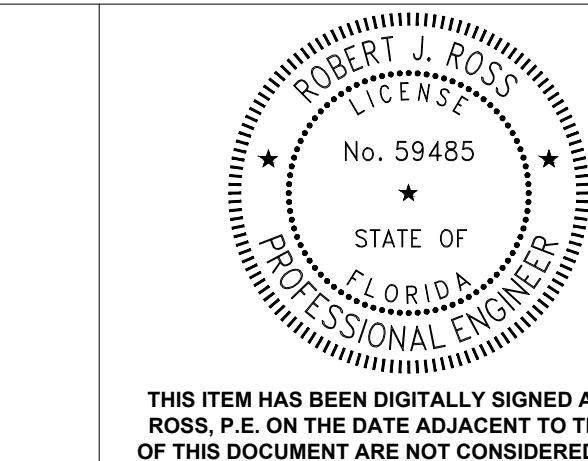
	ISSUED: 03/01/1994	DEPARTMENT OF PUBLIC UTILITIES STANDARD DETAIL	REVISED: 02/14/2018
DRAWN: EAM	APPROVED: XXX	TYPICAL 2-INCH HDPE WATER SERVICE FOR SINGLE/DUAL 5/8" TO 1" METERS, AND ANY SINGLE 1-1/2" TO 2" METERS	DRAWING NO. W-09

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HOLLYWOOD, FL 33021



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WATER METER SERVICE NOTES:

1. SUCCESIVE TAPS INTO THE WATER MAIN SHALL BE SPACED NOT LESS THAN 18" ON CENTER.
2. P.E. TUBING SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF AWWA STANDARD C901, "POLYETHYLENE (PE) PRESSURE PIPE AND TUBING, 1/2 IN. (13mm) THROUGH 3 IN. (76 mm), FOR WATER SERVICE".
3. MINIMUM SERVICE PIPE DIAMETER SHALL BE 1" FOR SINGLE OR DUAL 5/8" OR SINGLE 1"
4. MINIMUM SERVICE PIPE DIAMETER SHALL BE 2" FOR SINGLE OR DUAL 1-1/2" OR SINGLE 2"
5. FOR METER DIAMETERS LARGER THAN 2", THE MINIMUM SERVICE PIPE DIAMETER SHALL BE THE SAME AS THE METER DIAMETER.
6. APPROVED COPPER TUBING MAY BE USED AT THE CITY'S DISCRETION.
7. FOR NEW METER INSTALLATIONS, ALL SADDLES, VALVES, PIPING, FITTINGS, CURB STOPS, METER VALVES, METER COUPLINGS, METER VAULTS AND COVERS SHALL BE FURNISHED AND INSTALLED BY THE CONTRACTOR. THE WATER METERS WILL BE PROVIDED AND INSTALLED BY THE CITY OF HOLLYWOOD (NEW ACCOUNTS).
8. FOR METER RELOCATIONS, ALL SADDLES, VALVES, PIPING, FITTINGS, CURB STOPS, METER VALVES, METER COUPLINGS, METER VAULTS AND COVERS SHALL BE FURNISHED AND INSTALLED BY THE CONTRACTOR. THE EXISTING WATER METER TO BE RELOCATED AND INSTALLED BY CONTRACTOR.
9. FOR EXISTING METERS ABUTTING THE RIGHT-OF-WAY THAT ARE BEING DISCONNECTED FROM EXISTING MAINS AND RECONNECTED TO NEW MAINS, THE CONTRACTOR SHALL:
 - CUT AND PLUG THE EXISTING SERVICE LINE AT THE MAIN AND AT THE METER, AND REMOVE THE EXISTING BALL VALVE CURB STOP.
 - FURNISH AND INSTALL SERVICE SADDLE, CORPORATION STOP OR SERVICE VALVE AND VALVE BOX, PIPING AND FITTINGS UP TO AND INCLUDING THE BALL VALVE CURB STOP.
10. THE ELEVATION AT THE TOP OF THE METER BOX SHALL MATCH THE ELEVATION OF THE BACK OF SIDEWALK, WHENEVER PRACTICAL.
11. AS PART OF THE SERVICE INSTALLATION, THE CONTRACTOR SHALL RESTORE THE RIGHT-OF-WAY TO MATCH EXISTING CONDITIONS, INCLUDING ROADWAY PAVEMENT, PAVEMENT MARKINGS AND RPMs, CONCRETE CURBS, SIDEWALKS, RAMPS (INCLUDING DETECTABLE WARNING SURFACE), SODDING, AND ALL OTHER IMPROVEMENTS REMOVED OR DAMAGED DURING THE SERVICE INSTALLATION.
12. FOR UNPAVED AREAS, THE MINIMUM GROUND COVER ACCEPTED BY THE CITY IS SODDING.

	ISSUED: 03/01/1994	DEPARTMENT OF PUBLIC UTILITIES STANDARD DETAIL	REVISED: 11/06/2017
DRAWN: EAM	APPROVED: XXX	WATER METER SERVICE NOTES FOR 5/8" THROUGH 2" METERS	DRAWING NO. W-07

100% CONSTRUCTION DOCUMENTS

WATER AND SEPTIC PLAN DETAILS (SHEET 1 OF 2)

Table 18.4.5.1.2 Minimum Required Fire Flow and Flow Duration for Buildings						
		Fire Flow Area ft^2 ($> 0.0929 \text{ m}^2$)		Fire Flow gpm ¹ ($> 3750 \text{ ft}^3 \text{ per minute}$)		
		10,000	10,000	10,000	10,000	Flow Duration (hours)
I4400, I4320, I4320 ²	10,000	10,000	10,000	10,000	10,000	
0-22,200	6-12,700	8-22,000	9-37,000	10-59,000	12-80,000	130
22,200-30,200	12,700-17,000	12,700-19,000	12,700-22,000	12,700-30,000	12,700-40,000	175
30,200-38,700	17,000-21,800	10,900-12,200	7900-9800	4800-6200	2000	2
38,700-48,300	21,800-34,200	12,900-17,400	9800-12,600	6200-7700	225	
48,300-59,000	24,200-31,200	17,400-21,300	12,600-15,400	7700-9400	2500	
59,000-70,900	31,200-38,700	21,300-25,500	15,400-18,400	9400-11,300	2750	
70,900-83,700	38,700-47,100	25,500-30,300	18,400-21,800	11,300-13,400	3000	
83,700-97,700	47,100-54,900	30,300-35,200	21,800-25,800	13,400-15,600	3250	
97,700-112,700	54,900-64,400	35,200-38,700	25,800-30,300	15,600-18,400	3500	3
112,700-128,700	64,400-74,400	38,700-46,400	30,300-35,200	18,400-20,000	3750	
128,700-145,900	74,400-82,100	46,400-52,500	35,200-37,900	20,000-23,300	4000	
145,900-164,200	82,100-90,400	52,500-59,100	37,900-42,700	23,300-26,300	4250	
164,200-183,400	90,400-103,000	59,100-66,000	42,700-47,700	26,300-29,300	4500	
183,400-203,700	103,000-114,600	66,000-73,100	47,700-53,000	29,300-32,600	4750	
203,700-225,200	114,600-126,700	73,100-81,000	53,000-58,600	32,600-36,000	5000	
225,200-247,200	126,700-139,400	81,000-89,200	58,600-65,400	36,000-39,600	5250	
247,200-271,200	139,400-152,600	89,200-97,700	65,400-70,600	39,600-43,400	5500	
271,200-295,300	152,600-166,500	97,700-106,500	70,600-77,000	43,400-47,400	5750	
Greater than 295,900	Greater than 166,500	106,500-115,800	77,000-83,700	47,400-51,500	6000	4
		115,800-128,700	83,700-90,400	51,500-58,600	6250	
		128,700-135,500	90,400-97,900	58,600-62,000	6400	
		135,500-143,800	97,900-106,300	62,000-68,800	6750	
		143,800-156,700	106,300-113,200	68,800-76,900	7000	
		156,700-167,900	113,200-121,300	76,900-84,600	7250	
		167,900-179,400	121,300-129,600	84,600-93,900	7500	
		179,400-191,400	129,600-138,300	93,900-105,100	7750	
		Greater than 191,400	Greater than 138,300	Greater than 85,000	8000	

*Types of construction are based on NFPA 220.

¹Measured at 20 psi (39.9 kPa).

Sunshine 80
Call 800-843-8888 before digging to have buried facilities located and marked.
Check positive response codes before digging.

BUILDING CONSTRUCTION NOTES	
CONSTRUCTION: TYPE II-A (TYPE II 000) OCCUPANCY: S	
NOTE: FOR NON-COMBUSTIBLE CONSTRUCTION TYPES, FIRE SPRINKLER PROTECTION IS NOT REQUIRED IN CONCEALED SPACES THAT COMPLY WITH NFPA 13 - 8.15.1.2	
EXTERIOR WALL TYPE: MASONRY/CONCRETE BLOCK	
PARTITION TYPE: METAL STUD & DRYWALL	
STRUCTURAL SUPPORT TYPE: STEEL BEAMS, BAR JOIST & ROOF TRUSS	
BUILDING AREA: 30,495 SQ. FT	

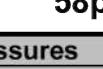
Hydrant Flow Test Procedure

Procedure For One & Two Flow Hydrant Test:

- Establish hydrants closest to location and associated water main(s).
- Static/Residual hydrant (P) should be located close to location (preferably off same main as to provide future water source).
- Flow hydrant(s) (F) should be located off same main up and down stream from mid-point test (static/residual hydrant).
- Note static system pressure of P hydrant before opening any other (note any unusual or remarkable anomalies such as high demand sources, construction, etc.)
- Flow F1 hydrant and record GPM and residual off P hydrant.
- Flow F2 hydrant and record GPM and residual off P hydrant.
- Flow F1 & F2 simultaneously and record GPM separately from F1 and F2 and record P hydrant residual.

Legend:

- F1 & F2 Designation shall represent first and second flowed hydrants respectively
- P Designation shall represent test hydrant for static and residual distribution system pressures.

Date: 4/7/22	Time: 2:36pm	Static Pressure -  58psi
Residual/Static Hydrant Address/Location Residual Pressures		
P - Hydrant FH002267	3090 Sheridan St	F-1 Only 55psi F-2 Only 55psi F-1 & F-2 50psi
Flow Hydrants Address/Location Flow Rate		
F-1 Hydrant (Individual) FH002266	3090 Sheridan St	GPM 1030
F-2 Hydrant (Individual) FH002270	3090 Sheridan St	GPM 1060
F-1 Hydrant (Both Flowing)		GPM 920
F-2 Hydrant (Both Flowing)		GPM 1030

FIRE FLOW REQUIREMENT CALCULATIONS

FIRE FLOW GPM PER TABLE 18.4.5.1.2 = 3750 GPM FOR A TYPE II (000) BUILDING WITH 30,495 SQ. FT

WAREHOUSE STORAGE BUILDING HAS A QUICK RESPONSE FIRE SPRINKLER SYSTEM IN COMPLIANCE WITH NFPA 13 ALLOWING A 75% REDUCTION PER PARAGRAPH 18.4.5.2.2. SEE FIRE SPRINKLER GENERAL DATA FXX.0 FOR FURTHER SPRINKLER DETAILS.

3750 GPM X 75% = 2812.5 GPM

3750 GPM - 2812.5 GPM (FIRE FLOW CREDIT) = 937.5 GPM

937.5 GPM > 600 GPM MINIMUM FOR QUICK RESPONSE SYSTEM

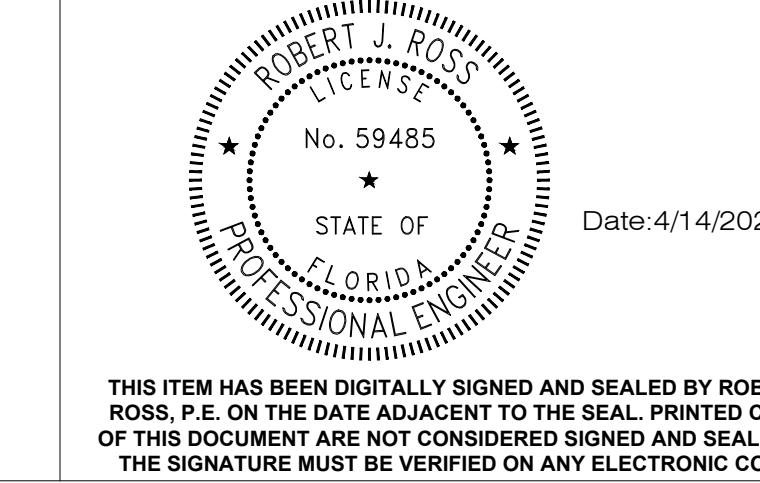
937.5 GPM < FH002266 1030 GPM & FH002270 1060 GPM INDIVIDUAL FLOW

ZT	4/14/23	 3/27/23	CITY OF HOLLYWOOD - TAC SUBMITTAL COMMENTS	
DESIGNED BY	DATE			
ZT	4/14/23			
DRAWN BY	DATE			
RR	4/14/23			
CHECKED BY	DATE			
RR	4/14/23			
APPROVED BY	DATE	No.	DATE	REVISIONS

SAFEGUARD STORAGE
3090 SHERMAN STREET
HOLLYWOOD, FL 33021



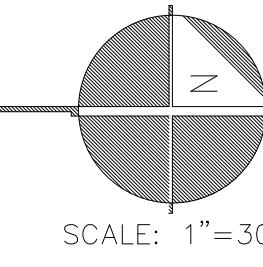
3325 S. UNIVERSITY DRIVE, SUITE 111
DAVIE, FLORIDA 33328
(954)318-0624 (954)358-0190 FAX
CERTIFICATE OF AUTHORIZATION No. 9808



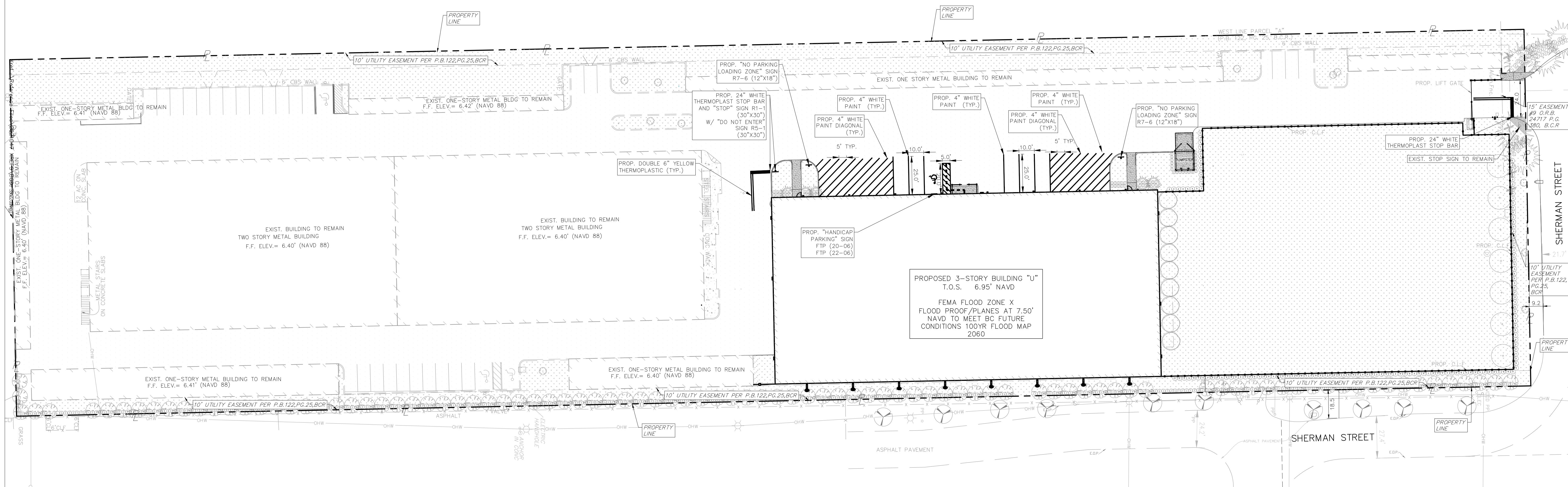
WATER AND SEPTIC PLAN DETAILS (SHEET 2 OF 2)

SCALE: SHEET No. C-4.2

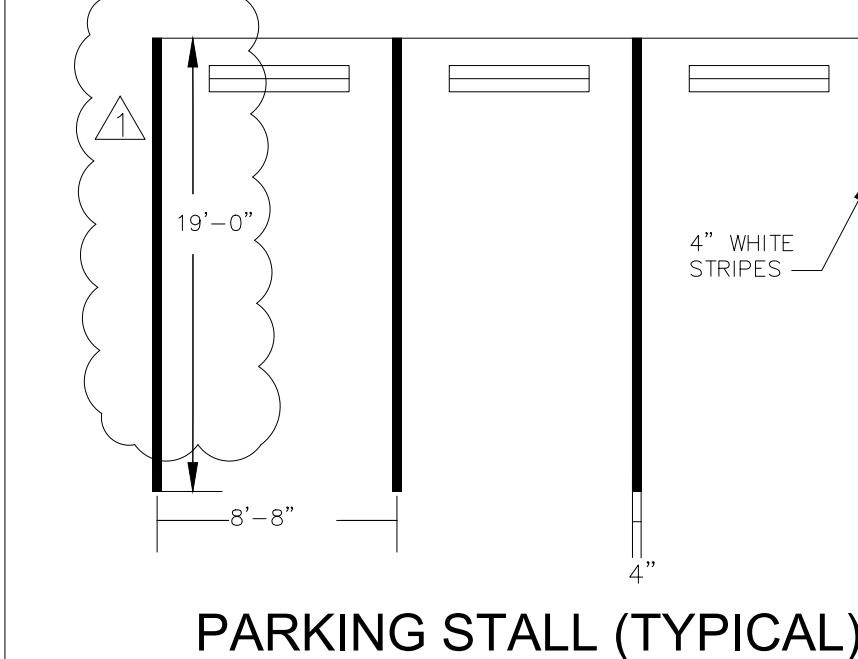
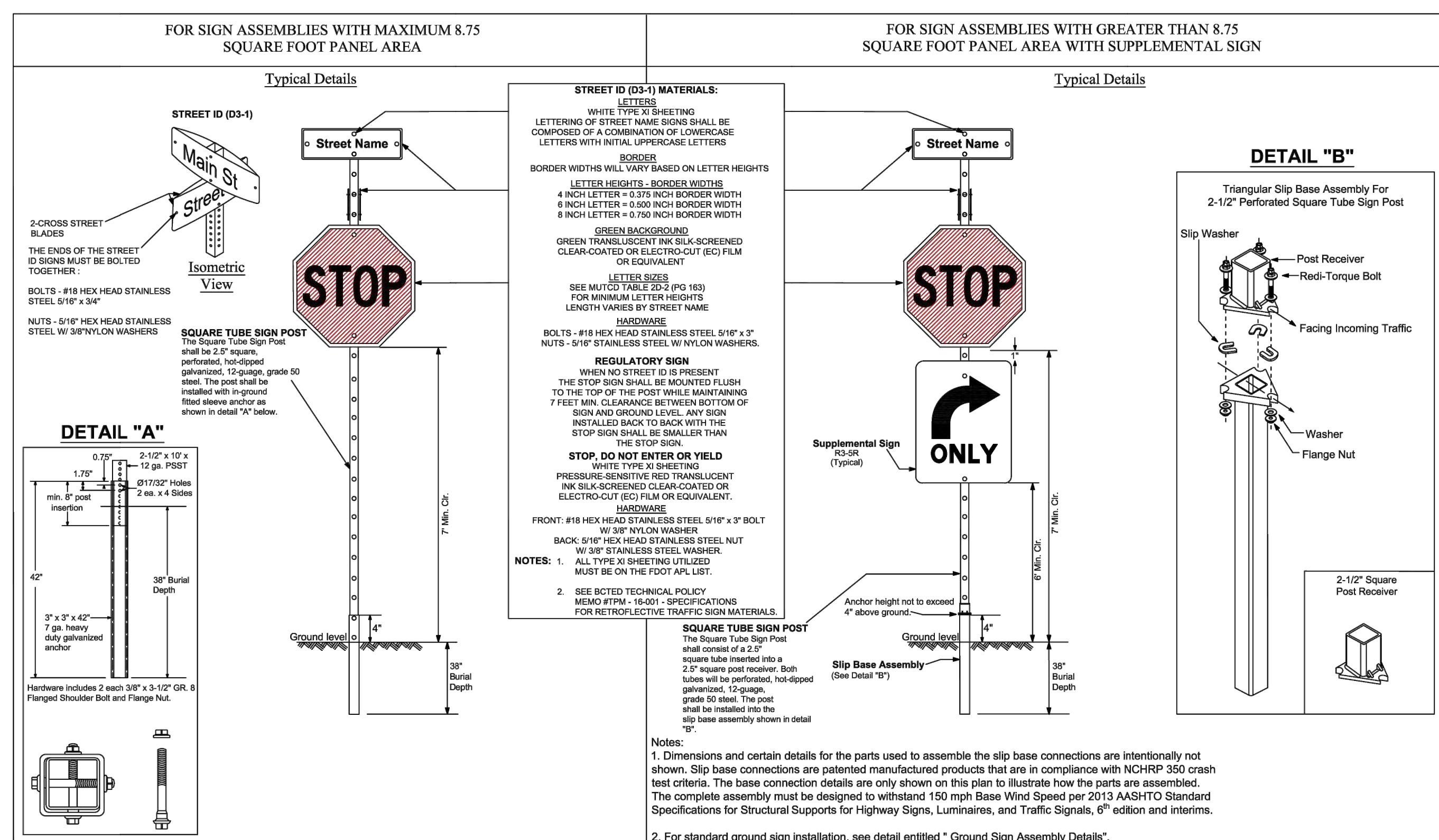
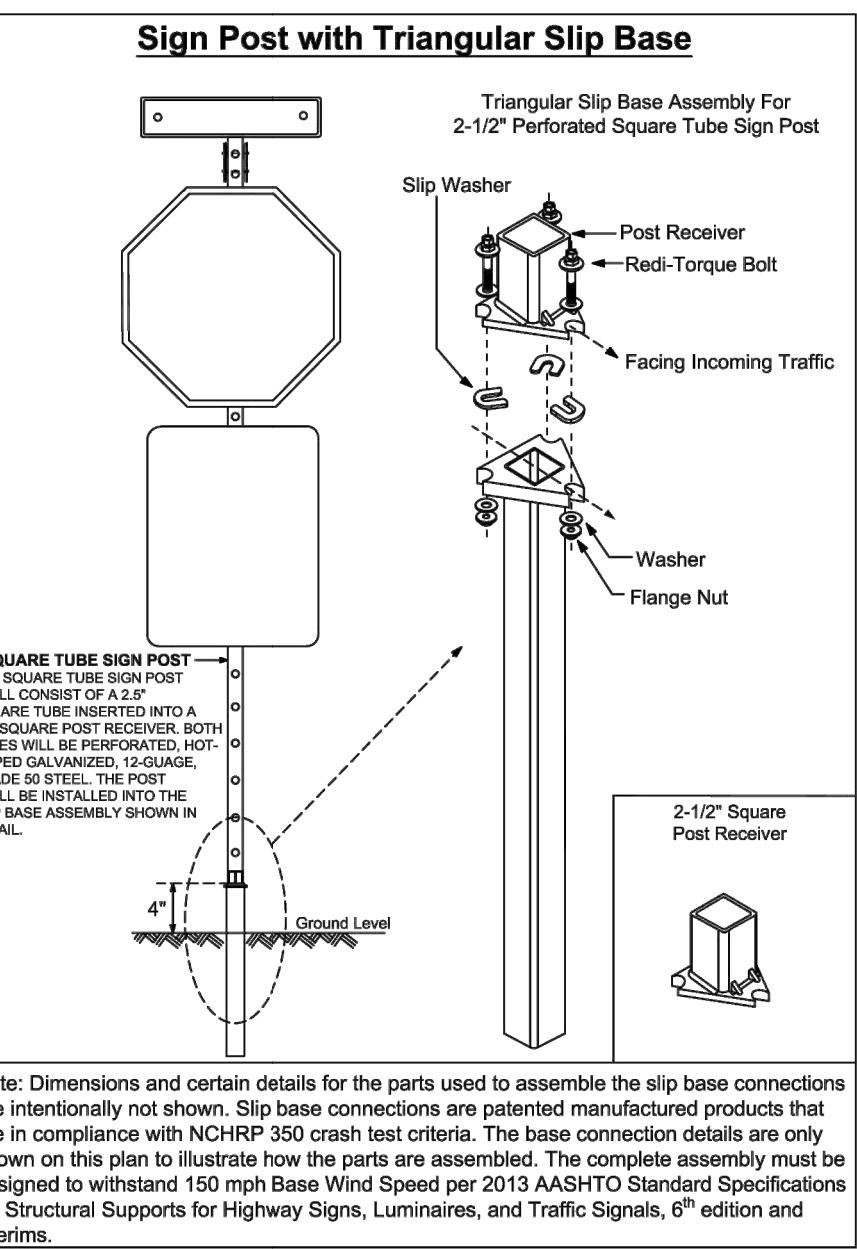
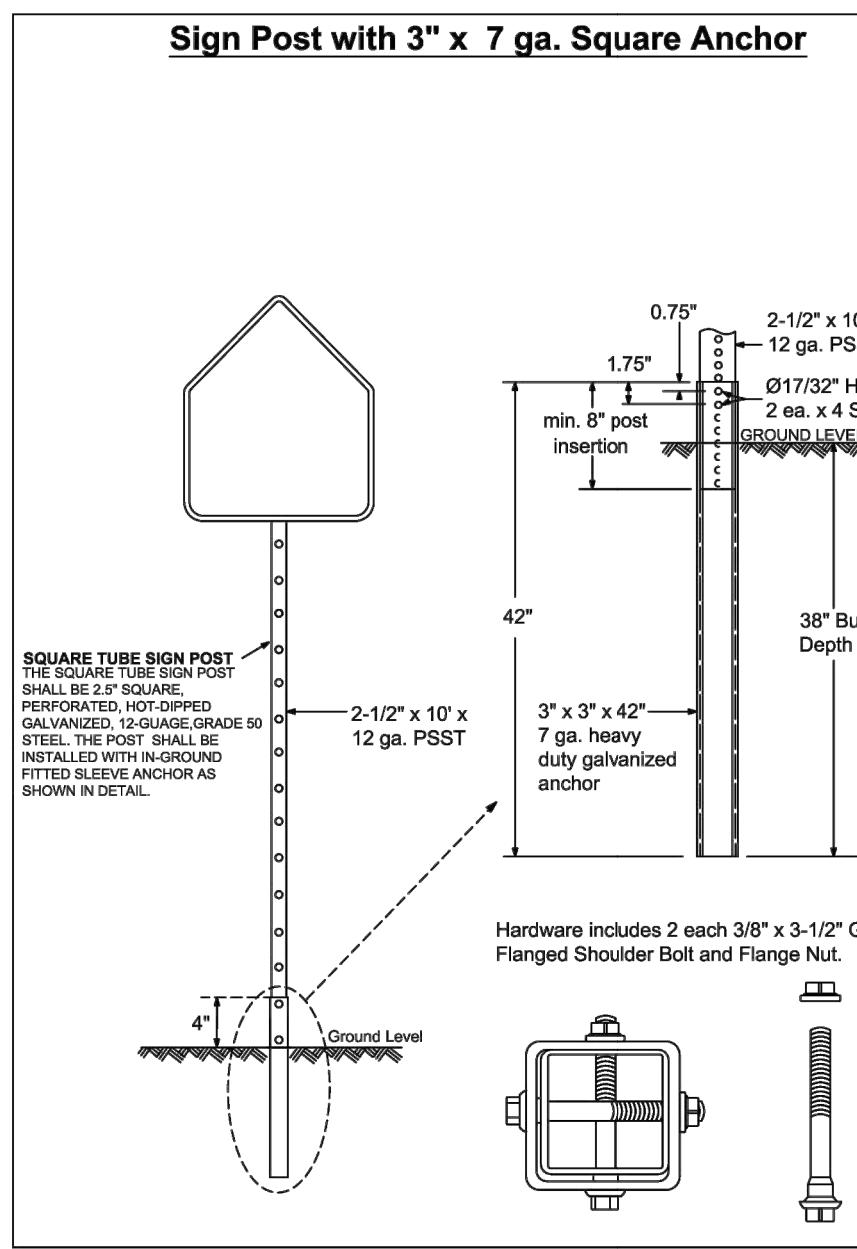
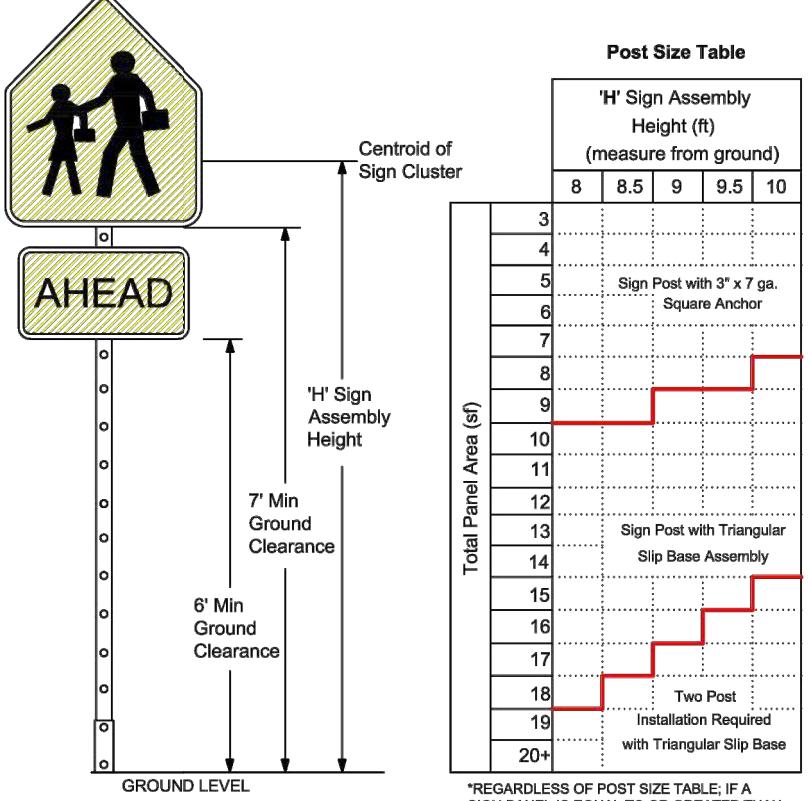
100% CONSTRUCTION DOCUMENTS



Sunshine 811
Call 811 or visit sunshine811.com two full business days before digging to have buried facilities located and marked.
Check positive response codes before you dig!



GUIDE TO USE THIS STANDARD:
 1. Calculate the total sign area and the centroid 'C' for an individual sign or a sign cluster.
 2. Determine the height 'H' from the groundline for the individual sign or the cluster.
 3. Consult the Post Size Table and find the intersection point.
 4. Design the post and the foundation according to the required Post Size and Assembly Details.



PARKING STALL (TYPICAL)

NTS

DATE	DESCRIPTION	BROWARD COUNTY FLORIDA
03-21-2017	UPDATED POST SIZE	
11-24-2020	ADDED POST SIZE NOTE	
02-05-2021	UPDATED POST BASE HEIGHT	

DATE	DESCRIPTION	PUBLIC WORKS DEPARTMENT TRAFFIC ENGINEERING DIVISION
	DESIGN BY: WES D'ANJOU, P.E.	
	DRAWN BY: STEPHON RAMOUTAR	SCALE: NTS
	CHECKED BY: ANDREW SEBO, P.E., PTOE	

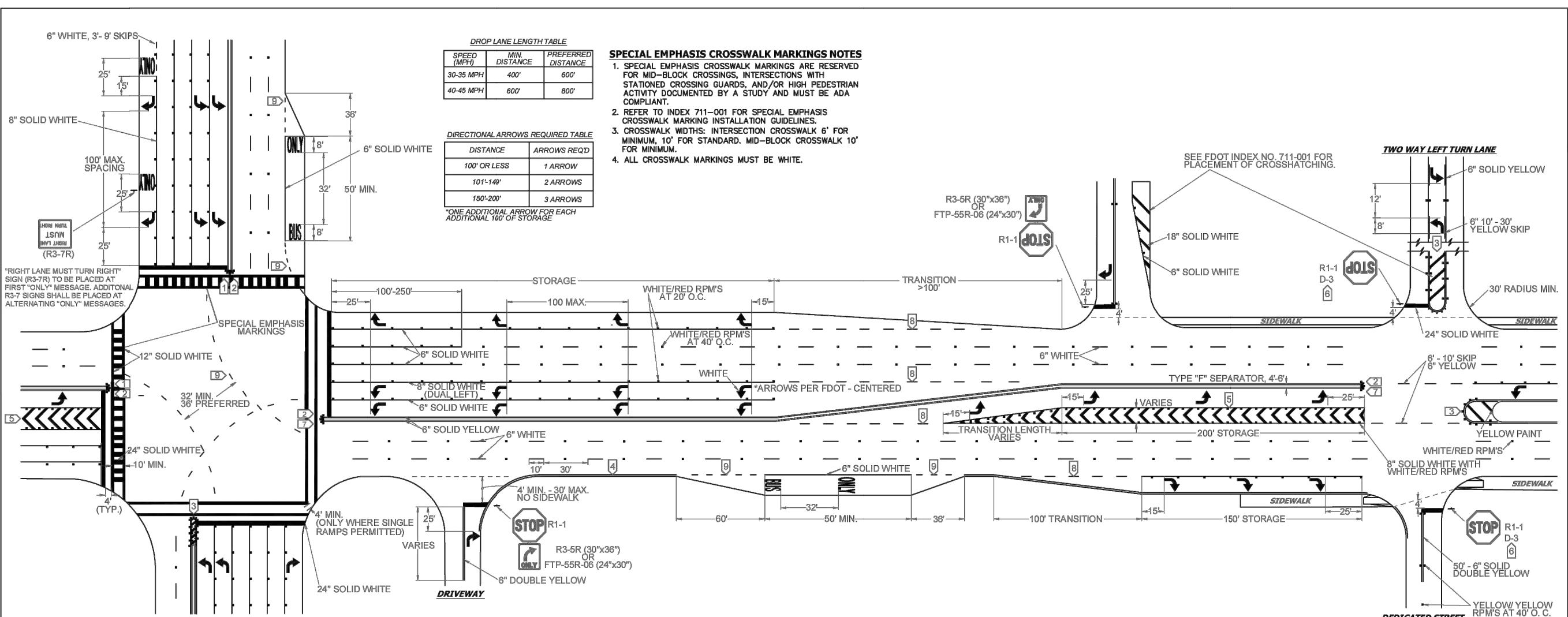
GROUND SIGN ASSEMBLY DETAILS

SHEET NO.
1 OF 1

DATE	DESCRIPTION	BROWARD COUNTY FLORIDA
04-09-2019	UPDATED MATERIAL NOTES	
02-28-2020	ADDED ISOMETRIC VIEW	
02-05-2021	UPDATED POST BASE HEIGHT	

DATE	DESCRIPTION	PUBLIC WORKS DEPARTMENT TRAFFIC ENGINEERING DIVISION
	DESIGN BY: CARMELO CARATOZZOLO, P.E.	
	DRAWN BY: STEPHON RAMOUTAR	SCALE: NTS
	CHECKED BY: ANDREW SEBO, P.E., PTOE	

SHEET NO.
1 OF 1



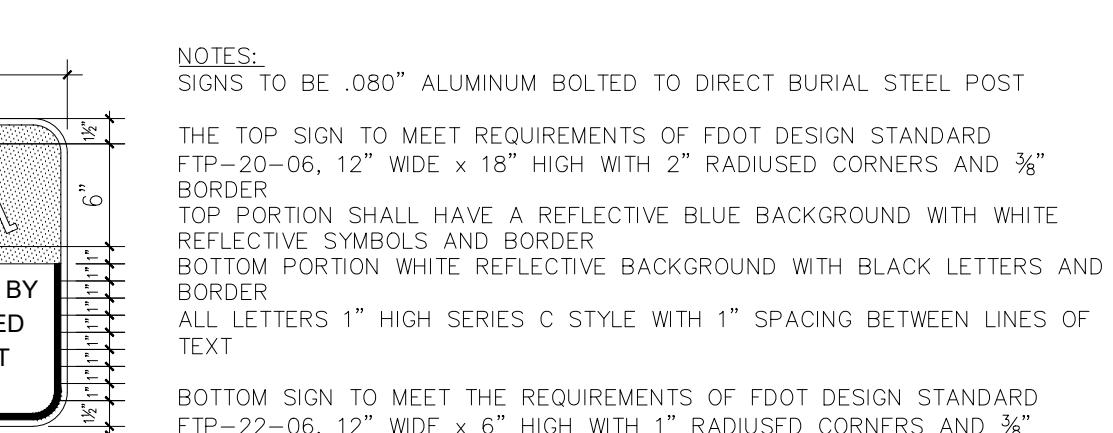
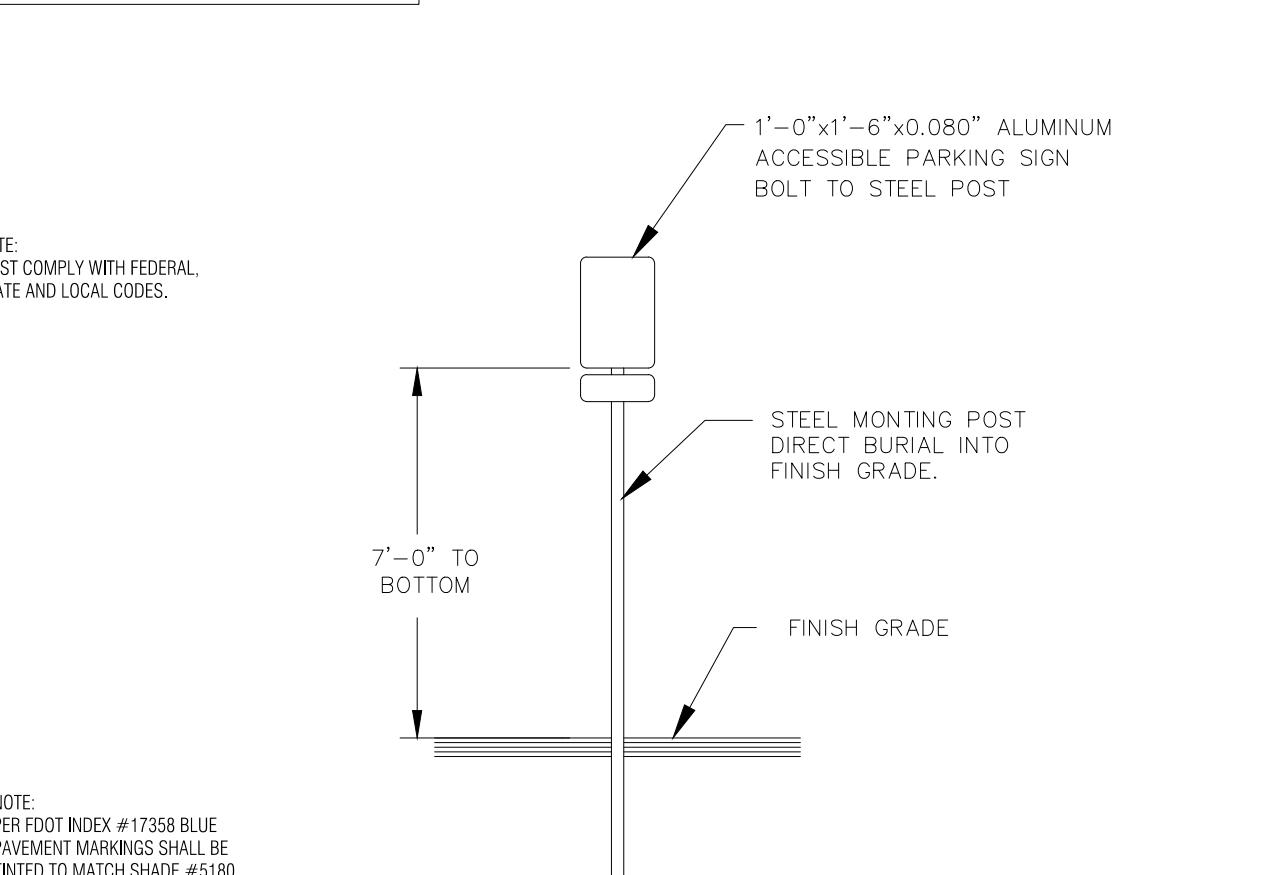
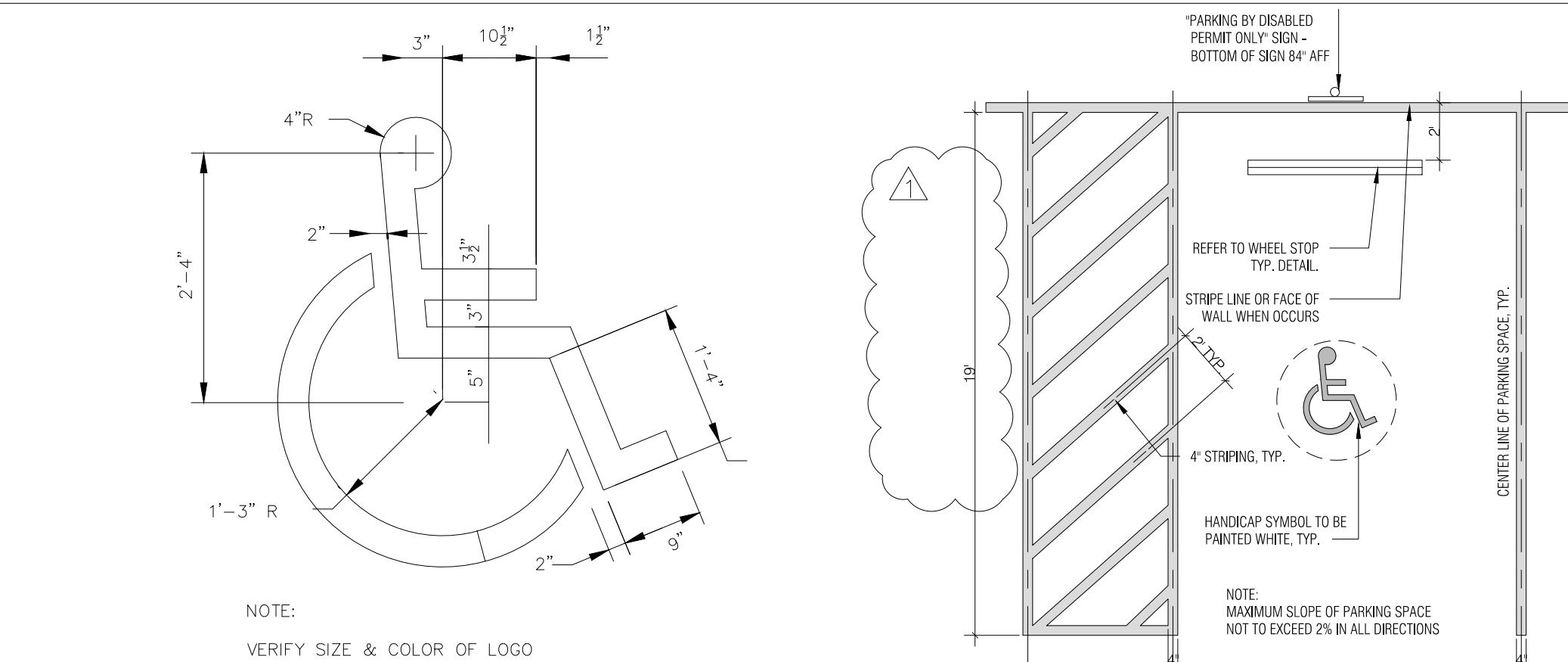
NOTES
 1. ALL PAINTED MARKINGS SHALL BE ALIVE BASED, THERMOPLASTIC AND FULLY RETROREFLECTORIZED.
 2. PAINTED MARKINGS OR PAINT TAPES SHALL BE 3M SOLID SERIES TAPE OR EQUIVALENT AND APPLIED WITH FOAM ADHESIVE AS PER MANUFACTURER'S SPECIFICATIONS.
 3. ALL PAINTED MARKINGS AND SIGNING SHALL BE IN ACCORDANCE WITH THE 'MANUAL ON UNIFORM MARKINGS FOR HIGHWAY USE' (FHWA, 2009) AND THE 'MANUAL ON SIGNING AND MARKING' (FHWA, 2009).
 4. SEE FOOT INDEX NO 700-001 FOR PLACEMENT OF RPM (FOR BULKAGE RPM TREATMENT, SEE LEGEND NO. 1).
 5. RPM SHALL BE CLASS 1B OR EQUIVALENT, APPLIED WITH EPOXY OR BULKAGE ADESIVE.
 6. FOAM APPROVED SEALER SHALL BE USED WHEN APPLYING MARKINGS ON CONCRETE.
 7. FOR BIK LANE DETAILS SEE FOOT INDEX NO. 711-002.
 8. EXIST MARKINGS SHALL BE REMOVED BY WATER WASHING OR SAND BLASTING.
 9. DO NOT RETAIN TO EXIST MARKINGS OR BULKAGE.
 10. PAINTING MARKING REFLECTIVITY SHALL BE UNIFORM ACROSS THE ENTIRE STRIPE AND SHALL HAVE A 10% REFLECTIVITY FOR 250 MILES. FOR WHITE AND 175 MILES. FOR YELLOW.
 11. ALL PRODUCTS MUST BE ON FOAM APPROVED PROCESS (FOAM).
 12. RPM SHALL NOT BE INSTALLED ADJACENT TO BIK LANE.
 13. LONGITUDINAL CROSSWALKS SHALL BE INSTALLED AT ALL CROSS STREETS ON COUNTY ROADS THAT ARE FOUR LANE OR GREATER.

DATE	DESCRIPTION	BROWARD COUNTY FLORIDA
11-24-2020	UPDATED NOTES	
04-14-2021	UPDATED NOTES	
05-06-2022	UPDATED NOTES	

DATE	DESCRIPTION	PUBLIC WORKS DEPARTMENT TRAFFIC ENGINEERING DIVISION
	DESIGN BY: CARMELO CARATOZZOLO, P.E.	
	DRAWN BY: STEPHON RAMOUTAR	SCALE: NTS
	CHECKED BY: CARMELO CARATOZZOLO, P.E.	

PAVEMENT MARKINGS AND SIGNS DETAILS

SHEET NO.
1 OF 1



NOTES: SIGNS TO BE .080" ALUMINUM BOLTED TO DIRECT BURIAL STEEL POST

THE TOP SIGN TO MEET REQUIREMENTS OF FDOT DESIGN STANDARD FTP-20-06, 12" WIDE x 18" HIGH WITH 2" RADIIED CORNERS AND $\frac{3}{8}$ " BORDER
 TOP PORTION SHALL HAVE A REFLECTIVE SYMBOLS AND BORDER
 BOTTOM PORTION WHITE REFLECTIVE BACKGROUND WITH BLACK LETTERS AND BORDER
 ALL LETTERS 1" HIGH SERIES C STYLE WITH 1" SPACING BETWEEN LINES OF TEXT

BOTTOM SIGN TO MEET THE REQUIREMENTS OF FDOT DESIGN STANDARD FTP-22-06, 12" WIDE x 6" HIGH WITH 1" RADIIED CORNERS AND $\frac{3}{8}$ " BORDER
 WHITE REFLECTIVE BACKGROUND WITH BLACK LETTERS AND BORDER
 ALL LETTERS 1" HIGH SERIES C STYLE WITH 1" SPACING BETWEEN LINES OF TEXT

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BOTTOM PORTION WHITE REFLECTIVE BACKGROUND WITH BLACK LETTERS AND BORDER

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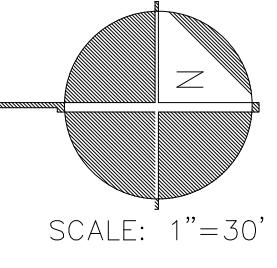
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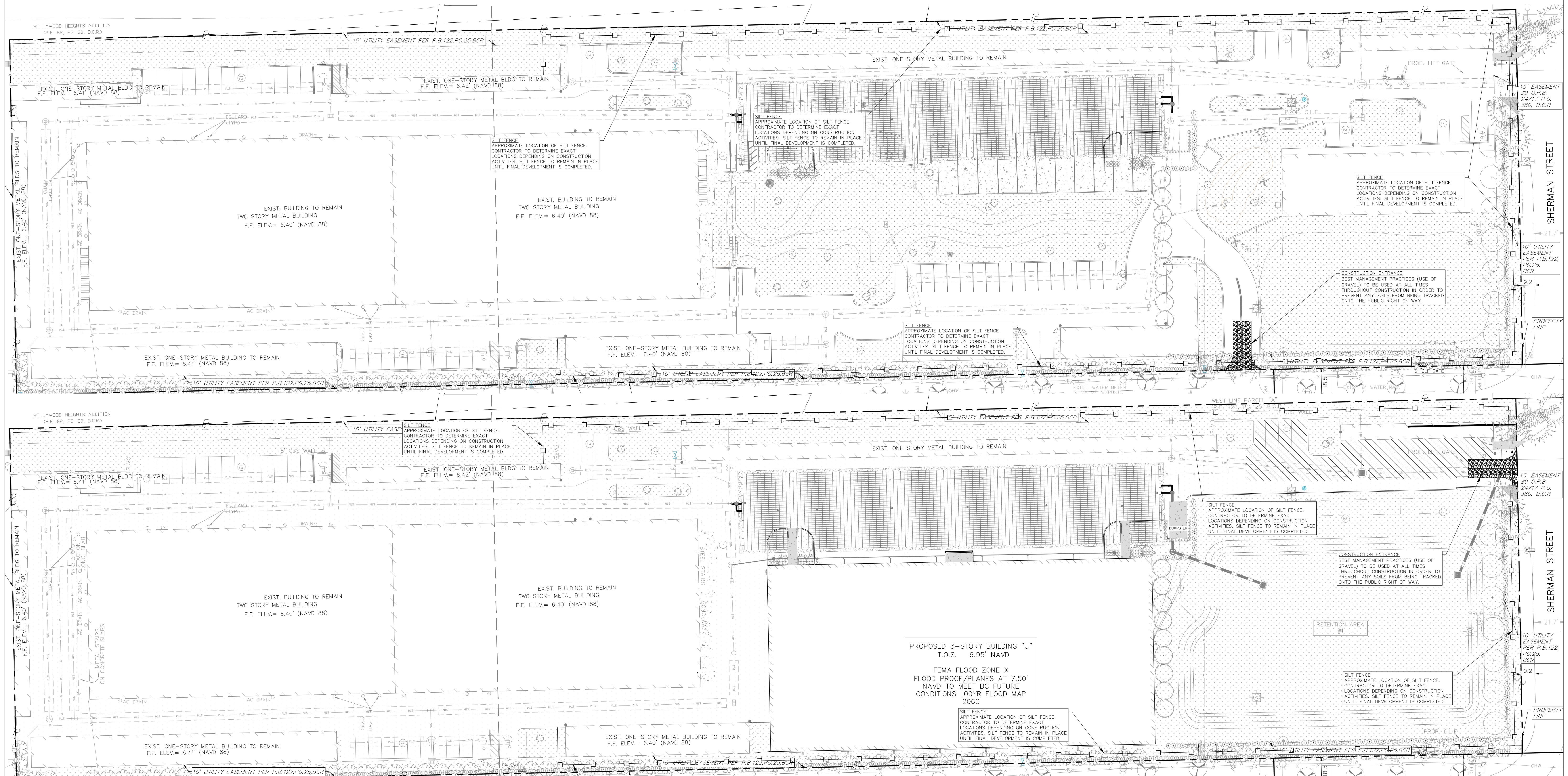


Sunshine 811
Call 811 or visit sunshine811.com two full business days before digging to have buried facilities located and marked.
Check positive response codes before you dig!

LEGEND



SCALE: 1" = 30'



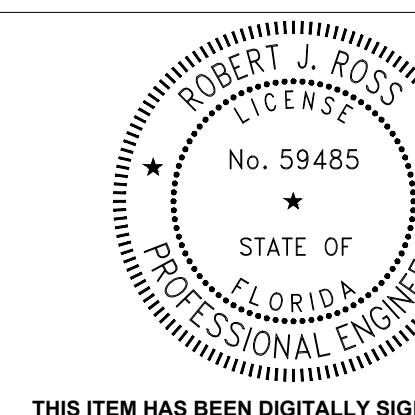
100% CONSTRUCTION DOCUMENTS

ZT	4/14/23		3/27/23	CITY OF HOLLYWOOD – TAC SUBMITTAL COMMENTS
DESIGNED BY	DATE			
ZT	4/14/23			
DRAWN BY	DATE			
MR	4/14/23			
CHECKED BY	DATE			
RR	4/14/23			
APPROVED BY	DATE	No.	DATE	REVISIONS

SAFEGUARD STORAGE
3090 SHERMAN STREET
HOLLYWOOD, FL 33021



33230 SW 111 & 112 VERSATILE REVIEWS SUITE 21411
DIAMOND, FLORIDA 33328
(986)468-8624 (354)352-0099 FAX
CERTIFICATE OF AUTHORIZATION No. 9808



Date: 1/1/2020

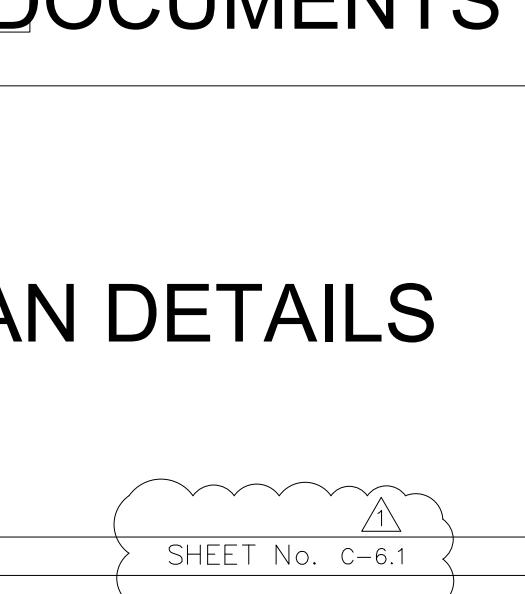
POLLUTION PREVENTION CONTROL PLAN

THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY ROBERT J. ROSS, P.E. ON THE DATE ADJACENT TO THE SEAL. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

SCALE 1" = 30'

SHEET No. C-6

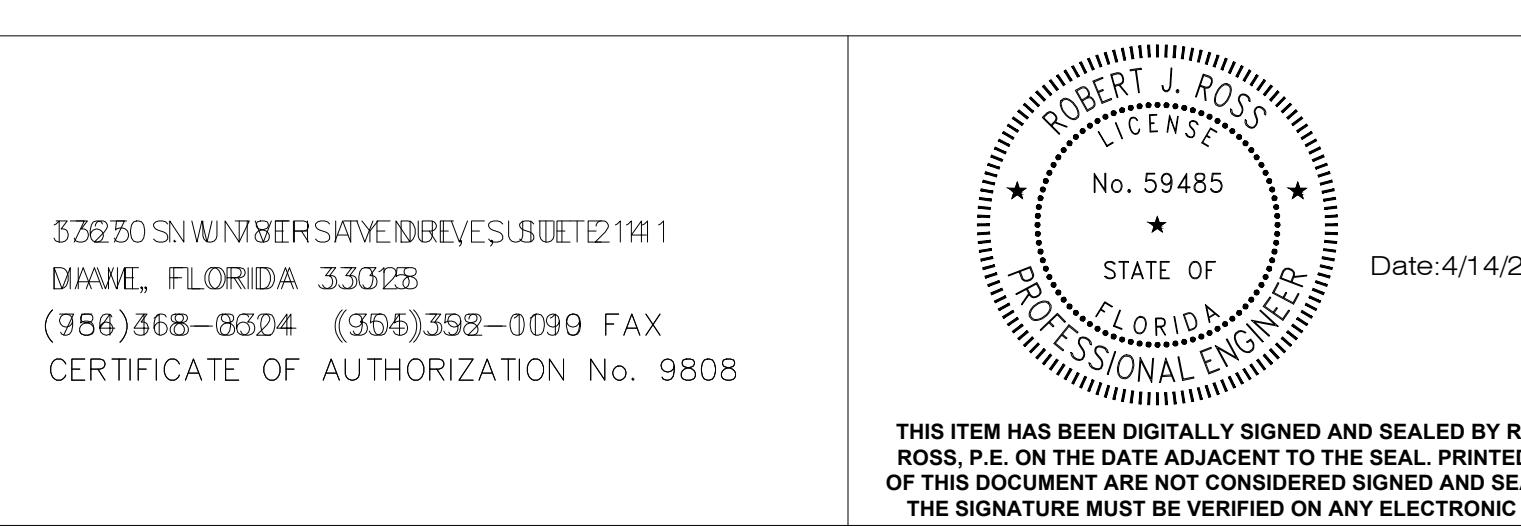
STORM WATER POLLUTION PREVENTION PLAN									
SITE DESCRIPTION		GENERAL							
PROJECT NAME AND LOCATION: SAFEGUARD STORAGE 3090 SHERMAN ST, HOLLYWOOD, FL 33021		<p>THE CONTRACTOR SHALL AT A MINIMUM IMPLEMENT THE CONTRACTOR'S REQUIREMENTS OUTLINED BELOW AND THOSE MEASURES SHOWN ON THE EROSION AND TURBIDITY CONTROL PLAN. IN ADDITION THE CONTRACTOR SHALL UNDERTAKE ADDITIONAL MEASURES REQUIRED TO BE IN COMPLIANCE WITH APPLICABLE PERMIT CONDITIONS AND STATE WATER QUALITY STANDARDS. DEPENDING ON THE NATURE OF MATERIALS AND METHODS OF CONSTRUCTION THE CONTRACTOR MAY BE REQUIRED TO ADD FLOCULANTS TO THE RETENTION SYSTEM PRIOR TO PLACING THE SYSTEM INTO OPERATION.</p>							
OWNER NAME AND ADDRESS: CITY OF PEMBROKE PINES 601 CITY CENTER WAY PEMBROKE PINES, FLORIDA 33025		<p>THE NORTH 985.83 FEET OF PARCEL "A", TAFT STREET INDUSTRIAL PARK, ACCORDING TO THE PLAT THEREOF, AS RECORDED IN PLAT BOOK 122, PAGE 25, OF THE PUBLIC RECORDS OF BROWARD COUNTY, FLORIDA.</p> <p>SAID LANDS LYING IN THE CITY OF HOLLYWOOD, BROWARD COUNTY, FLORIDA AND CONTAINING NET TOTAL OF 228.71256 SQUARE FEET (5.25 ACRES) MORE OR LESS.</p> <p>THE STORM DRAINAGE SYSTEM WILL CONSIST OF IMPROVEMENTS, WHICH WILL REQUIRE FILLING OF THE SITE TO THE PROPOSED GRADES AND REQUIRE EXCAVATION OF COLLECTION/CONVEYANCE SWALES, MINOR CONSTRUCTION OF YARD DRAINAGE SYSTEM IMPROVEMENTS WHICH WILL CONVEY TO EXISTING DRAINAGE SYSTEM STRUCTURES AND DISCHARGE TO EXISTING LAKES WITHIN THE S.B.D.D.'S BASIN SURFACE WATER SYSTEM.</p>							
DESCRIPTION: SOIL DISTURBING ACTIVITIES: SOIL DISTURBING ACTIVITIES WILL INCLUDE, CLEARING AND GRUBBING, INSTALLING A STABILIZED CONSTRUCTION ENTRANCE, GRADING, STORM DRAINAGE SYSTEM, IMPROVEMENTS PARKING AREAS AND PREPARATION FOR PLANTING AND SEEDING.		<p>SEQUENCE OF MAJOR ACTIVITIES:</p> <p>1. INSTALL EROSION AND SEDIMENT CONTROL MEASURES; 2. CLEAR AND GRUB SITE; 3. START BUILDING FOUNDATION; 4. CONDUCT EROSION CONTROL MEASURES; 5. INSTALL UNDERGROUND UTILITIES; 6. BRING SITE TO GRADE AND INSTALL CURBING; 7. CONTINUE WITH E&S CONTROL MEASURES; 8. INSTALL BASE COURSE; 9. COMPLETE FINAL GRADING OPERATIONS; 10. CONDUCT EROSION CONTROL OPERATIONS; 11. CONTINUE WITH E&S CONTROL MEASURES; 12. COMPLETE BUILDING; 13. REMOVE ACCUMULATED SEDIMENTS FROM STORM WATER MANAGEMENT SYSTEM.</p>							
SITE MAPS: REFER TO TOPOGRAPHIC SURVEYOR PLAN		<p>SEQUENCE OF MAJOR ACTIVITIES:</p> <p>1. INSTALL EROSION AND SEDIMENT CONTROL MEASURES; 2. CLEAR AND GRUB SITE; 3. START BUILDING FOUNDATION; 4. CONDUCT EROSION CONTROL MEASURES; 5. INSTALL UNDERGROUND UTILITIES; 6. BRING SITE TO GRADE AND INSTALL CURBING; 7. CONTINUE WITH E&S CONTROL MEASURES; 8. INSTALL BASE COURSE; 9. COMPLETE FINAL GRADING OPERATIONS; 10. CONDUCT EROSION CONTROL OPERATIONS; 11. CONTINUE WITH E&S CONTROL MEASURES; 12. COMPLETE BUILDING; 13. REMOVE ACCUMULATED SEDIMENTS FROM STORM WATER MANAGEMENT SYSTEM.</p>							
SITE AREA: 1. TOTAL AREA OWNED: 5.25 AC (PROJECT AREA)		<p>THE PROPOSED SYSTEM WILL CONVEY STORM WATER RUNOFF VIA RAIN TANKS AND PERFORATED PIPES TO EXISTING EXFILTRATION TRENCH AND TO ON SITE DRY RETENTION AREAS.</p>							
CONTROLS		<p>THIS PLAN UTILIZES BEST MANAGEMENT PRACTICES TO CONTROL EROSION AND TURBIDITY CAUSED BY STORM WATER RUNOFF. AN EROSION AND TURBIDITY PLAN HAS BEEN PREPARED TO INSTRUCT THE CONTRACTOR ON PLACEMENT OF THESE CONTROLS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO INSTALL AND MAINTAIN THE CONTROLS PER PLAN AS WELL AS ENSURE THE PLAN IS PROVIDING THE PROPER PROTECTION AS REQUIRED BY FEDERAL, STATE AND LOCAL LAWS. REFER TO "CONTRACTOR'S RESPONSIBILITY" FOR A VERBAL DESCRIPTION OF THE CONTROLS THAT MAY BE IMPLEMENTED.</p> <p>STORM WATER MANAGEMENT: STORM WATER DRAINAGE WILL BE PROVIDED BY A POSITIVE DRAINAGE COLLECTION SYSTEM. THE PROPOSED SYSTEM WILL CONVEY STORM WATER RUNOFF VIA RAIN TANK, PERFORATED PIPES TO EXISTING EXFILTRATION TRENCH AND TO DRY RETENTION AREAS.</p> <p>FOR THE PROJECT, AREAS WHICH ARE NOT TO BE CONSTRUCTED ON, BUT WILL BE REGRADED, SHALL BE STABILIZED IMMEDIATELY AFTER GRADING IS COMPLETE, WHEN CONSTRUCTION IS COMPLETE, A TOTAL OF 16± ACRES WILL HAVE BEEN REGRADED. WHERE PRACTICAL, TEMPORARY SEDIMENT BINS WILL BE USED TO INTERCEPT SEDIMENT BEFORE ENTERING THE ON-SITE DRAINAGE SYSTEM. THIS IS IN ACCORDANCE WITH THE REQUIREMENTS SET FORTH BY FEDERAL, STATE AND LOCAL REGULATIONS.</p>							
TIMING OF CONTROLS/MEASURES		<p>AS INDICATED IN THE SEQUENCE OF MAJOR ACTIVITIES, THE SILT FENCES AND HAY BALES, STABILIZED CONSTRUCTION ENTRANCE AND SEDIMENT BASIN WILL BE CONSTRUCTED PRIOR TO CLEARING OR GRADING OF ANY OTHER PORTIONS OF THE SITE. STABILIZATION MEASURES SHALL BE INITIATED AS SOON AS PRACTICAL IN PORTIONS OF THE SITE WHERE CONSTRUCTION ACTIVITIES HAVE TEMPORARILY OR PERMANENTLY CEASED. ONCE CONSTRUCTION ACTIVITY CEASES PERMANENTLY IN AN AREA, THAT AREA WILL BE STABILIZED PERMANENTLY IN ACCORDANCE WITH THE PLANS. AFTER THE ENTIRE SITE IS STABILIZED, THE ACCUMULATED SEDIMENT WILL BE REMOVED FROM THE SEDIMENT TRAPS AND THE EARTH DIKE/SWALES WILL BE REGRADED/REMOVED AND STABILIZED IN ACCORDANCE WITH THE EROSION & TURBIDITY CONTROL PLAN.</p> <p>ENDANGERED SPECIES AND CRITICAL HABITAT: 1. ARE THERE ENDANGERED SPECIES ON SITE? <input type="checkbox"/> NONE KNOWN 2. ARE THERE CRITICAL HABITAT ON SITE? <input type="checkbox"/> NONE KNOWN IF YES TO EITHER QUESTION, PLEASE EXPLAIN: _____</p>							
CONTROLS		<p>IT IS THE CONTRACTOR'S RESPONSIBILITY TO IMPLEMENT THE EROSION AND TURBIDITY CONTROLS AS SHOWN ON THE GRADING, DRAINAGE & EROSION CONTROL PLAN. IT IS ALSO THE CONTRACTOR'S RESPONSIBILITY TO ENSURE THESE CONTROLS ARE PROPERLY INSTALLED, MAINTAINED AND FUNCTIONING PROPERLY TO PREVENT TURBID OR POLLUTED WATER FROM LEAVING THE PROJECT SITE AND CONTROLLING ADDITIONAL EROSION AND TURBIDITY CONTROLS SHOWN ON THE GRADING, DRAINAGE & EROSION CONTROL PLAN AND ADD ADDITIONAL CONTROL MEASURES, AS REQUIRED, TO ENSURE THE SITE MEETS ALL FEDERAL, STATE AND LOCAL EROSION AND TURBIDITY CONTROL REQUIREMENTS. THE FOLLOWING BEST MANAGEMENT PRACTICES WILL BE IMPLEMENTED BY THE CONTRACTOR IN RELATION TO THE GRADING, DRAINAGE & EROSION CONTROL PLAN AND AS REQUIRED TO MEET THE EROSION AND TURBIDITY REQUIREMENTS IMPOSED ON THE PROJECT SITE BY THE REGULATORY AGENCIES.</p> <p>EROSION AND SEDIMENT CONTROLS: STABILIZATION PRACTICES (IF APPLICABLE): 1. HAY BALE BARRIER: HAY BALE BARRIERS CAN BE USED BELOW DISTURBED AREAS SUBJECT TO SHEET AND RILL EROSION WITH THE FOLLOWING LIMITATIONS: A. WHERE THE MAXIMUM SLOPE BEHIND THE BARRIER IS 33 PERCENT. B. IN MINOR SWALES OR DITCH LINES WHERE THE MAXIMUM CONTRIBUTING DRAINAGE AREA IS NO GREATER THAN 2 ACRES. C. WHERE EFFECTIVENESS IS REQUIRED FOR LESS THAN 3 MONTHS. D. EVERY EFFORT SHOULD BE MADE TO LIMIT THE USE OF STRAW BALES CONSTRUCTED IN LIVE STREAMS OR IN SWALES WHERE THERE IS THE POSSIBILITY OF A WASHOUT. IF NECESSARY, MEASURES SHALL BE TAKEN TO PROPERLY ANCHOR BALES TO INSURE AGAINST WASHOUT. REFER TO EROSION CONTROL DETAILS FOR CONSTRUCTING THE HAY BALE BARRIER. ALSO REFER TO THE GRADING, DRAINAGE & EROSION CONTROL PLAN FOR PROPER LOCATION. 2. FILTER FABRIC BARRIER: FILTER FABRIC BARRIERS CAN BE USED BELOW DISTURBED AREAS SUBJECT TO SHEET AND RILL EROSION WITH THE FOLLOWING LIMITATIONS: A. WHERE THE MAXIMUM SLOPE BEHIND THE BARRIER IS 33 PERCENT. B. IN MINOR SWALES OR DITCH LINES WHERE THE MAXIMUM CONTRIBUTING DRAINAGE AREA IS NO GREATER THAN 2 ACRES. REFER TO THE EROSION CONTROL DETAILS FOR PROPER CONSTRUCTION OF THE FILTER FABRIC BARRIER.</p>							
TIMING OF CONTROLS/MEASURES		<p>REFER TO " CONTRACTOR'S RESPONSIBILITY" FOR THE TIMING OF CONTROL/MEASURES.</p> <p>CERTIFICATION OF COMPLIANCE WITH FEDERAL, STATE AND LOCAL REGULATIONS</p> <p>IN ACCORDANCE WITH FEDERAL, STATE AND LOCAL LAWS RELATED TO STORM WATER MANAGEMENT AND EROSION AND TURBIDITY CONTROLS, THE FOLLOWING PERMITS HAVE BEEN OBTAINED.</p> <p>SWFMD PERMIT # _____</p> <p>NPDES PERMIT # _____</p> <p>POLLUTION PREVENTION PLAN CERTIFICATION</p> <p>I CERTIFY UNDER PENALTY OF LAW THAT THIS DOCUMENT AND ALL ATTACHMENTS WERE PREPARED UNDER MY DIRECTION OR SUPERVISION IN ACCORDANCE WITH A SYSTEM DESIGNED TO ENSURE THAT QUALIFIED PERSONNEL PROPERLY GATHERED AND EVALUATED THE INFORMATION SUBMITTED. BASED ON MY INQUIRY OF THE PERSON OR PERSONS WHO MANAGE THE SYSTEM, OR THOSE PERSONS DIRECTLY RESPONSIBLE FOR GATHERING THE INFORMATION, THE INFORMATION SUBMITTED IS, TO THE BEST OF MY KNOWLEDGE AND BELIEF, TRUE, ACCURATE, AND COMPLETE. I AM AWARE THAT THERE ARE SIGNIFICANT PENALTIES FOR SUBMITTING FALSE INFORMATION, INCLUDING THE POSSIBILITY OF FINE AND IMPRISONMENT FOR KNOWING VIOLATIONS.</p>							

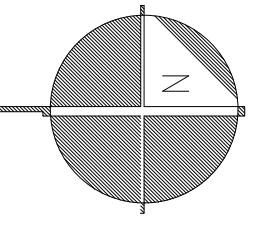


ZT	4/14/23	<input checked="" type="checkbox"/>	3/27/23	CITY OF HOLLYWOOD – TAC SUBMITTAL COMMENTS
DESIGNED BY	DATE			
ZT	4/14/23			
DRAWN BY	DATE			
MR	4/14/23			
CHECKED BY	DATE			
RR	4/14/23			
APPROVED BY	DATE	No.	DATE	REVISIONS

SAFEGUARD STORAGE
3090 SHERMAN STREET
HOLLYWOOD, FL 33021

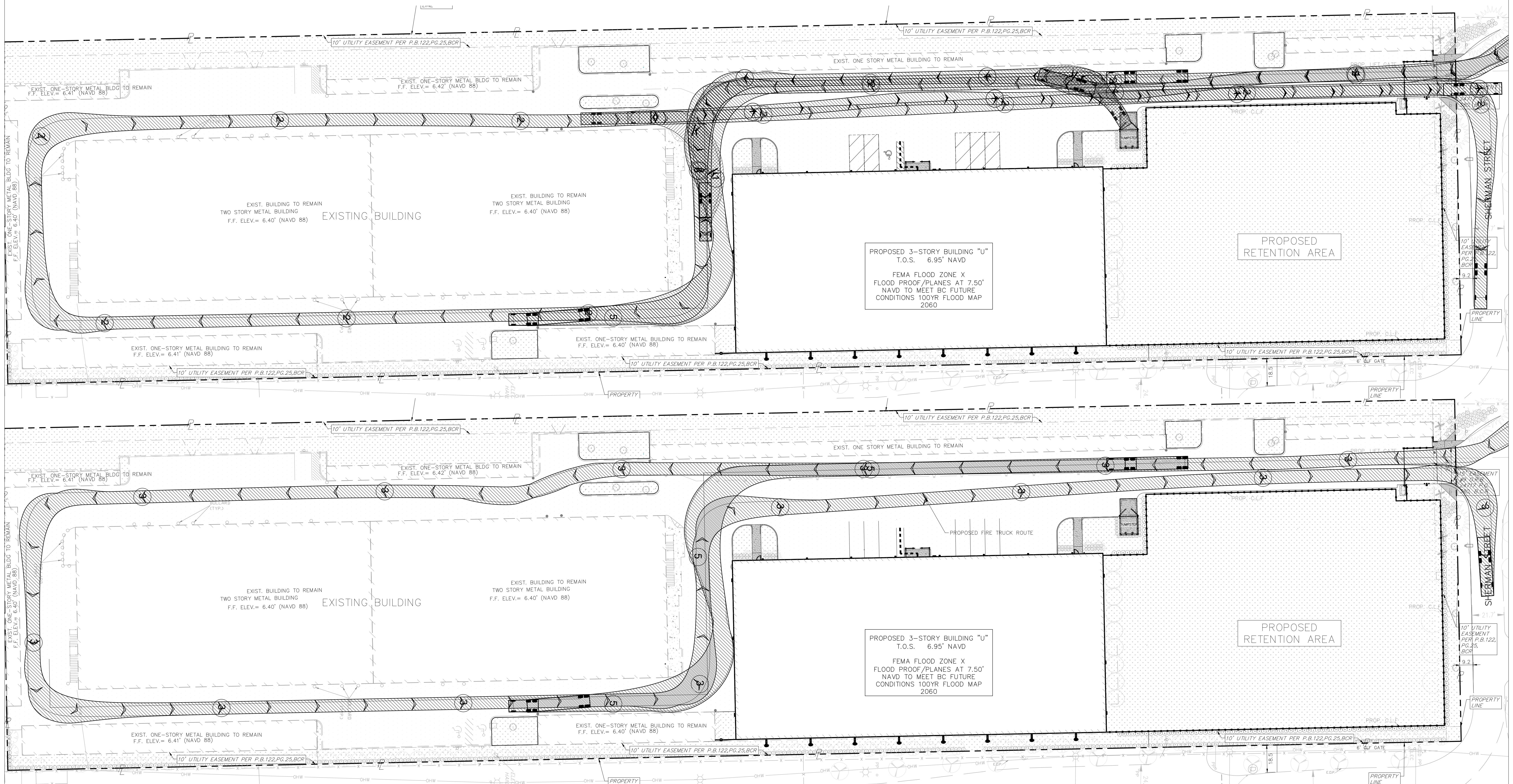
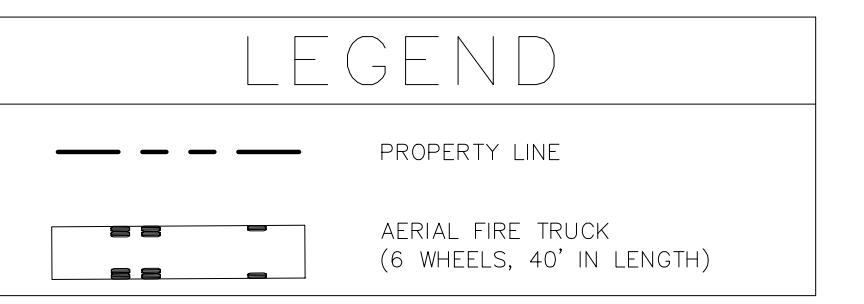
RE
ROSS ENGINEERING, INC.





11 or visit sunshine811.com two full business days before digging to have underground facilities located and marked. *positive response codes before you dig*

LEGEND



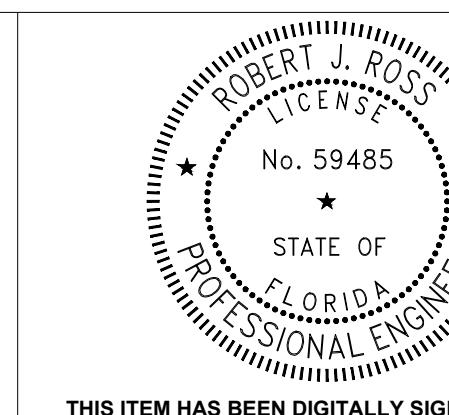
RK	4/14/23		3/27/23	CITY OF HOLLYWOOD – TAC SUBMITTAL COMMENTS		
DESIGNED BY	DATE					
RK	4/14/23					
DRAWN BY	DATE					
RR	4/14/23					
CHECKED BY	DATE					
RR	4/14/23					
APPROVED BY	DATE	No.	DATE	REVISIONS		

SAFEGUARD STORAGE

3090 SHERMAN STREET
HOLLYWOOD, FL 33021



3325 S. UNIVERSITY DRIVE, SUITE 111
DAVIE, FLORIDA 33328
(954)318-0624 (954)358-0190 FAX
CERTIFICATE OF AUTHORIZATION No. 9808

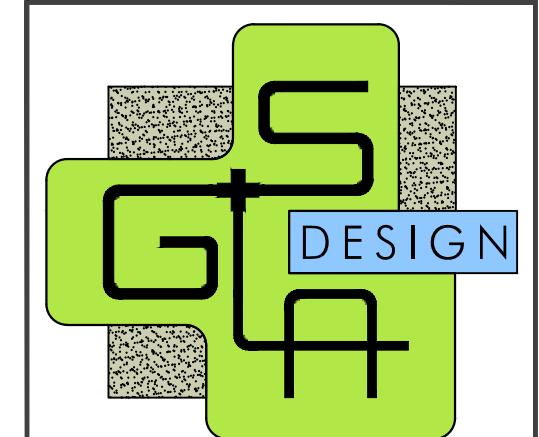
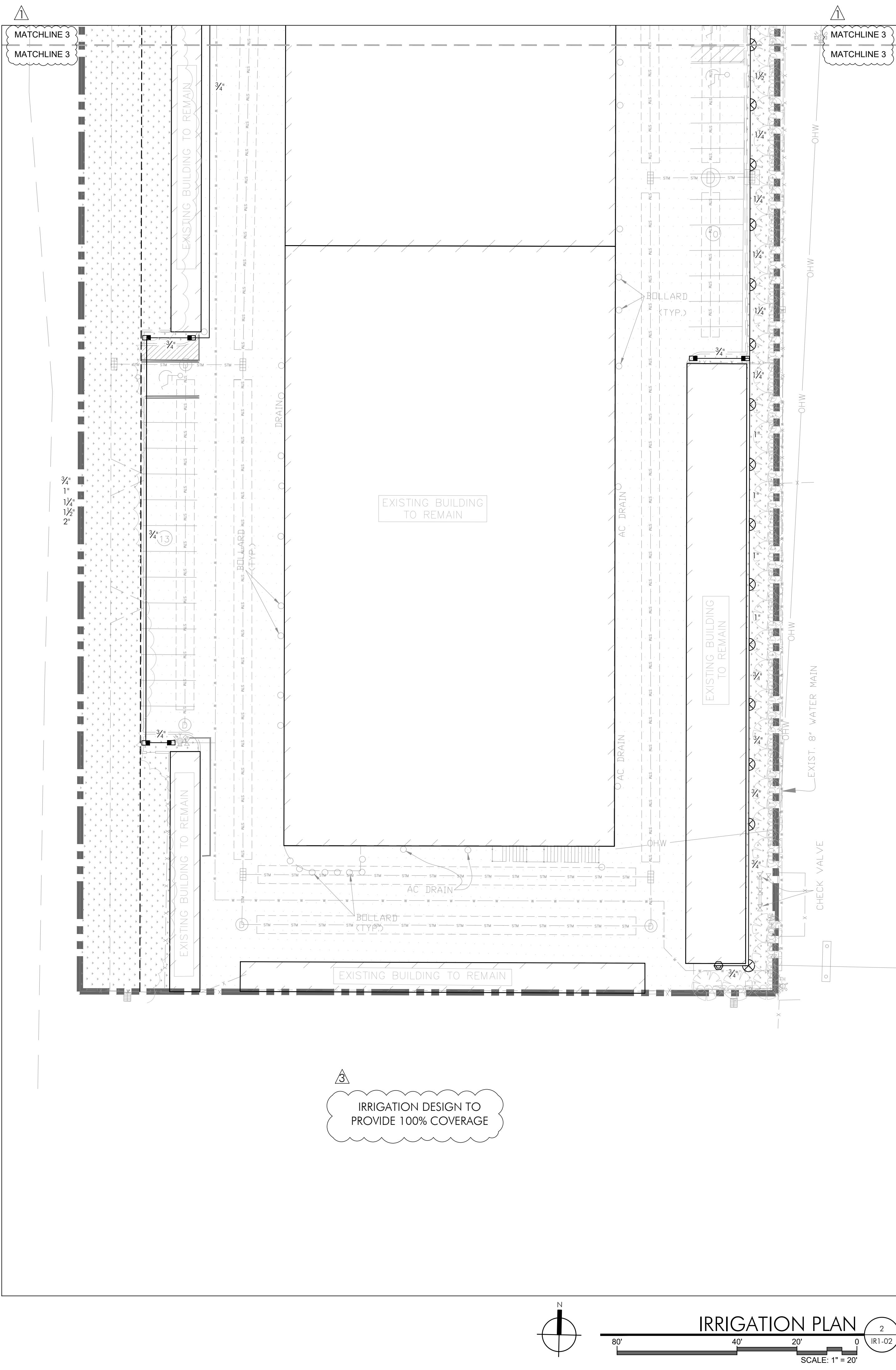
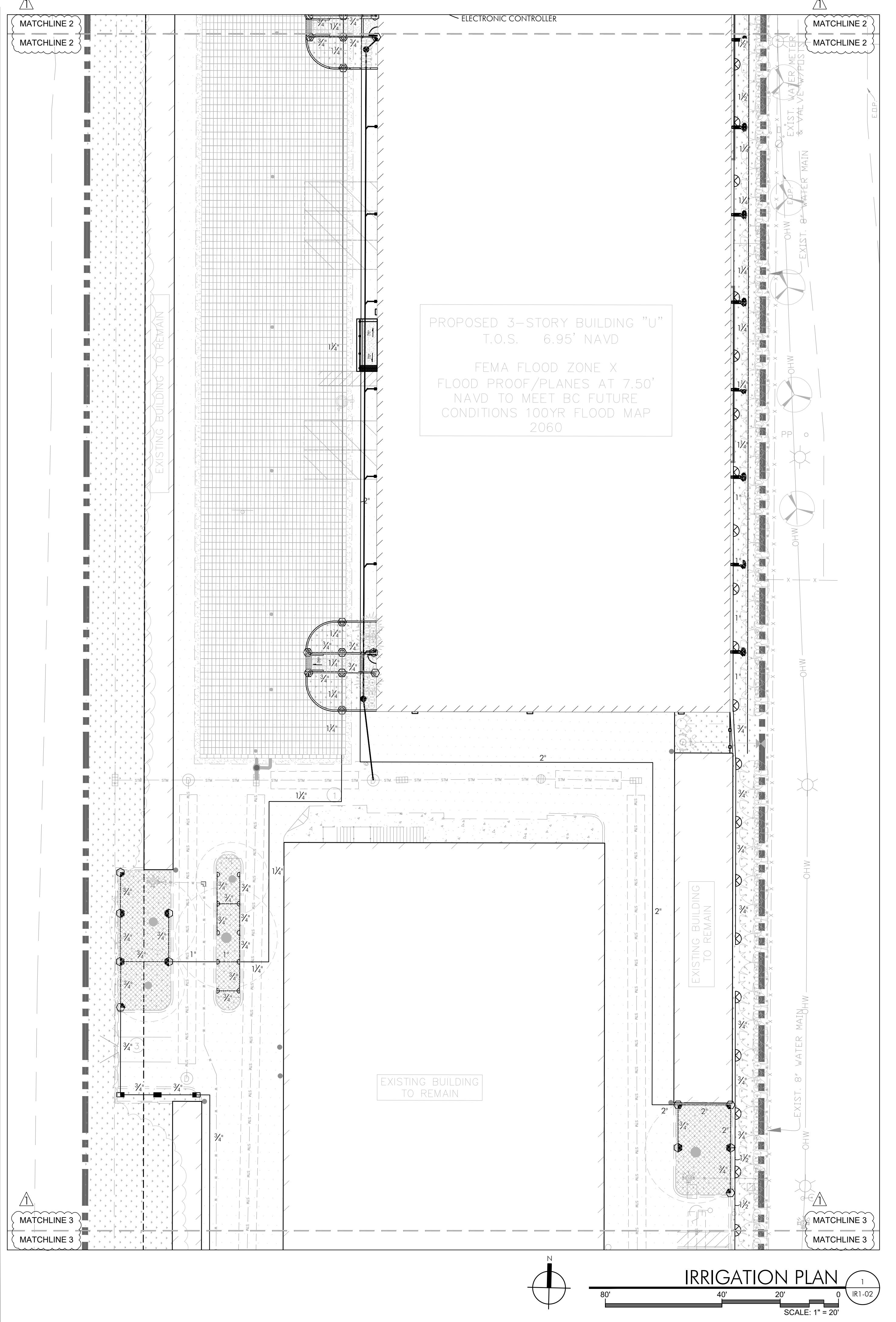


Date: 1/1/2020

FIRE TRUCK VEHICLE TRACKING EXHIBIT

SCALE: 1" = 30'

SHEET No. EXHIBIT 1

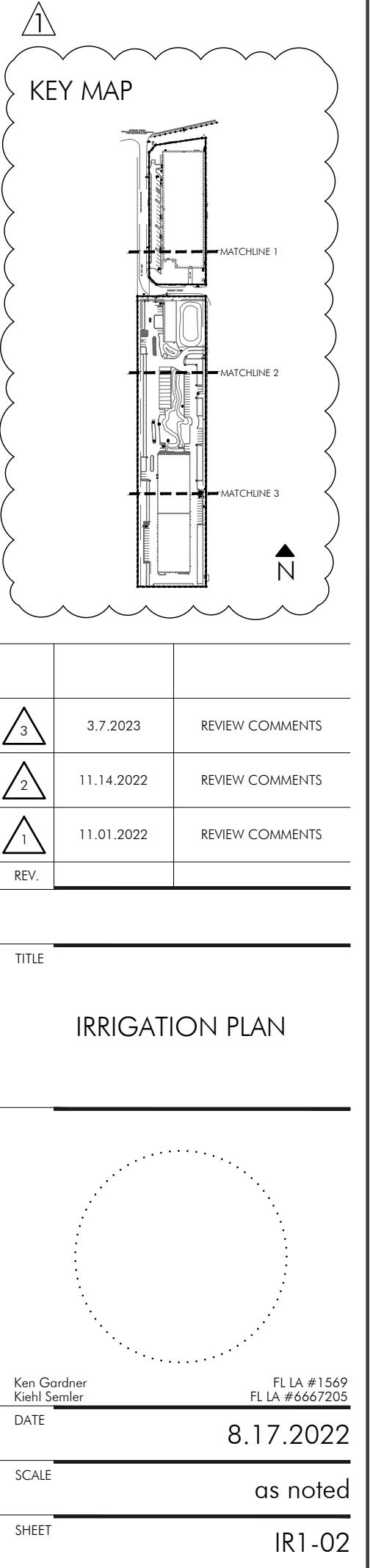


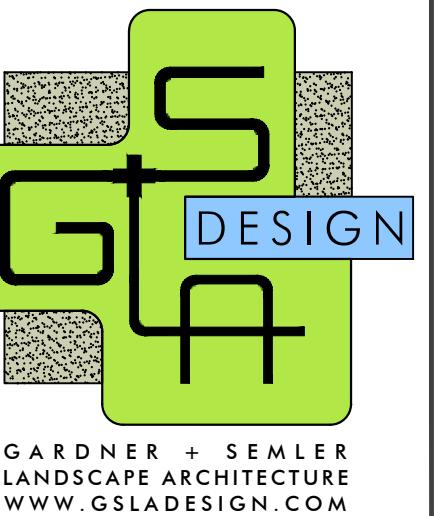
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**SHERIDAN STREET STORAGE ADDITION
DESIGN DEVELOPMENT SITE PLAN**
3090 SHERMAN STREET, HOLLYWOOD, FL 33021





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SHERIDAN STREET STORAGE ADDITION DESIGN DEVELOPMENT SITE PLAN 3090 SHERMAN STREET, HOLLYWOOD, FL 33021

GENERAL NOTES:
1. SCOPE OF WORK: The Contractor shall furnish all labor, machinery, tools, supplies, and equipment as necessary to construct and provide on operating system, as indicated in the Plans. The work shall include, but not be limited to, furnishing materials (pipe, valves, sprinkler heads, fittings, controllers, electrical, wire and fittings, primer, glue, etc.), layout, protection to the public, excavation, assembly, installation, backfilling, compaction, repair of road or pavement surfaces, controller and low voltage feed to the valves, clean-up, maintenance and guarantee, and as-built plans.

2. Contractor shall coordinate with General Contractor or other pertinent Contractors on the job to insure that sleeves are provided and installed under hard surfaces to allow access to all areas to be irrigated. All sleeves shall be constructed of Class 200 PVC. Bury all sleeves a minimum of 18" below the surface. Sleeve to be double the size if the pipe running through it. Sleeve shall extend 24" past the edge of pavement into the area to be irrigated.

3. GUARANTEE: The irrigation system shall be guaranteed for a minimum of one calendar year from the time of final acceptance.

4. REPAIR UTILITIES: The Contractor shall be responsible to verify the location of all utilities by hand excavation or other appropriate measures before performing any work that may result in damage to utilities, structures, or property. The Contractor shall take immediate steps to repair, replace, or restore all services to any utilities which are disrupted due to his operations. All costs involved in disruption of service and repair due to negligence on part of the Contractor shall be his responsibility.

5. AS-BUILT DRAWINGS: Prints of the plans will be supplied to the Contractor for recording "as-built" information. Immediately upon installation of any work which deviates from what is shown on the Plans, the Contractor shall clearly indicate such changes in red pencil on the prints. Such changes shall include, but not be limited to, changes in (1) materials; (2) sizes of material; (3) location; and (4) quantities.

6. The entire installation shall fully comply with all applicable local and state codes and ordinances. The Contractor shall take out all required plumbing and electrical applications and permits, arrange for all necessary inspections and shall pay all fees and expenses in connection with same as part of work under the contract.

7. UNIT PRICES: The successful bidder shall furnish, to the Owner, a unit price breakdown for all materials. The Owner may at his own discretion, add to or delete from the materials, using the unit price breakdown submitted to and accepted by the Owner.

8. MAINTENANCE PERIOD: The irrigation system shall be maintained for a period of 90 days after final acceptance of installation. Maintenance shall include checking of the system 2 times per week. Contractor shall be responsible to replace/repair any broken or malfunctioning parts of the system including those damaged by accidents or vandalism. Repairs shall be made immediately at the time of inspection or when notified by the Landscape Architect.

9. The irrigation system shall provide 100% coverage with a minimum of 90% overlap of water spray.

10. The system is design to provide sprinkler precipitation rates that are nearly equal in each zone. Mixing of sprinklers with widely varying precipitation rates in a zone will not be accepted.

11. Irrigation mainline shall be made of Class 200 PVC and all laterals shall be Class 200 PVC, except flexible PVC (or Toro funny pipe) for flexible swing joint and Schedule 40 PVC risers for spray heads in shrub areas. Schedule 40 galvanized steel pipe is to be used for all above ground fittings. Pipe locations shall be adjusted in the field. When laying out mains and laterals, locate pipe near edges of pavement or against buildings wherever possible, to allow space for plant rootballs. Coordinate pipe locations with planting. Bury all mains and laterals 18" min. below surface. Depth shall be measured to top of pipe.

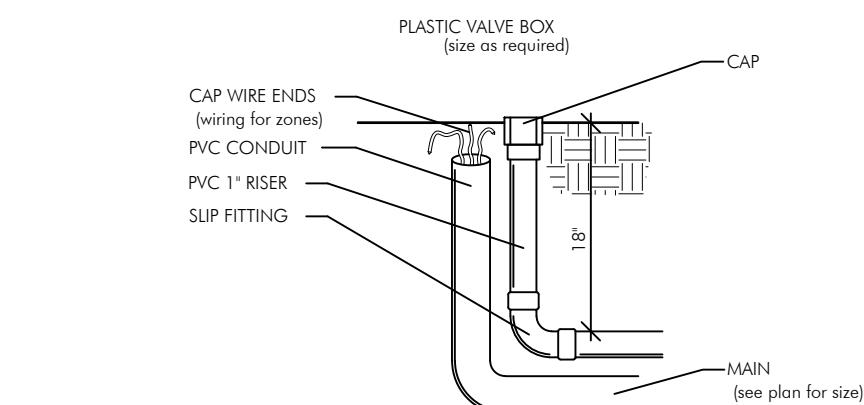
12. Keep pop-up sprinkler heads a minimum of 8" from edges of pavement and curbing, and heads on risers a minimum of 18", or as indicated in the plans.

13. All heads located in shrub or groundcover beds shall be installed on a riser as per details in the plans. All other heads shall be installed on a swing joint as per details in the plans.

14. Place irrigation control wire in conduit in the same trench as mains and under the main. ASI wire shall be #14 or larger solid copper UL approved underground direct burial cable and shall be continuous with no splices from controller to solenoid valve.

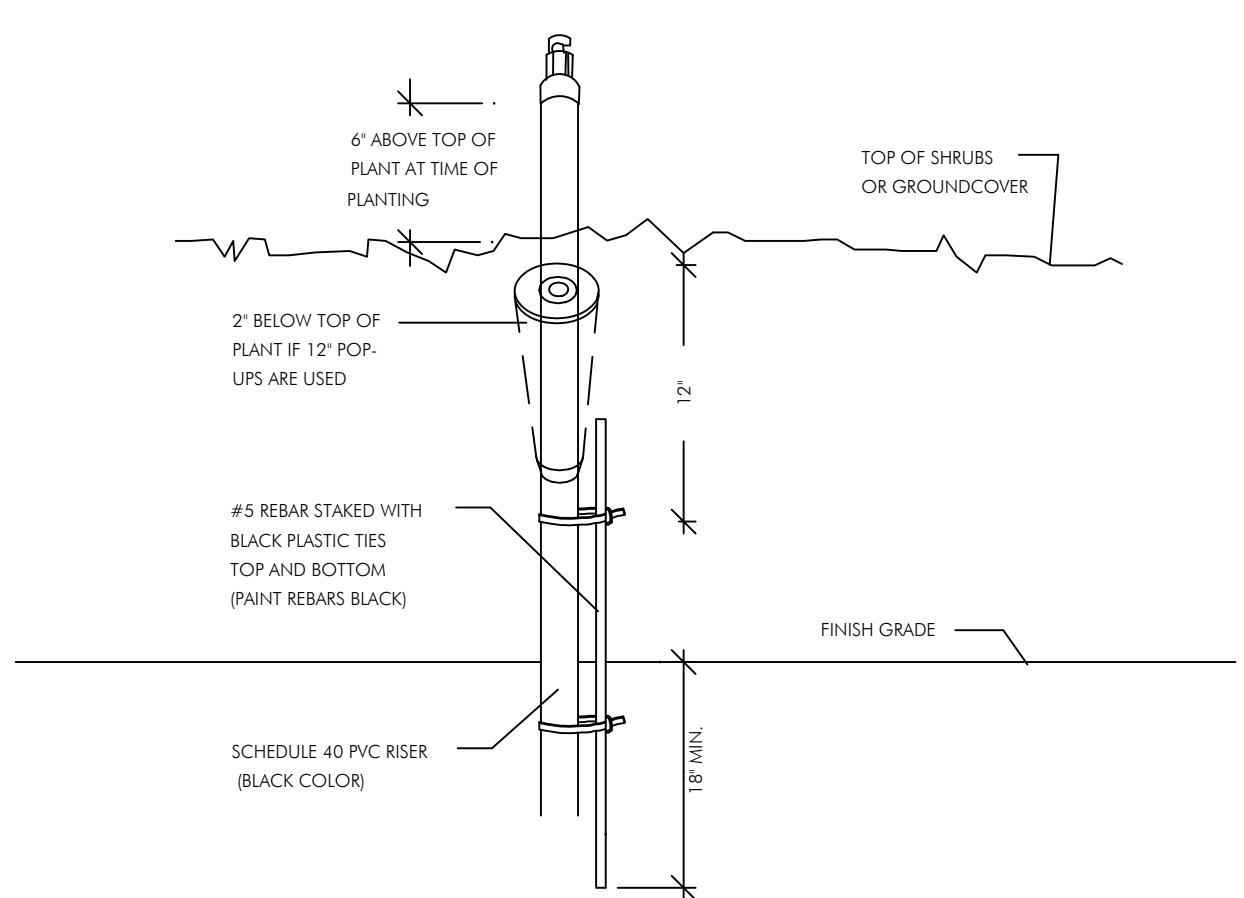
15. Valve locations are schematic and shall be adjusted in the field. Each valve shall be in a separate valve box (12" x 16" min.). When pruning valve boxes in grass or groundcover areas, set boxes a minimum of 12" apart to allow grass or groundcover to grow between them. When possible, hide valve boxes in shrub beds, a minimum of 12" from edge of beds. Set all valve boxes, concrete or plastic, in ground with cover flush with finish grade, and level, with a minimum of 6" of pea gravel at the bottom of the box, with at least 2" of clearance from the bottom of the valve to the top of the gravel.

16. TESTING: Notify the Landscape Architect in writing when testing will be conducted. Conduct test in the presence of the Landscape Architect. After all PVC assembly is completed the lines shall be flushed to insure that no rocks, sand, or other foreign debris remains in the lines. The mains shall be filled with water and all outlets shall be capped and plugged. The main shall be pressurized to 100 PSI for a minimum of one hour. No section of the main shall be removed if the pressure drops below 5 PSI at the end of one hour period. Leaks shall be repaired immediately and the system shall be re-tested until found satisfactory by the Landscape Architect.



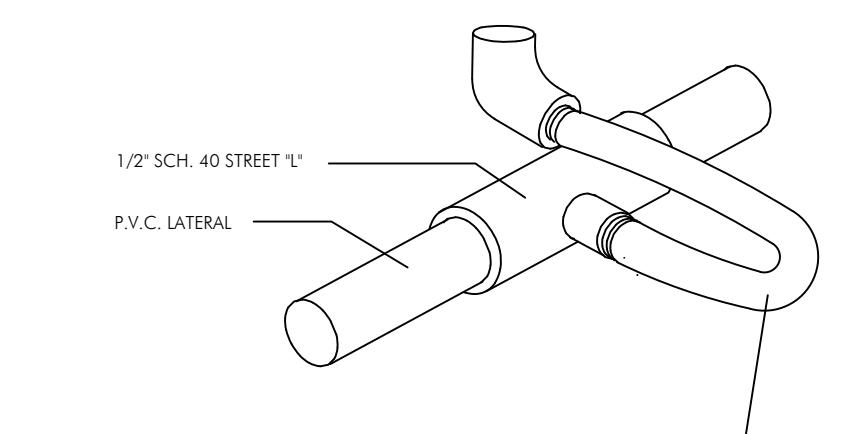
DETAIL OF STUB-OUT FOR FUTURE USE

N.T.S.



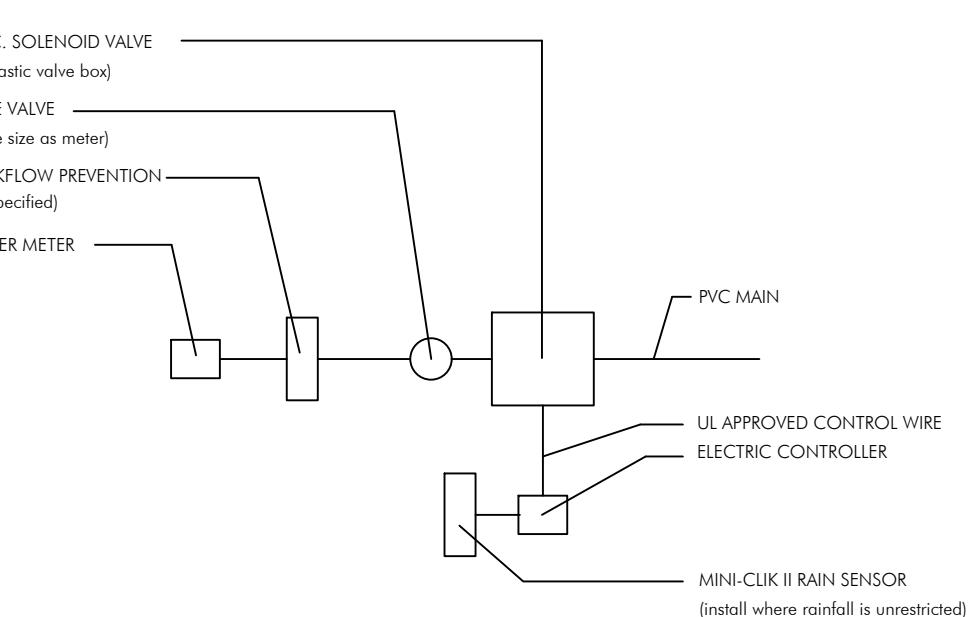
SPRINKLER ON RISER DETAIL FOR SHRUB AREAS

N.T.S.



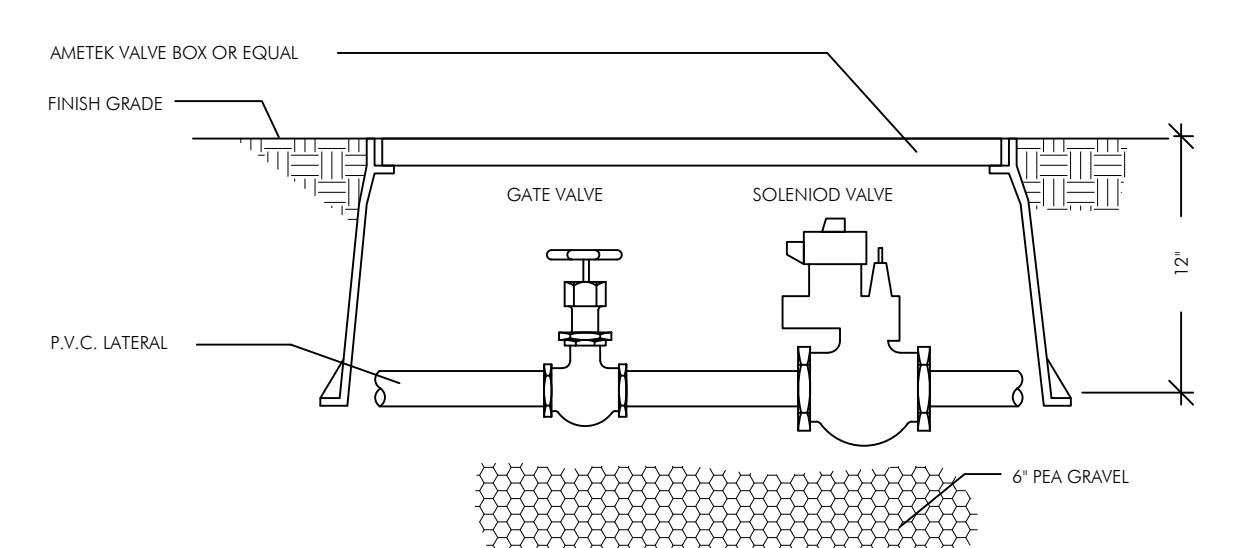
FLEXIBLE SWING JOINT DETAIL

N.T.S.



CONNECTION TO METER DETAIL

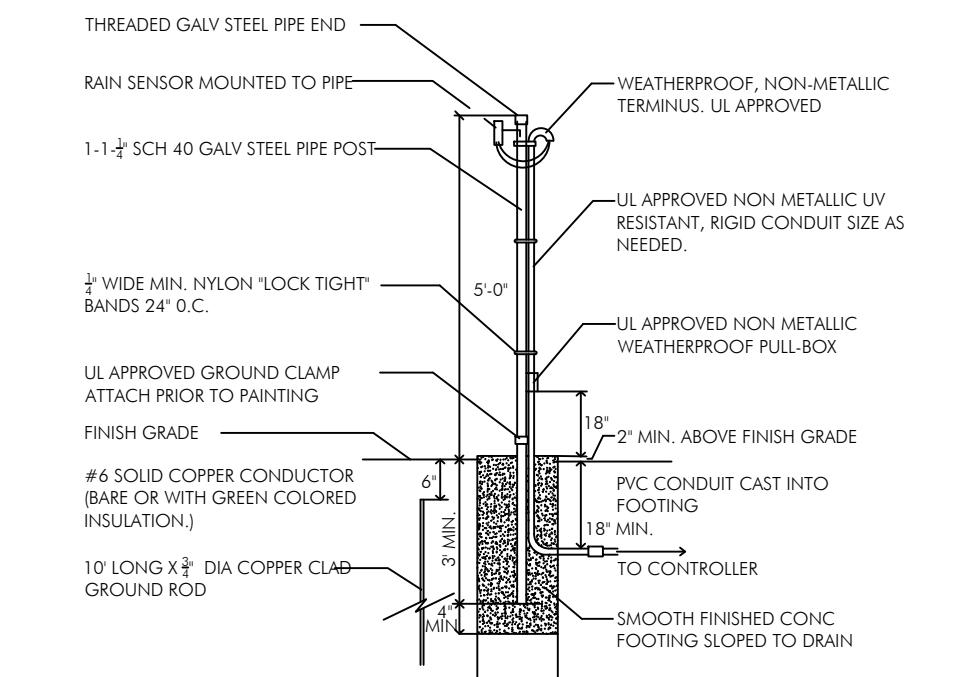
N.T.S.



TYPICAL SOLENOID VALVE ASSEMBLY

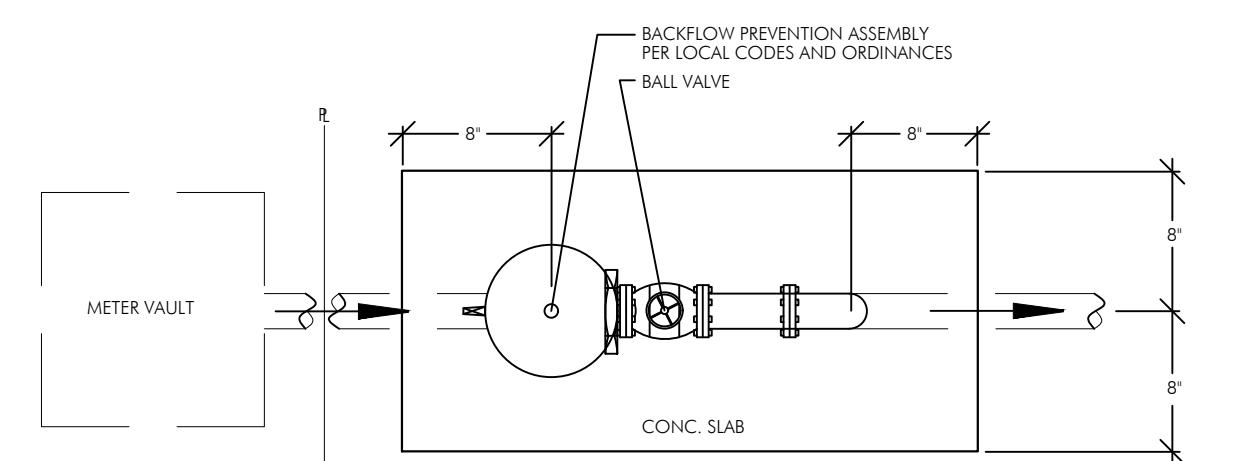
N.T.S.

NOTE:
ALL WIRE CONNECTIONS SHALL BE APPROVED WATERTIGHT CONNECTIONS.
FINISH ENTIRE ASSEMBLY, EXCEPT FOR EQUIPMENT, WITH FLAT BLACK ACRYLIC ENAMEL PAINT.
PRIME METALLIC SURFACES WITH ZINC CHROMATE PRIOR TO FINISHING.



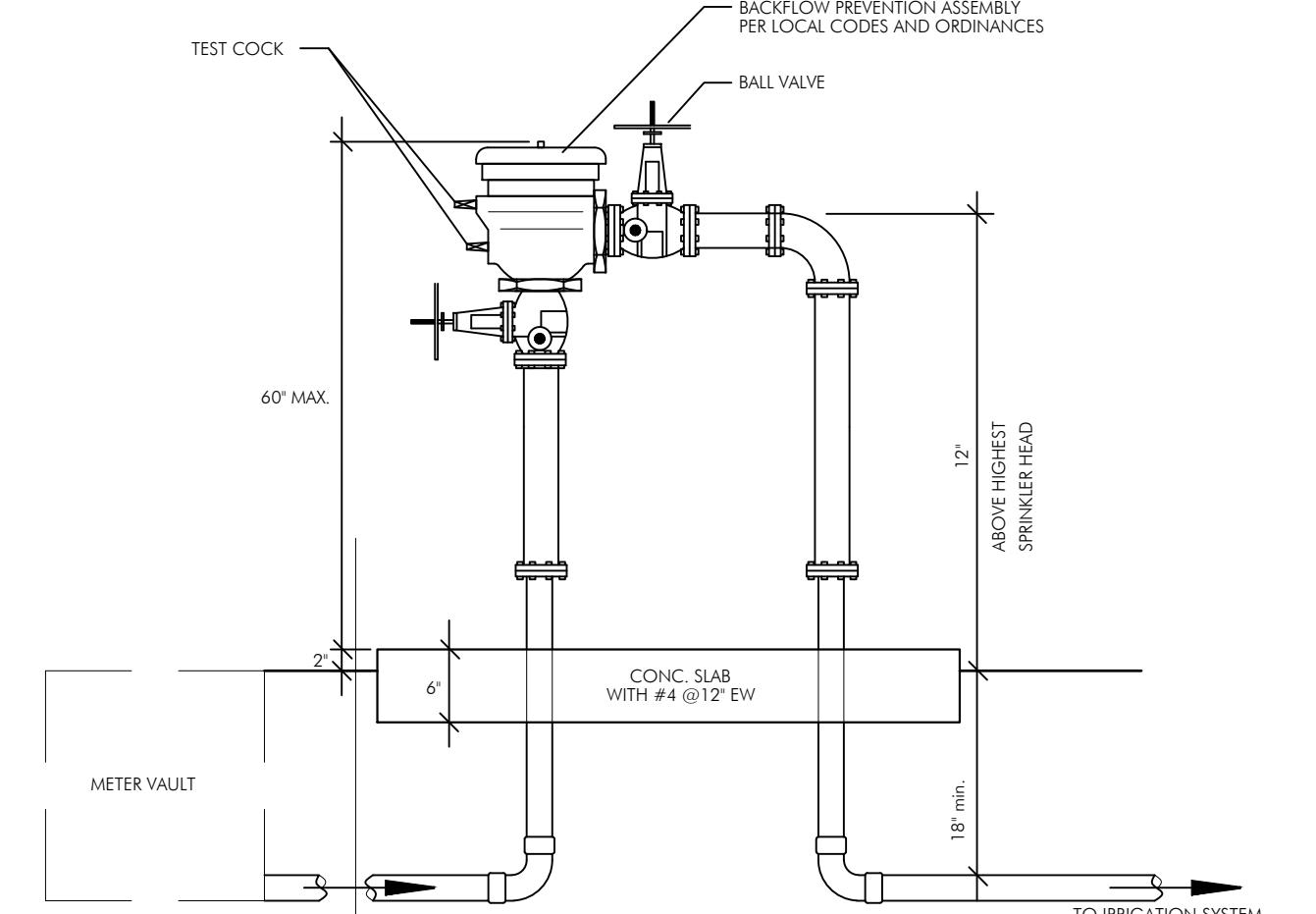
RAIN SENSOR DETAIL

N.T.S.



PLAN VIEW

N.T.S.



ELEVATION VIEW

N.T.S.

BACKFLOW PREVENTION ASSEMBLY DETAIL
IRRIGATION SYSTEM ONLY

N.T.S.

NOTES:
- MATERIALS: PIPES AND FITTINGS SHALL BE APPROVED BY ASAE STANDARDS APPENDIX F.
- THE ASSEMBLY SHALL BE INSTALLED WITH MINIMUM HORIZONTAL CLEARANCES OF 30 INCHES FREE FROM OBSTRUCTIONS IN ALL DIRECTIONS.
- GUARD POSTS SHALL BE INSTALLED IF THE ASSEMBLY IS EXPOSED TO POSSIBLE DAMAGE FROM VEHICULAR TRAFFIC, AS DETERMINED BY THE DEPARTMENT.
- THE ASSEMBLY SHALL BE INSTALLED IN AN ACCESSIBLE LOCATION, APPROVED BY THE DEPARTMENT.
- PIPING SHALL BE SCHEDULE 40 BRASS OR TYPE K COPPER PIPE WITH THREADED FITTINGS IN ACCORDANCE WITH WSD CONSTRUCTION SPECIFICATIONS FOR DONATION WATER MAINS. PVC PIPING IS NOT ACCEPTED BY WSD.
- THE DEPARTMENT SHALL HAVE UNRESTRICTED AND CONTINUOUS ACCESS TO THE VACUUM BREAKER ASSEMBLY.
- SEE SPECIFICATIONS AND CONTACT DEPARTMENT FOR CURRENTLY APPROVED TYPES OF BACKFLOW PREVENTION ASSEMBLIES AND PRESSURE VACUUM BREAKERS (SEE WS 4.18 SHEET 4 OF 4 OR ON THIS SHEET)

IRRIGATION MATERIALS LIST

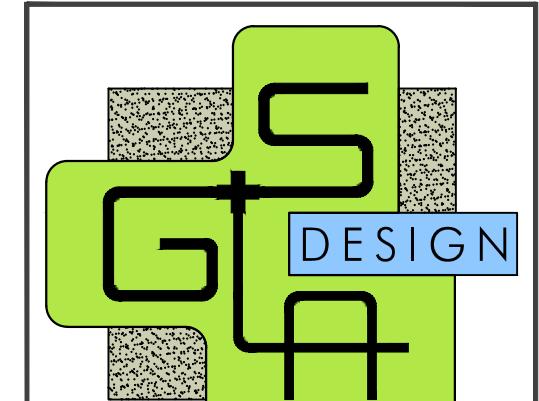
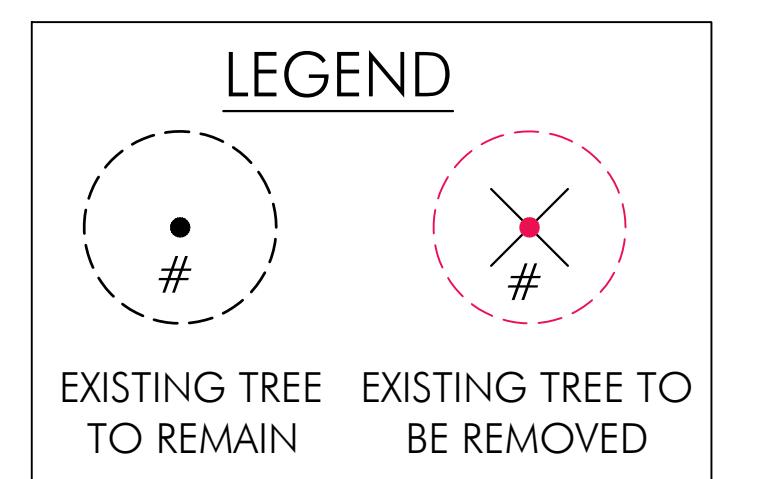
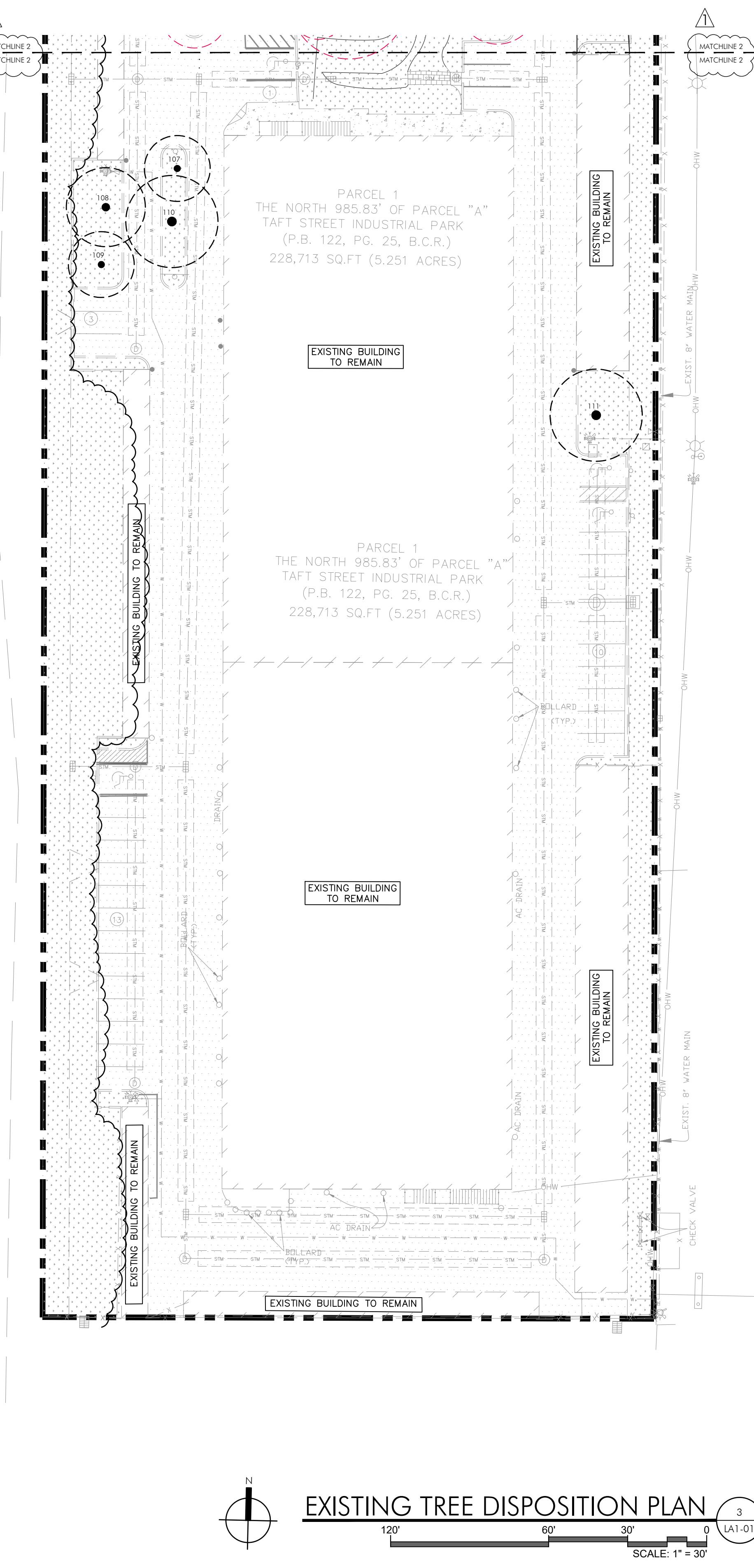
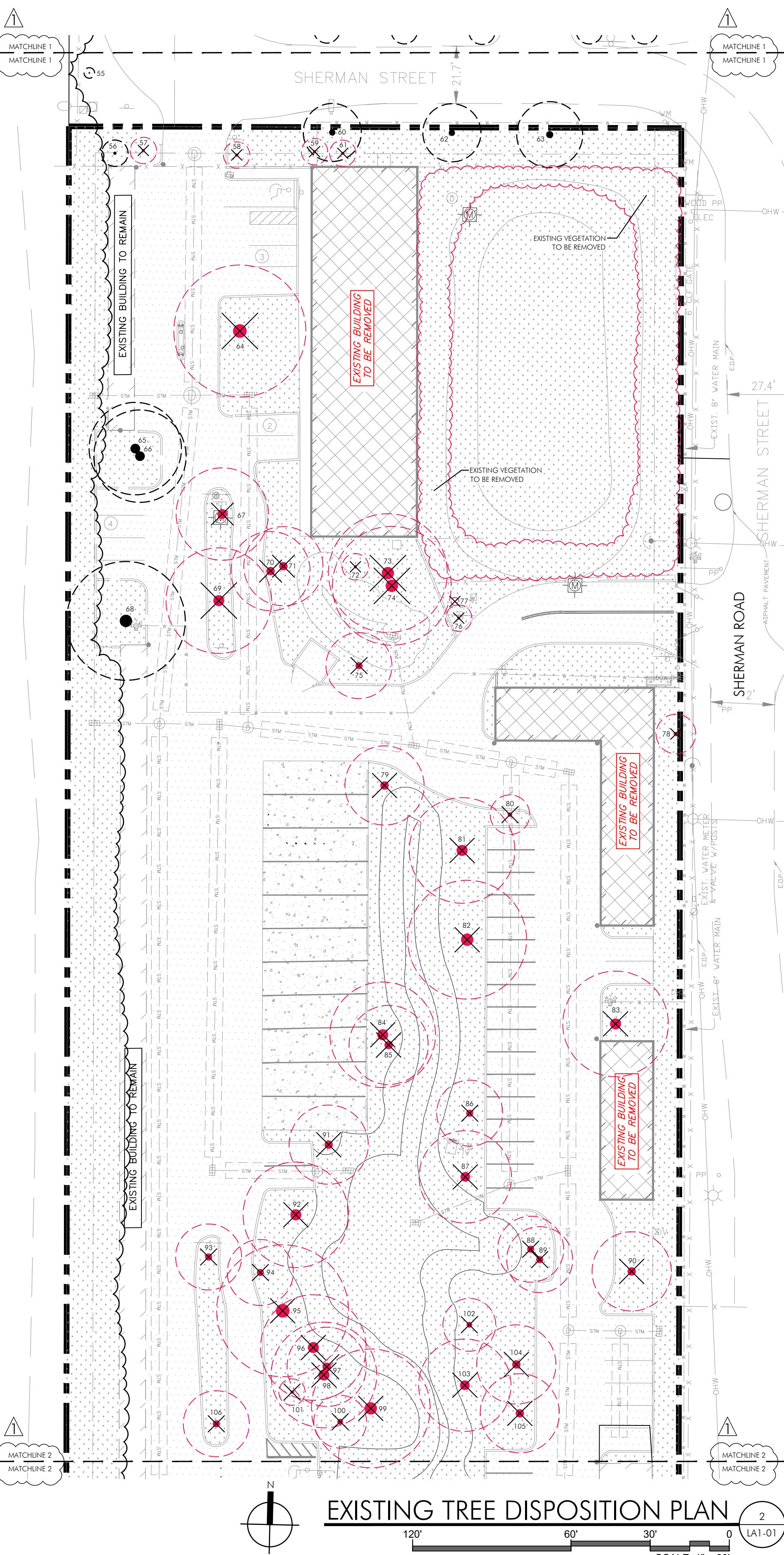
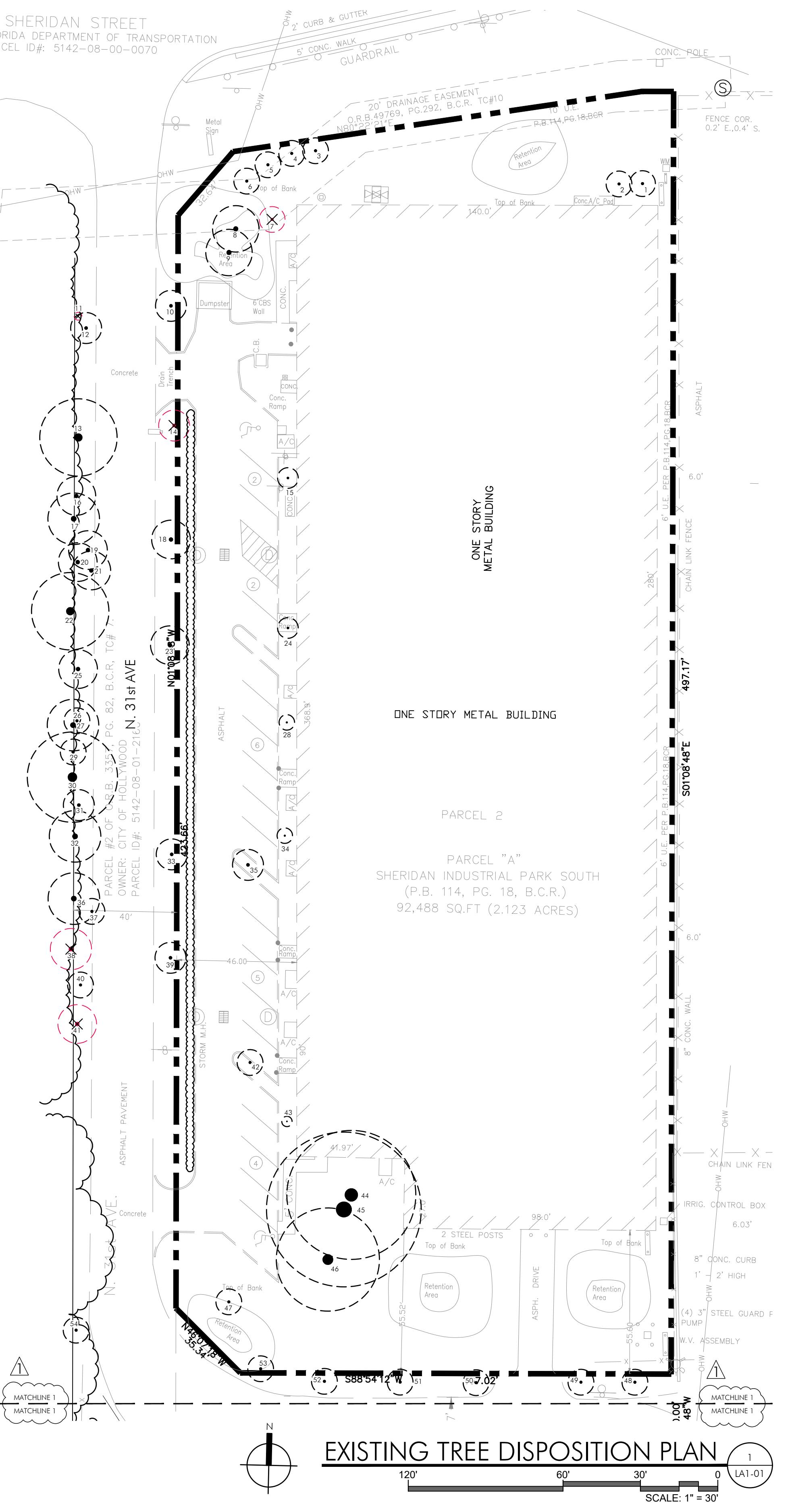
KEY	ITEM	QTY.
—	PVC laterals shall be Class 200 PVC (sized as shown on plans)	as required
—	MAIN shall be Class 200 PVC	as required
—	Bubbler Zone Piping	as required
—	PVC sleeves shall be Class 200 PVC (sized double the width of the pipe running through it)	as required
—	Flexible PVC or Poly pipe (for swing joints)	as required
WM	WATER METER (See Civil Plans)	1
EC	Electric Controller RAINBIRD _____ Series Controller	1
▲	Rainbird RSD Series Rain Sensor (locate in area of free rainfall)	1
●	RAINBIRD 200-PESB 2" Electromechanical Solenoid Control Valve	11
—	Irrigation Control Wire	as required
—	RAINBIRD Spray Heads 1800 @ 30 PSI Series w/MPR nozzles	as required
—	6" pop-up in grass areas	
—	12" pop-up on risers in shrub beds	
—	(1.21 gpm) 15-est (.61 gpm)	
—	9-sst (1.73 gpm)	
—	10-F (1.58 gpm)	
—	10-TQ (1.18 gpm)	
—	10-H (.79 gpm)	
—	10-T (.53 gpm)	
—	10-Q (.39 gpm)	
—	5-F (.41 gpm)	
—	5-TQ (.33 gpm)	
—	5-H (.20 gpm)	
—	5-T (.13 gpm)	
—	5-Q (.10 gpm)	
—	RAINBIRD Rotary Spray Heads 17'-24" radius R-VAN 24 Series @ 40 PSI	as required
—	6" pop-up in grass areas	
—	12" pop-up on risers in shrub beds	
—	(3.13 gpm) 24-360 (2.31 gpm)	
—	24 (270°) (1.54 gpm)	
—	24 (180°) (1.0 gpm)	
—	24 (120°) (.77 gpm)	
—	RAINBIRD Rotary Spray Heads 13'-18" radius R-VAN 18 Series @ 40 PSI	as required
—	6" pop-up in grass areas	
—	12" pop-up on risers in shrub beds	
—	(1.8 gpm) 18-360 (1.42 gpm)	
—	18 (270°) (.98 gpm)	
—	18-(180°) (.6 gpm)	
—	18-(120°) (.5 gpm)	
—	RAINBIRD Rotary Spray Heads 8'-14' radius R-VAN 14 Series @ 40 PSI	as required
—	6" pop-up in grass areas	
—	12" pop-up on risers in shrub beds	
—	(1.22 gpm) 14-360 (1.92 gpm)	
—	14 (270°) (.61 gpm)	
—	14-(180°) (.4 gpm)	
—	14-(120°) (.31 gpm)	
—	FALCON 6504 w/ Rain-Curtain Nozzle @ 50 PSI	as required
—	6.0 nozzle (14.3 gpm) 4.0 nozzle (11.0 gpm) 3.0 nozzle (7.4 gpm) 1.5 nozzle (3.7 gpm)	
—	5000/Series Rotor Heads @ 45 PSI 35' RADIUS	
—	8.0 nozzle (7.58 gpm)	
—	6.0 nozzle (5.73 gpm)	
—	4.0 nozzle (3.81 gpm)	
—	2.0 nozzle (1.92 gpm)	
—	LATERAL PIPE SIZING	
—	The Contractor is responsible to properly size all laterals. All laterals shall be sized according to the following schedule. Total gallons per pipe section shall be calculated by adding the GPM per head downstream of the pipe.	
—	IRRIGATION DESIGN TO PROVIDE 100% COVERAGE	
—	IRRIGATION SPECIFICATIONS AND DETAILS	
—	Ken Gardner Kelli Semler DATE 8.17.2022 SCALE as noted SHEET IR5-01	

3	3.7.2023	REVIEW COMMENTS
2	11.14.2022	REVIEW COMMENTS
1	11.01.2022	REVIEW COMMENTS

REV		
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TITLE	IRRIGATION SPECIFICATIONS AND DETAILS
FLA #1569 FLA #667205	

SHERIDAN STREET
IRIDA DEPARTMENT OF TRANSPORTATION
CEL ID#: 5142-08-00-0070

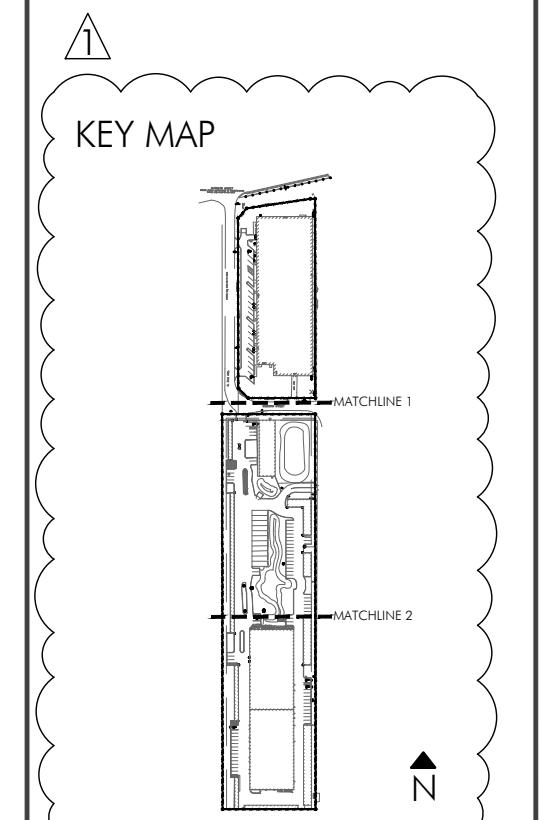


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**SHERIDAN STREET STORAGE ADDITION
DESIGN DEVELOPMENT SITE PLAN**
3090 SHERMAN STREET, HOLLYWOOD, FL 33021

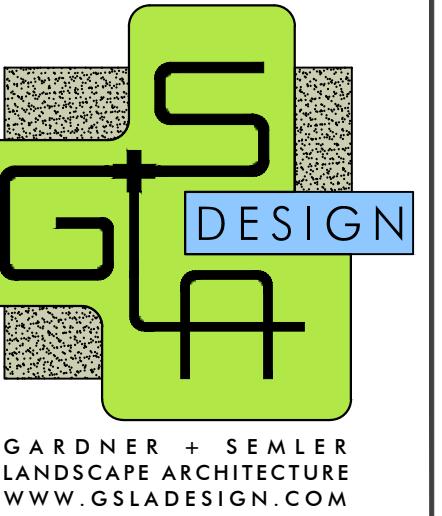


3	3.7.2023	REVIEW COMMENTS
2	11.14.2022	REVIEW COMMENTS
1	11.01.2022	REVIEW COMMENTS

TITLE
EXISTING TREE
DISPOSITION
PLAN

Ken Gardner
Kobi Sempel
DATE 8.17.2022
SCALE as noted
SHEET LA1-01
FL LA # 1560
FL LA # 6667205

**SHERIDAN STREET STREET STORAGE ADDITION
DESIGN DEVELOPMENT SITE PLAN**
3090 SHERMAN STREET, HOLLYWOOD, FL 33021

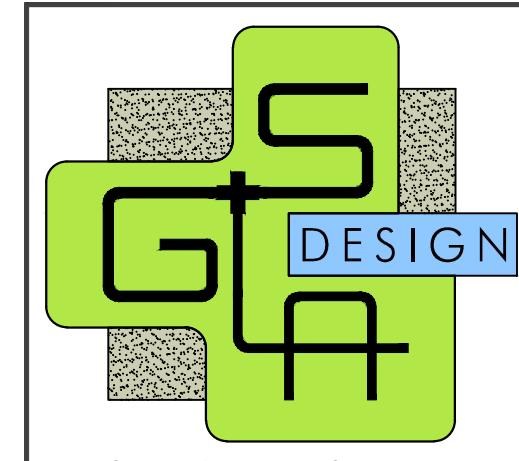
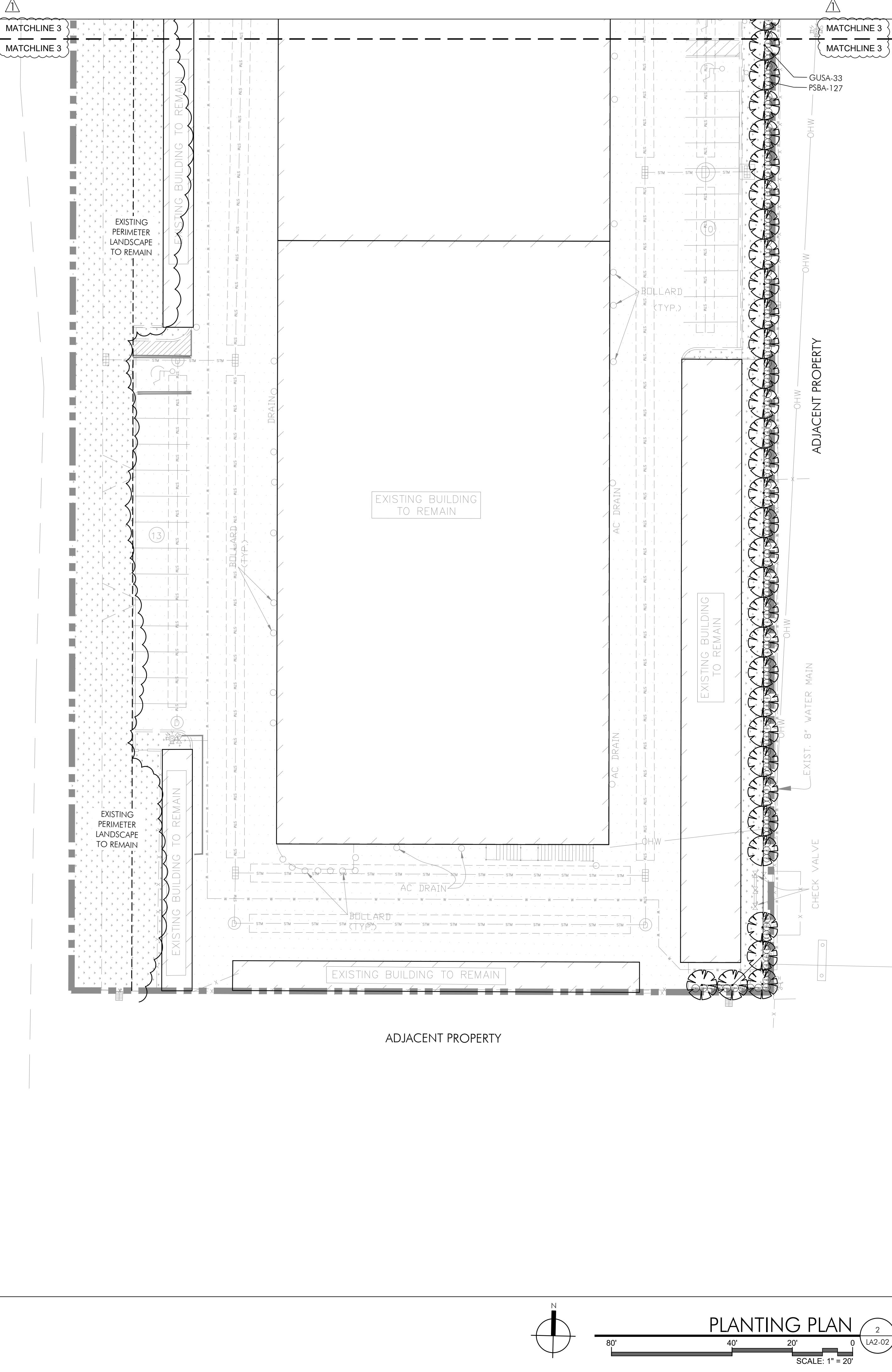
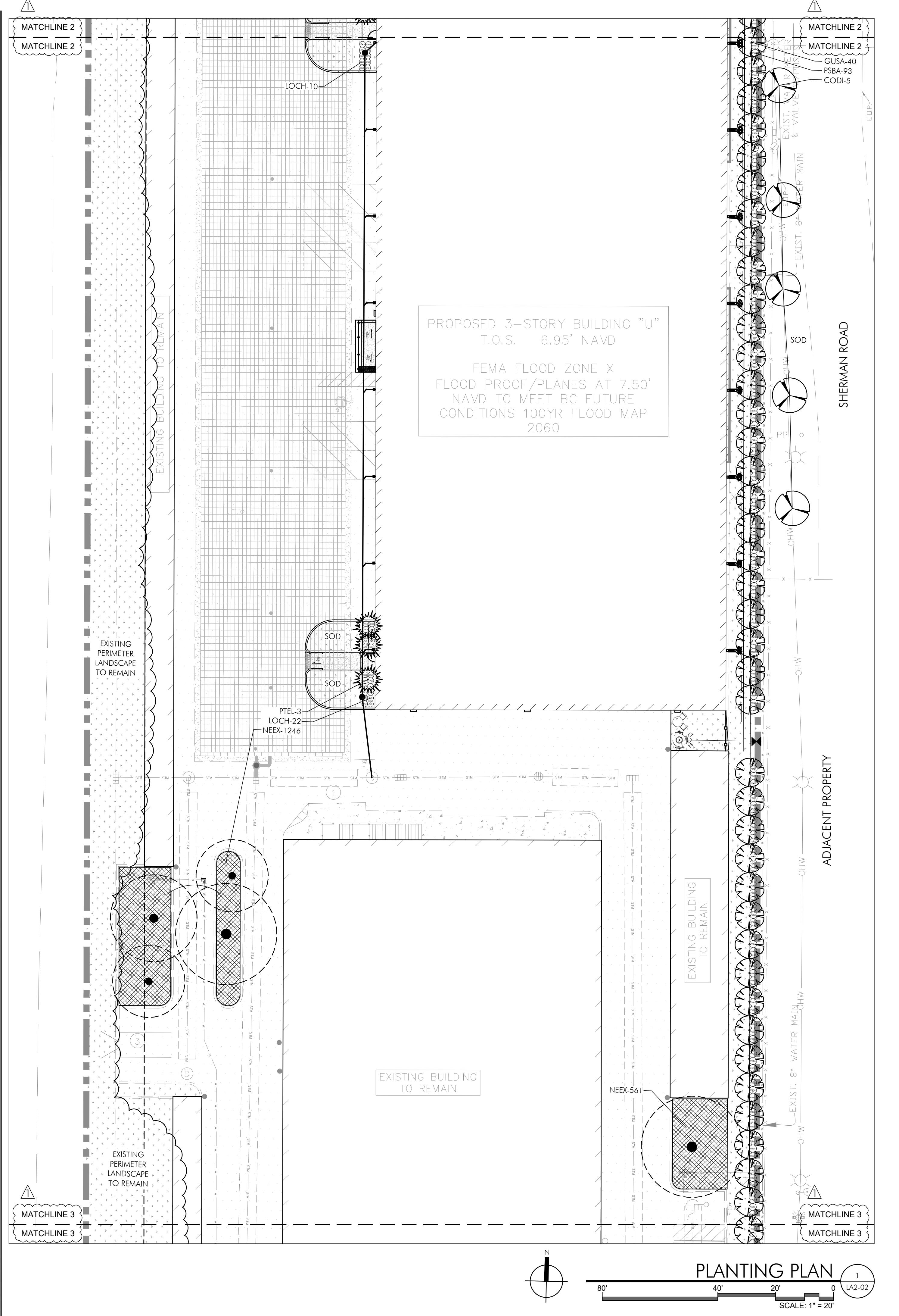


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KEY	BOTANICAL NAME	COMMON NAME	SIZE			DISPOSITION	NOTES	MITIGATION DBH (in.)
			HT.(ft.)	SPD.(ft.)	DBH (in.)			
1	Sabal palmetto	Sabal Palm	18	10	10	remove		10
2	Sabal palmetto	Sabal Palm	18	10	10			
3	Sabal palmetto	Sabal Palm	18	10	10			
4	Sabal palmetto	Sabal Palm	18	10	10			
5	Sabal palmetto	Sabal Palm	18	10	10			
6	Sabal palmetto	Sabal Palm	18	10	10			
7	Ficus aurea	Strangler Fig	18	10	10	remove		10
8	Conocarpus erectus 'Sericeus'	Silver Buttonwood	15	18	10+5			
9	Conocarpus erectus 'Sericeus'	Silver Buttonwood	12	18	10+6			
10	Tabebuia cariba	Yellow Trumpet Tree	16	12	12			
11	Dead Trunk					remove		n/a
12	Sabal palmetto	Sabal Palm	15	10	10			
13	Ficus aurea	Strangler Fig	30	30	4' wide			
14	Dead					remove		n/a
15	Ilex cassine	Dahoon Holly	12	8	8			
16	Sabal palmetto	Sabal Palm	15	10	10			
17	Ficus aurea	Strangler Fig	25	20	5			
18	Tabebuia cariba	Yellow Trumpet Tree	15	15	12			
19	Sabal palmetto	Sabal Palm	12	10	10			
20	Sabal palmetto	Sabal Palm	20	10	10			
21	Sabal palmetto	Sabal Palm	8	10	10			
22	Ficus aurea	Strangler Fig	25	30	6' wide			
23	Conocarpus erectus	Green Buttonwood	18	15	10			
24	Ilex cassine	Dahoon Holly	15	8	7			
25	Ficus aurea	Strangler Fig	20	15	12			
26	Sabal palmetto	Sabal Palm	8	10	too low			
27	Ficus aurea	Strangler Fig	20	20	10			
28	Ilex cassine	Dahoon Holly	12	6	6			
29	Sabal palmetto	Sabal Palm	15	10	10			
30	Ficus aurea	Strangler Fig	30	35	8' wide			
31	Sabal palmetto	Sabal Palm	20	10	10			
32	Ficus aurea	Strangler Fig	18	20	14			
33	Conocarpus erectus	Green Buttonwood	20	12	10			
34	Ilex cassine	Dahoon Holly	12	6	4			
35	Conocarpus erectus 'Sericeus'	Silver Buttonwood	12	12	5			
36	Ficus aurea	Strangler Fig	18	20	10+8			
37	Sabal palmetto	Sabal Palm	16	10	10			
38	Dead / Hacked off					remove		n/a
39	Tabebuia cariba	Yellow Trumpet Tree	18	12	10			
40	Sabal palmetto	Sabal Palm	15	10	10			
41	Dead					remove		n/a
42	Conocarpus erectus 'Sericeus'	Silver Buttonwood	12	10	6			
43	Ilex cassine	Dahoon Holly	12	4	3			
44	Quercus virginiana	Live Oak	40	50	21			
45	Quercus virginiana	Live Oak	40	50	48			
46	Quercus virginiana	Live Oak	30	40	26			
47	Sabal palmetto	Sabal Palm	18	10	10			
48	Sabal palmetto	Sabal Palm	18	10	10			
49	Sabal palmetto	Sabal Palm	18	10	10			
50	Sabal palmetto	Sabal Palm	18	10	10			
51	Sabal palmetto	Sabal Palm	18	10	10			
52	Sabal palmetto	Sabal Palm	18	10	10			
53	Sabal palmetto	Sabal Palm	18	10	10			
54	Sabal palmetto	Sabal Palm	20	10	10			
55	Ficus aurea	Strangler Fig	12	4	2+3			
56	Sabal palmetto	Sabal Palm	16	10	10			
57	Sabal palmetto	Sabal Palm	18	10	10			
58	Sabal palmetto	Sabal Palm	18	10	10	remove		1 palm
59	Sabal palmetto	Sabal Palm	18	10	10	remove		1 palm
60	Quercus virginiana	Live Oak	25	22	12			
61	Sabal palmetto	Sabal Palm	18	10	10	remove		1 palm
62	Quercus virginiana	Live Oak	25	22	10			
63	Quercus virginiana	Live Oak	28	25	14			
64	Quercus virginiana	Live Oak	50	30	40	remove		40
65	Quercus virginiana	Live Oak	35	35	19			
66	Pinus elliottii	Slash Pine	50	25	20	remove		20
67	Quercus virginiana	Live Oak	45	45	26			
68	Quercus virginiana	Live Oak	45	40	32	remove		32
69	Quercus virginiana	Live Oak	40	30	15	remove		15
70	Quercus virginiana	Live Oak	40	30	15	remove		15
71	Quercus virginiana	Live Oak	40	30	15	remove		15
72	Sabal palmetto	Sabal Palm	22	10	remove			1 palm
73	Quercus virginiana	Live Oak	45	45	28	remove		28
74	Quercus virginiana	Live Oak	45	45	21	remove		21
75	Quercus virginiana	Live Oak	20	25	12	remove		12
76	Sabal palmetto	Sabal Palm	20	10	remove			1 palm
77	Dead					remove		n/a
78	Ilex cassine	Dahoon Holly	12	12	multi 2	remove		12
79	Quercus virginiana	Live Oak	30	30	10	remove		10
80	Quercus virginiana	Live Oak	20	15	6+6+6	remove		18
81	Quercus virginiana	Live Oak	40	40	34	remove		34
82	Quercus virginiana	Live Oak	45	45	54	remove		54
83	Quercus virginiana	Live Oak	40	40	45	remove		45
84	Quercus virginiana	Live Oak	45	40	30	remove		30
85	Quercus virginiana	Live Oak	40	30	24	remove		24
86	Conocarpus erectus	Green Buttonwood	25	25	10+10	remove		20
87	Quercus virginiana	Live Oak	40	35	25	remove		25
88	Quercus virginiana	Live Oak	35	25	25+20	remove		45
89	Quercus virginiana	Live Oak	35	25	28+20	remove		48
90	Quercus virginiana	Live Oak	30	30	32	remove		32
91	Quercus virginiana	Live Oak	30	30	12	remove		12
92	Quercus virginiana	Live Oak	40	40	30	remove		30
93	Quercus virginiana	Live Oak	40	25	15	remove		15
94	Pinus elliottii	Slash Pine	50	25	16	remove		16
95	Quercus virginiana	Live Oak	45	50	25	remove		25
96	Quercus virginiana	Live Oak	40	40	28	remove		28
97	Quercus virginiana	Live Oak	30	30	12	remove		12
98	Quercus virginiana	Live Oak	40	40	12	remove		12
99	Quercus virginiana	Live Oak	45	45	43	remove		43
100	Pinus elliottii	Slash Pine	35	20	10	remove		10
101	Quercus virginiana	Live Oak	20	10	remove			10
102	Quercus virginiana	Live Oak	25	20	14	remove		14
103	Quercus virginiana	Live Oak	40	35	18	remove		18
104	Quercus virginiana	Live Oak	30	30	15	remove		15
105	Quercus virginiana	Live Oak	30	30	14	remove		14
106	Quercus virginiana	Live Oak	28	25	12	remove		12
107	Quercus virginiana	Live Oak	25	25	10			
108	Quercus virginiana	Live Oak	35	30	12			
109	Quercus virginiana	Live Oak	35	25	20		</	



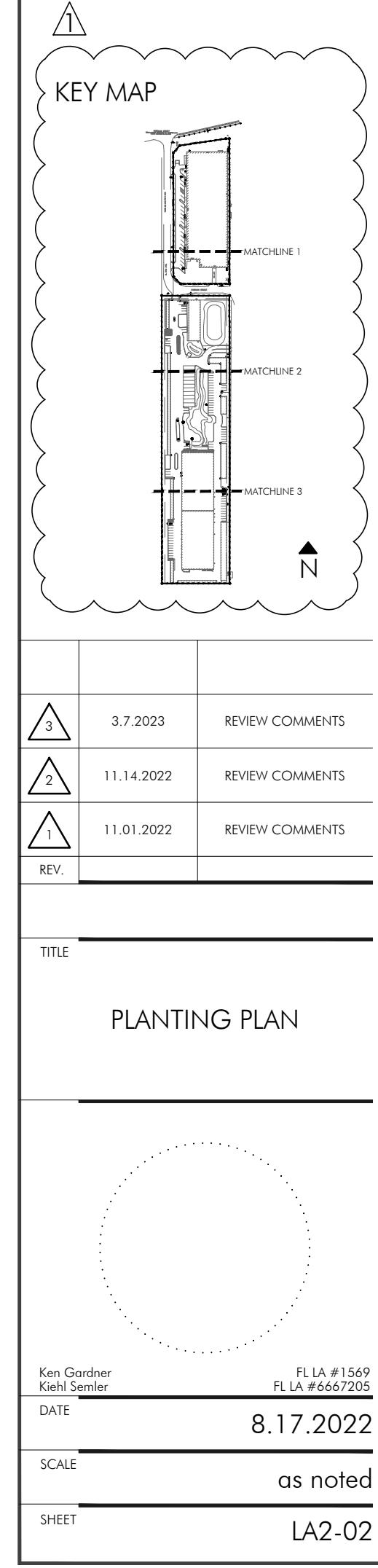
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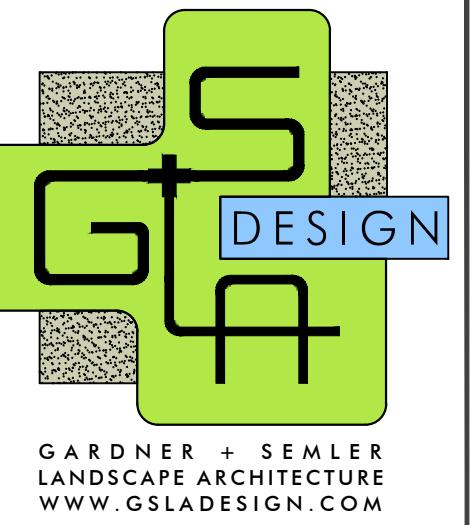
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SHERIDAN STREET STORAGE ADDITION DESIGN DEVELOPMENT SITE PLAN

3090 SHERMAN STREET, HOLLYWOOD, FL 33021





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**SHERIDAN STREET STORAGE ADDITION
DESIGN DEVELOPMENT SITE PLAN**

3090 SHERMAN STREET, HOLLYWOOD, FL 33021

PLANT LIST			
TREES - All proposed trees shall be Florida #1 Grade with a min. ht. of 12 feet			
KEY	PLANT NAME	QTY.	UT. SIZE
BUSI	Bursera simaruba ...Gumbo Limbo	8	ea. 14' tall x 6' spread, 2" DBH
CODI	Coccocoba diversifolia ...Pigeon Plum	10	ea. 12' tall x 5' spread, 2" DBH
COES	Conocarpus erectus var. "Sericeus" ...Silver Buttonwood	21	ea. 12' tall x 5' spread, 2" DBH; single trunk
GUSA	Guaiacum sanctum ...Lignum Vitae	95	ea. 12' tall x 5' spread, 2" DBH
ILCA	Ilex cassine ...Dahoon Holly	15	ea. 12' tall x 5' spread, 2" DBH
LIJA	Ligustrum japonicum ...Japanese Privet	15	ea. 12' tall x 5' spread, 2" DBH; single trunk
TADI	Taxodium distichum ...Bald Cypress	9	ea. 14' tall x 6' spread, 2" DBH
QUVI	Quercus virginiana ...Live Oak	7	ea. 14' tall x 6' spread, 2" DBH
PALMS			
KEY	PLANT NAME	QTY.	UT. SIZE
PTEL	Ptychosperma elegans ...Solitaire Palm	5	ea. 20'-22' tall OA
CONG	Cocos nucifera ...Maypan Coconut Palm	5	ea. 3 @ 24' tall OA (8' CT); 2 @ 28' tall OA (12' CT); heavily leaning. Mitigation Palms.
SHRUBS - 24" minimum shrub height at time of planting as per city code			
KEY	PLANT NAME	QTY.	UT. SIZE
CLGU	Clusia guttifera ...Clusia	164	ea. 30' x 30'
LOCH	Loropetalum chinense ...Chinese Fringe Flower	44	ea. 24" x 24"
POMA	Podocarpus macrophyllus ...Podocarpus	20	ea. 24" x 24"
PSBA	Psychotria bahamensis ...Bahamas Wild Coffee	366	ea. 24" x 24"
SPBA	Spartina bakerii ...Sand Cordgrass	57	ea. 24" x 24"
TRDA	Tripsacum dactyloides ...Fakahatchee Grass	56	ea. 24" x 24"
GROUNDCOVERS			
KEY	PLANT NAME	QTY.	UT. SIZE
NEEX	Nephrolepis exaltata ...Boston Fern	2683	ea. 12' x 12'
MISCELLANEOUS			
sod	St. Augustine "CitraBlue"	as req.	s.f. solid sod
	Planting Soil 70% Silica Sand 20% Everglades Muck 10% Shredded Pinebark	as req.	c.y.
	Shredded Melaleuca Mulch	as req.	c.y. 3" layer in all shrub beds

LANDSCAPE LEGEND			
Zoning District			IM-1
Total Site Area			7.37
		Required	Provided
Pervious Area (40%)		128,480	101,958
Code Section	Description	Required	Provided
9.5 A	Perimeter Landscape Setback	5'	5'
9.5 B	Terminal Islands	10	10
9.5 C 1	1 at end of all rows of parking spaces		
9.5 C 1	Landscaping of Paved VUA (25% of VUA)	19,331	19,331
9.5 C 1	VUA = 77,327 s.f. * .25 = 19,331		
9.5 D	Remove EPPC's invasive pest plants	yes	yes
9.5 E	Native Requirement		
9.5 E	60% of trees	111	165
9.5 E	50% of shrubs	399	643
9.5 F	Required Trees		
9.5 F	1/190 s.f. terminal islands	10	10
9.5 F	1/1000 s.f. of pervious area of property	102	142
9.5 G	Required Hedges or Perimeter Screening		
9.5 G	1/10 l.f. abutting adjacent property	52	52
9.5 G	1/5 l.f. abutting adjacent ROW	265	265
Required Street Trees 1/50 l.f.)			
9.5 G	Sheridan St = 172 l.f. = 4 trees	4	4
9.5 G	N. 31st Ave = 456 l.f. = 10 trees	10	10
9.5 G	Sherman Rd. = 700 l.f. = 14 trees	14	14
TOTALS			
	TREES	140	180
	SHRUBS	317	707
	MINIMUM TREE SPECIES	6	8
	50% MAXIMUM OF PALMS (3:1)	6 Palms	10 Palms

NOTE:
(438) 2" dbh trees required for mitigation.
(5) required mitigation palms provided by (5) coconut palms with minimum height of 8' clear trunk.

140 site trees required as per city code.
- 45 existing trees to remain and count towards site tree requirements
=95 required site trees

180 trees proposed
- 95 required site trees
=85 free surplus to count toward mitigation

438 required mitigation trees - 85 surplus trees = 353 required mitigation trees.
Mitigation trees shall be installed at a minimum size of 2" DBH / 12' height

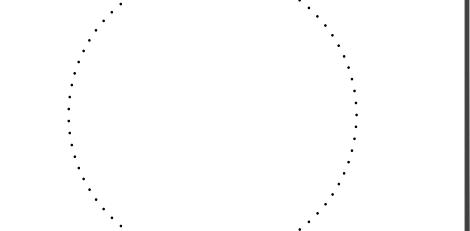
There is a mitigation shortfall of 353 - 2" dbh trees. There isn't sufficient room on site to plant these trees, therefore a donation to the City Trust Fund must be made in the amount of \$123,550.
(\$350 per 2" dbh tree)

3	3.7.2023	REVIEW COMMENTS
2	11.14.2022	REVIEW COMMENTS
1	11.01.2022	REVIEW COMMENTS

REV

TITLE

PLANT LIST

PLANT LIST		
		
Ken Gardner	FLA #1569	
Keiki Semler	FLA #667205	
DATE	8.17.2022	
SCALE	as noted	
SHEET	LA2-03	